

Geotechnical Completion Report

HITCHEN BLOCK STAGE 13B

For

DFH JOINT VENTURE LIMITED

J00113 | LDE Limited | 17 January 2022

Ref No: J00113

17 January 2022

DFH Joint Venture Limited PO Box 302 877 North Harbour 1330

Attention: Mr R Parkinson

Dear Russell

RE: Geotechnical Completion Report for Hitchen Block Stage 13B, Pokeno

This report presents all supporting geotechnical data and our Suitability Statement in relation to land development works undertaken at the above location.

It has been prepared in accordance with instructions received from DFH Joint Venture Limited and forms part of the documentation required by Waikato District Council to achieve certification under Section 224(c) of the Resource Management Act.

If you have any queries or you require any further clarification on any aspects of this report, please do not hesitate to contact the undersigned.

For and On Behalf of LDE Limited

Prepared by:

K.moro

K. Meffan Engineering Geologist MEngNZ

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1 INTRODUCTION AND DESCRIPTION OF SUBDIVISION

This Geotechnical Completion Report has been prepared for DFH Joint Venture Limited as part of the documentation required to be submitted to the Waikato District Council following residential subdivisional development.

It contains our Suitability Statement, relevant test data and the CivilPlan Consultants Limited as-built plan set relating to Stage 13B of the Hitchen Block Residential Subdivision as follows:

| Title | Reference No. | Date | | |
|----------------------------|------------------|----------------|--|--|
| As Built Contours | 126701-13B-AB200 | September 2021 | | |
| As Built Cut-Fill Contours | 126701-13B-AB201 | September 2021 | | |
| Stormwater As Built | 126701-13B-AB400 | September 2021 | | |
| Wastewater As Built | 126701-13B-AB401 | September 2021 | | |

Table 1: CivilPlan Consultants Limited As-Built Plans

This report covers the construction period October 2019 to December 2021. It is intended to be used for certification purposes as follows:

- 58 residential lots numbered 752 to 809.
- 2 new roads named Harris Place and Leathern Crescent (part).
- 5 jointly owned access lots (JOAL's) numbered 5 to 9
- 1 local purpose (drainage) reserve number lot 11. This Lot contains a stormwater pond.

This stage of the subdivision is located as shown on the attached CivilPlan Consultants Limited as-built plans. As can be seen on the Cut-Fill Contours As-Built plan, approximately half of the lots have been partly or totally affected by filling, to a maximum depth of approximately 4m within Lot 11 (drainage reserve). Elsewhere, fills are generally 1m high or less.

2 RELATED REPORTS

A Geotechnical Investigation Report on the subject land was prepared by this Consultancy, reference J00741 (Earthworks Stage 5), dated 29 August 2018. The conclusions and recommendations of that report have been reviewed during the preparation of this document, along with the following Geotechnical Completion Reports (prepared by Lander Geotechnical Consultants Limited) on adjacent recently completed stages of the subdivision which are tabulated below:

| - | | | | | |
|---|--|---------------|------------------|--|--|
| | Subdivision Title | Reference No. | Issue Date | | |
| | Hitchen Block Stage 3A2, 3B and 3C (Residential) | J00113 | 20 December 2017 | | |
| | Hitchen Block Stage 5A (Residential) | J00113 | 26 January 2018 | | |
| | Hitchen Block Stage 4A (Residential) | J00113 | 23 March 2018 | | |
| | Hitchen Block Stage 6A and 6B (Residential) | J00113 | 23 May 2018 | | |
| | Hitchen Block Stage 4B (Residential) | J00113 | 28 August 2018 | | |

 Table 2: Lander Geotechnical Consultants Geotechnical Completion Reports



| Subdivision Title | Reference No. | Issue Date |
|--|---------------|------------------|
| Hitchen Block Stage 7A & 7B (Residential) | J00113 | 2 November 2018 |
| Hitchen Block Stage 8A & 8B (Residential) | J00113 | 9 May 2019 |
| Hitchen Block Stage 6D (Residential) | J00113 | 4 November 2019 |
| Hitchen Block Stages 11, 12 and 14 (Residential) | J00113 | 13 March 2020 |
| Hitchen Block Stage 9 (Residential) | J00113 | 24 June 2020 |
| Hitchen Block Stages 6E & 10A to 10D (Residential) | J00113 | 16 December 2020 |
| Hitchen Block Stages 10E & 10F (Residential) | J00113 | 11 March 2021 |
| Hitchen Block Stage 12D (Residential) | J00113 | 20 April 2021 |
| Hitchen Block Stage 13A (Residential | J00113 | 16 August 2021 |

3 EARTHWORKS OPERATIONS

3.1 Plant

The main items of plant used by the Contractor, Kerry Dines Limited were:

- 7 x bulldozers with scoops;
- 1 x elevating motorscraper;
- 3 x articulated dump trucks;
- 2 x 4WD sheepsfoot compactors;
- 4 x 20T hydraulic excavators;
- 1 x Tractor with disc ploughs.

3.2 Construction Programme

Earthworks operations for this Stage commenced in late October 2021 with topsoil stripping and bulk cut operations of the majority of Stage 13B, including the muckout of the gully passing through the drainage reserve. Underfill drainage (comprising of 160mm perforated drain coils, covered with drainage aggregate and fully wrapped with geotextile cloth) was then installed along the gully invert and along a branch from the main gully where water seepage was observed.

Following this, a toe key (undercut beneath fill batter edge) was installed along the south-western boundary of the drainage reserve and a 3m high fill batter was formed here. The purpose pf the toe key was to 'lock in' the filling into the underlying competent ash soils. Placement of bulk engineered clay fills (Fills J and K as shown on the compaction control test plans included in Appendix 2) progressed until May 2019, however, most work completed during this time was on the surrounding stages.

In December 2020, stripping operations progressed through Stage 13B2 where dense welded tuff materials were encountered and approximately 1m of engineered clay fill was placed over these lots.

By November 2021, all public services and roading was essentially complete and all lots were topsoiled.



4 QUALITY ASSURANCE AND CONTROLS

4.1 Inspections

During earthworks construction, engineering observations were undertaken on a near regular basis to assess compliance with NZS 4431 and our project specific recommendations and specifications. Project specific inspections were required on this stage of the development for:

- Topsoil stripping of earthworks areas;
- Removal of soft sediments in the gully inverts;
- Placement of the underfill drainage;
- Fill placement and plant performance upon the subgrade periodically throughout the bulk filling works.

4.2 Quality Control Criteria

Due to the varying soil types being used as filling, the compaction control criteria of minimum allowable shear strength and maximum allowable air voids were mainly used for quality assurance purposes.

Specification details were as follows for general fills:

| Minimum Shear Strength and Maximum Air Voids | Method |
|--|--------|
|--|--------|

| (a) | Air Voids Percentage | |
|-------|--|---------|
| (4) | (As defined in NZS 4402) | |
| | Average value less than | 10% |
| | Maximum single value | 12% |
| (b) | Undrained Shear Strength | |
| | (Measured by Pilcon shear vane – calibrated using NZGS 2001 method) | |
| | Average value not less than | 140 kPa |
| | Minimum single value | 120 kPa |
| Note: | The average value shall be determined over any ten consecutive tests | |

In addition to the above, a higher specification was required for the fill embankments for the stormwater pond within the drainage reserve in Stage 13B1. This specification was:

- Maximum air voids of 6%
- Minimum undrained shear strength of 140kPa.

Where hardfill was used on site as part of bulk filling operations (within the toe key in the drainage reserve), a minimum Clegg Impact Value (CIV) of 20 was specified.

4.2.1 Compaction Assurance Testing

Regular insitu density, strength and water content tests were carried out on all areas of the filling at or in excess of the frequency recommended by NZS 4431, and a series of hand auger boreholes were also drilled at selected locations as an added check on quality control. The results of this testing (including testing some testing undertaken on adjacent stages of the subdivision) are appended in Appendix 2.



Control tests carried out on the filling showed that several occasions the required compaction standards were not being achieved. Results of the test failures were relayed to the site foreman and/or his staff, and the affected areas of fill were re-worked as necessary. In each case, further testing was carried out until compliance with the standards was achieved.

5 PROJECT EVALUATION

5.1 Bearing Capacity and Settlement of Building Foundations

Following the completion of earthworks operations, we returned to the site in October 2021 and drilled a series of hand auger boreholes in order to determine representative finished ground conditions and hence evaluate likely foundation options for future building development.

At current subgrade levels all filled and undisturbed natural ground has a geotechnical ultimate bearing capacity of 300 kPa within the influence of conventional shallow residential building foundation loads.

Where any building platforms have been rutted by heavy machinery subsequent to this report, or softened due to ponded rainwater, engineering advice should be sought with a view affected areas be trimmed back to competent ground and reinstated with compacted hardfill to design subgrade level prior to the commencement of building construction.

It should be noted that NZS 3604 only allows a maximum backfill depth of 600mm over the building platform of a dwelling unless an Engineering design solution or endorsement is proposed, on account of the risk of induced consolidation of the subsoils caused by the weight of the backfill.

5.2 Expansive Soils

Two sets of Atterberg Limit expansive soil tests and two Shrink-Swell Index tests were carried out on samples selected from around the site and within the zone of likely influence of shallow building foundations to inform the expansive Site Class for this stage of the subdivision.

The Atterberg Limit tests were carried out in accordance with NZS 4402, "Methods of Testing Soils for Civil Engineering Purposes" test section 2 and the Shrink-Swell Index tests were carried out in accordance with AS 1289, "Methods of Testing Soils for Engineering Purposes" test method 7.1.1. These tests were primarily intended to assess the Expansive Classes of the site materials as defined in AS 2870, "Residential Slabs and Footings – Construction" and MBIE Acceptable Solutions and Verification Methods amendment 19¹.

Based on the laboratory testing and visual tactile assessments of the soils observed in our postconstruction boreholes, the assessed expansive site class for all residential lots when assessed in accordance with MBIE (Acceptable Solution and Verification Methods Amendment 19) and AS2870:2011 is presented in Table 3 below.

¹ Ministry of Business, Innovation and Employment. Verification Methods and Acceptable Solutions Amendment 19 for NZ Building Code B1/AS1, Section 3 (as relevant to expansive soils and good ground). Effective 28 November 2019.



| Table 3. Expansive Site Class Summary | | | | | |
|---------------------------------------|--|--|--|--|--|
| Lot No. | MBIE ² Expansive Site Class | AS2870 Expansive Site Class | | | |
| 752-801, 804 and 807-809 | H (High); Characteristic ground movement up to 78mm. | H1 (High); Characteristic ground movement up to 60mm. | | | |
| 802, 803, 805 and 806 | S (Slight); Characteristic ground movement up to 22mm. | S (Slight); Characteristic ground movement up to 20mm. | | | |

Table 3. Expansive Site Class Summary

*Note: The AS2870:2011 assessment is based on the scaling factor of the site being adjusted to a 1/500yr event to meet the recommendations of MBIE.

Specific design alternatives for these Site Classes are presented in the Suitability Statement. These classifications may be re-addressed by end users during building consent if site specific laboratory shrink-swell testing is undertaken by end users, as recommended in the MBIE document attached.

For Class H soils, if slab on-grade floor slab construction takes place during a long dry summer, exposed building platform soils may dry put and become highly desiccated. Over time the rehydration of the soils below the floor slab can cause swelling and floor slab uplift. Floor slab uplift can cause distress of tile floors and in garages where cracks are more apparent. It may also rack upper storeys and/or rooflines if non-load bearing ground floor walls are lifted and act as struts. It is prudent to place hardfill immediately upon completion of subgrade trimming, followed by thorough soaking of the hardfill prior to concrete placement (e.g. for slab on-grade construction), all of which can help to limit the problem.

5.3 Lot Gradients

The appended as built contours plan shows areas having gradients steeper than 1(v) in 4(h) or being immediately adjacent to land having such gradients. The extent of these areas has been determined by the surveyed site gradients and our final walkover inspection, but there may be localised areas having such gradients that have not been shown on the plans.

A site-specific computer slope stability analysis was completed for the stormwater pond in Lot 11 (drainage reserve in adjacent Stage 13B1) as part of our GIR and global factors of safety were found to be satisfactory. Additionally, our observations during construction found that ground conditions were consistent with the design assumptions and the as-builts show no departure from the design.

We are satisfied that these lots are <u>not</u> subject to the hazards described in section 71(3) of the Building Act.

Details of resulting building and earthworks restrictions within the vicinity of these lots are presented in the Suitability Statement.

5.4 Fill Induced Settlement

As a result of our pre-fill inspections, the installation of subsoil drainage, quality control testing and the elapsed time since the placement of the majority of the filling (i.e. in excess of 12 months), we are of the

² Ministry of Business, Innovation and Employment. Verification Methods and Acceptable Solutions Amendment 19 for NZ Building Code B1/AS1, Section 3 (as relevant to expansive soils and good ground). Effective 28 November 2019.



opinion that induced differential settlements beneath or within the certified filling due to its imposed weight should be insignificant with respect to conventional NZS 3604 residential building development.

5.5 Vegetation Cover

Wherever practical on sloping land beyond building platform areas, all existing bush and grass cover should be maintained. Any vegetation or bush cleared beyond the immediate area of building platforms for temporary construction purposes should be replaced as soon as possible.

The contribution of appropriate vegetation cover to overall site stability and erosion control should not be underestimated.

5.6 Stormwater Controls

It is important on all sloping lots that due care is paid to the design and construction of appropriate stormwater disposal systems. These systems should serve to collect all runoff from roofs, decks and paved areas, together with discharges from retaining wall drains and other subsoil drains and should connect directly into the public stormwater drainage network.

Uncontrolled stormwater discharges onto the ground surface or into soakage pits can cause erosion, scour and/or instability on sloping land and should not be permitted under any circumstances where stability could be compromised.

5.7 Service Trenches

As is normal on all subdivisions, building developments involving foundations within a 45° zone of influence from pipe inverts will require Engineering input.

5.8 Underfill Drains

The appended fill as-built cut-fill contour plans show the alignments of perforated underfill drains that was placed in a mucked out gully invert prior to filling to tap groundwater seepages. The drains run beneath portions of the drainage reserve within Stage 13B1 as indicated on the As-Built Cut-Fill Contours Plan.

This drain was intended to intercept localised groundwater seepages during earthworks and/or allow engineered fill placement as required by the project specifications. The drain was installed as a precautionary measure, not as remedial works for any existing instability, and it requires no specific maintenance.

Notwithstanding, it is recommended that future site development works preserve this drain. In the event that the drain is compromised by any future development works, then the drain should be reinstated under geotechnical engineering observational guidance.

5.9 Stormwater Detention Pond

A new wet stormwater detention pond has been constructed in the vicinity of the drainage reserve in the southern corner of the subdivision (within Stage 13B1). Where fill has been placed to form the pond walls, this was required to meet a higher fill compaction criteria as outlined in Section 4.2.1.

The base of this pond encountered cohesive soils and therefore no clay liner was required. If/where underfill drains passed beneath the base of the pond, a minimum capping layer of 1m of engineer



certified clay was placed over the drain to help eliminate the possibility of future piping erosion and/or seepage from the base of the pond.

Global slope stability of the stormwater pond batters was found to be satisfactory as discussed in Section 5.3.

5.10 Topsoil

Topsoil depths in likely building platform areas were checked by the drilling of a borehole in the approximate centre of each of the lots. Our findings, which are indicative only and subject to variation at other locations, show that likely topsoil depths are between 100 mm and 250 mm, however, lots 798 contains no topsoil.

5.11 Contractor's Work

We have relied on the Contractor's work practices and assume that the works have been carried out in The approved Contract drawings and design details,

- (i) The approved Contract specifications,
- (ii) Authorised Variations to (i) and (ii) during the execution of the works,
- (iii) The conditions of Resource, Earthworks and Building Consents where applicable,
- (iv)The relevant Lander Geotechnical Consultants Limited reports, recommendations and site instructions,

and that all as-built information and other details provided to the Client and/or :DE Limited are accurate and correct in all respects.

6 STATEMENT OF PROFESSIONAL OPINION AS TO THE SUITABILITY OF LAND FOR BUILDING DEVELOPMENT

- I, S.G. Lander, of LDE Limited, Auckland, hereby confirm that:
- 1. I am a Chartered Professional Engineer experienced in the field of geotechnical engineering as defined in section 1.2.3 of NZS 4404 and was retained by the Owner/Developer as the Geotechnical Engineer on Stage 13B of the Hitchen Block residential subdivision.
- The extent of preliminary investigations carried out to date are described in Geotechnical Investigation Report reference J00741, dated 29 August 2018, and the conclusions and recommendations of that document have been re-evaluated in the preparation of this report. The results of all tests carried out under Lander Geotechnical Consultants Limited direction are appended.
- 3. In my professional opinion, not to be construed as a guarantee, I consider that:
 - (a) The earth fills shown on the appended fill as-built plan have been placed in compliance with NZS 4431 and related documents.
 - (b) The completed earthworks give due regard to land slope and foundation stability considerations within the residential lots, however, as shown on the appended contour as-built plan lots 755,



756, 758 to 761, 775, 776, 778 to 781, 795 to 798, and 801 to 809 have gradients steeper than 1(v) in 4(h) or are adjacent to land having such gradients.

Any building development and/or earthworks proposals within the areas shown to be steeper than 1(v) in 4(h) on the as-built contours plan are subject to specific geotechnical investigations and/or foundation design, and/ or adequately designed retaining walls and/ or batter slopes.

- (c) The function of the underfill drain beneath portions of the drainage reserve in Stage 13B1 should not be impaired by any future development or landscaping works.
- (d) A geotechnical ultimate bearing capacity of 300 kPa may be assumed for foundation design on all lots (except where specific geotechnical endorsement is required on account of sloping land greater than 1(v in 4(h)).

Where a geotechnical bearing capacity greater than 300 kPa is required, (i.e. outside the limits of NZS 3604, such as when piling is undertaken), further specific site investigation and design of foundations should be carried out prior to building consent application.

(e) The backfilling and compaction of the stormwater and sanitary sewer trenches on this subdivision has where possible been carried out to appropriate standards having regard for the prevailing ground conditions and associated compaction induced pipe loadings.

Nevertheless, no building development should take place within the 45° zone of influence of drain inverts unless endorsed by specific site investigations, foundation designs and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that lateral stability and differential settlement issues are addressed and that building loads are transferred beyond the influence of the pipe and beyond the extent of the trench backfill.

(f) The assessed Expansive Site Class for residential lots 802, 803, 805 and 806 is Class S (slight – with characteristic ground surface movement of up to 22mm) in terms of MBIE Acceptable Solutions and Verification Methods Amendment 19 and Class S (slight – with characteristic ground surface movement of up to 20mm) in terms of AS2870:2011.

The assessed Expansive Site Class for residential lots 752 to 801, 804 and 807 to 809 is Class H (high – with characteristic ground surface movement of up to 78mm) in terms of MBIE Acceptable Solutions and Verification Methods Amendment 19 and Class H1 (high – with characteristic ground surface movement of up to 60mm) in terms of AS2870:2011.

Site specific laboratory Shrink-Swell testing and calculation of specific ys values may be undertaken by end-users to re-assess this during building consent stage.

- (g) Subject to the geotechnical limitations, restrictions, recommendations and expansive soil assessments associated with 3(b) to 3(f) above:
 - (i) The filled and undisturbed original ground within residential lot boundaries is generally suitable for residential buildings constructed in accordance with NZS 3604 and related documents.
 - (ii) On residential lots 802, 803, 805 and 806, foundation design may be carried out in accordance with one of the following methods:



- Class S in terms of MBIE Acceptable Solutions and Verifications Methods for NZ Building Code Clause B1 Structure, effective 28 November 2019;
- Class S in terms of AS2870:2011;
- A specific foundation and structural design may be undertaken by a Chartered Professional Engineer who should allow for expansive soil effects referenced above in the design.
- For buildings having brittle exterior cladding appropriate control joints should also be specifically designed depending on architectural specifications and structural form.

(iii) On residential lots 752 to 801, 804 and 807 to 809, foundation design may be carried out in accordance with one of the following methods:

- Class H in terms of MBIE Acceptable Solutions and Verifications Methods for NZ Building Code Clause B1 Structure, effective 28 November 2019;
- Class H1 in terms of AS2870:2011;
- A specific foundation and structural design may be undertaken by a Chartered Professional Engineer who should allow for expansive soil effects referenced above in the design.
- For buildings having brittle exterior cladding appropriate control joints should also be specifically designed depending on architectural specifications and structural form.
- Road subgrades and lot accessway subgrades have been formed having due regard for slope stability and settlement, although CBR values do vary between natural and filled ground as is to be expected.
- Geotechnical aspects of slope stability and pond permeability within the drainage reserve in stage 13B1 have been appropriately addressed and in these respects the pond is suitable for its intended use.

The professional opinion contained within this report is furnished to the Waikato District Council and DFH Joint Venture Limited for their purposes alone, with respect to the particular brief given to us. It may not be relied upon in any other context of for any other purpose without our prior review and agreement. It does not remove the necessity for the normal inspection of site conditions at the time of erection of any dwelling.

The appended table summarises the status of each residential lot covered by this Suitability Statement.

For and on behalf of LDE Limited

Prepared by:

K.moro

K. Meffan Engineering Geologist MEngNZ

Reviewed By:

C.J. Edwards Senior Engineering Geologist CMEngNZ (PEngGeol)

Authorised by:

111 1.

S.G. Lander Principal Geotechnical Engineer CMEngNZ, CPEng, IntPE(NZ)



| Table 4. | Suitability | / Statement | Summary |
|----------|-------------|-------------|---------|
| | Suitability | Julement | Summary |

| Lot No. | Comments | Topsoil Depth | Ultimate Bearing |
|------------|---|------------------|---------------------|
| | | (mm) | (kPa) |
| 752 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 753 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 754 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 100 | 300 |
| 755 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. | 200 | 300 |
| | Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | |
| 750 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to $1(v)$ in $4(h)$ gradient restrictions. | 100 | 300 |
| 756 | Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | | |
| 757 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to $1(v)$ in $4(h)$ gradient restrictions. | | |
| 758 | Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 150 | 300 |



| Lot No. | Comments | Topsoil Depth (mm) | Ultimate Bearing (kPa) |
|------------|--|--------------------------|------------------------------|
| 759 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 150 | 300 |
| 760 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 761 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 762 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 763 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 764 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 765 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 250 | 300 |
| 766 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |



| Lot No. | Comments | Topsoil Depth (mm) | Ultimate Bearing (kPa) |
|------------|---|--------------------------|------------------------------|
| 767 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 768 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 769 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 300 | 300 |
| 770 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 771 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 150 | 300 |
| 772 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 773 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 774 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 775 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 150 | 300 |



| Lot No. | Comments | Topsoil Depth (mm) | Ultimate Bearing (kPa) |
|------------|--|--------------------------|------------------------------|
| 776 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 777 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 778 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 779 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 780 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 150 | 300 |
| 781 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |



| Lot No. | Comments | Topsoil Depth (mm) | Ultimate Bearing (kPa) |
|------------|--|--------------------------|------------------------------|
| 782 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 250 | 300 |
| 783 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 784 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 785 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 100 | 300 |
| 786 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 787 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 788 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 789 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 150 | 300 |
| 790 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 791 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 100 | 300 |
| 792 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |



| Lot No. | Comments | Topsoil Depth (mm) | Ultimate Bearing (kPa) |
|------------|--|--------------------------|------------------------------|
| 793 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 150 | 300 |
| 794 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design. | 200 | 300 |
| 795 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 796 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 797 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 798 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 0 | 300 |
| 799 | Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 800 | Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing | 200 | 300 |



| Lot No. | Comments | Topsoil Depth (mm) | Ultimate Bearing (kPa) |
|------------|--|--------------------------|------------------------------|
| | depth 900mm or an engineer approved alternative foundation design. | | |
| 801 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 802 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 <u>or</u> AS2870:2011 Class S <u>or</u> NZS 3604 with minimum footing depth 450mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 803 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 or AS2870:2011 Class S or NZS 3604 with minimum footing depth 450mm or an engineer approved alternative foundation design. | 200 | 300 |
| 804 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 200 | 300 |
| 805 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 <u>or</u> AS2870:2011 Class S <u>or</u> NZS 3604 with minimum footing depth 450mm <u>or</u> an engineer approved alternative foundation design. | 100 | 300 |
| 806 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. | 200 | 300 |

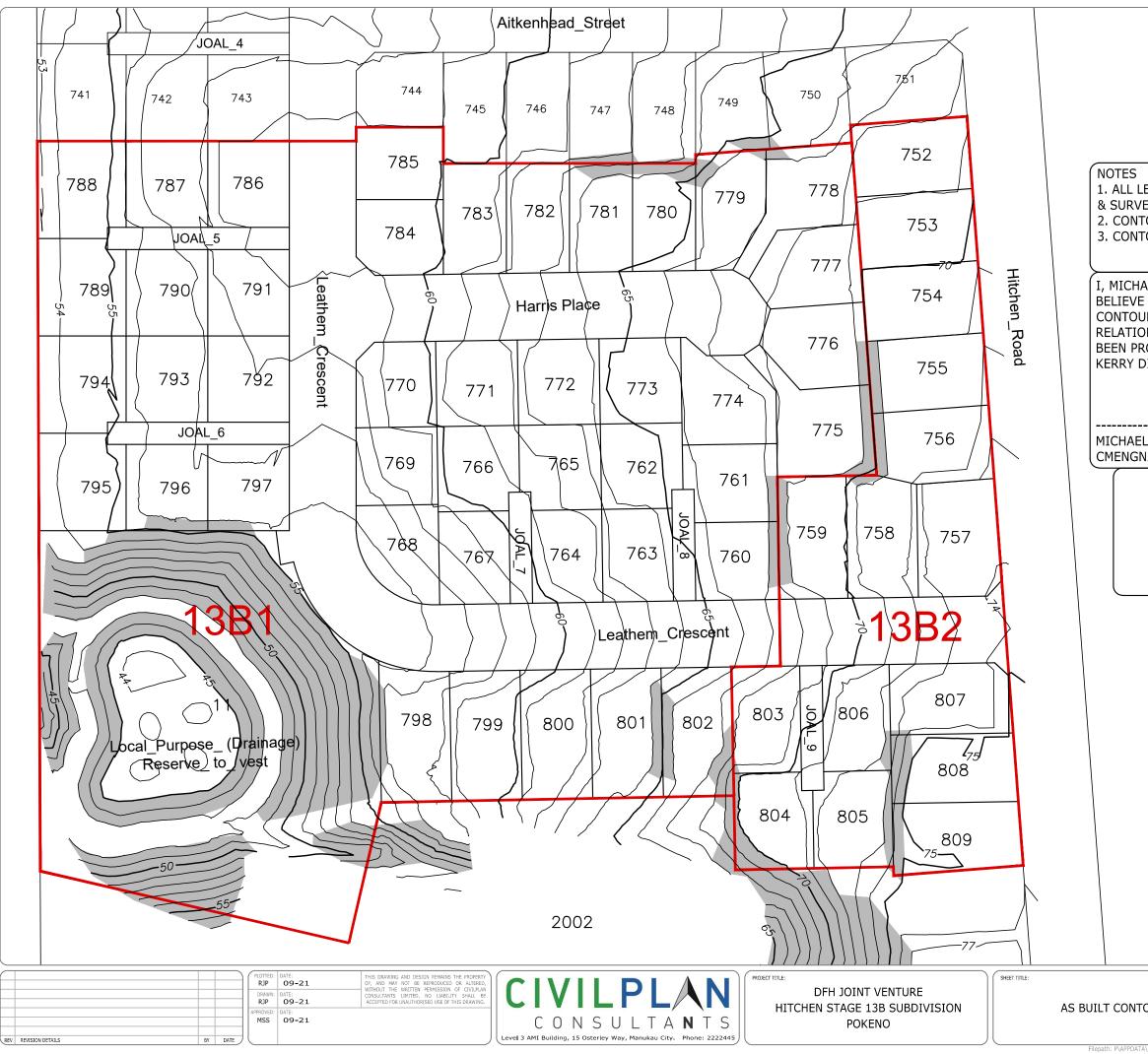


| Lot No. | Comments | Topsoil Depth (mm) | Ultimate Bearing (kPa) |
|------------|---|--------------------------|------------------------------|
| | Elsewhere, foundation design in accordance with either MBIE B1/AS1 <u>or</u> AS2870:2011 Class S <u>or</u> NZS 3604 with minimum footing depth 450mm <u>or</u> an engineer approved alternative foundation design. | | |
| 807 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 100 | 300 |
| 808 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 300 | 300 |
| 809 | Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design. | 300 | 300 |



Appendix 1

CivilPlan Consultants Limited As-Built Plans



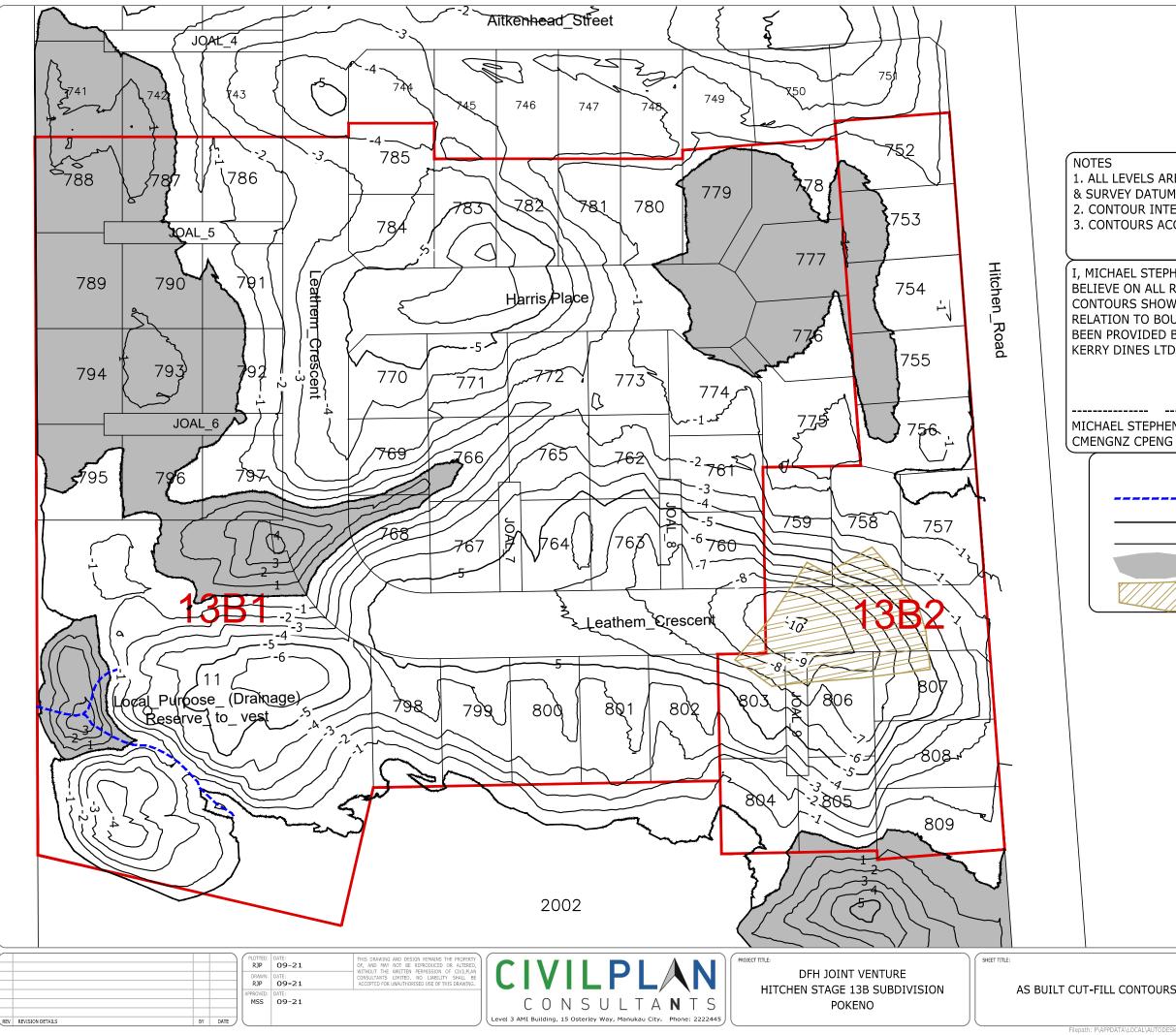


 ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS & SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND 1945.
 CONTOUR INTERVAL IS 1m
 CONTOURS ACCURACY IS + OR - 0.5m

| AEL STEPHEN SMITH, CMENGNZ CPENG HEREBY |
|--|
| ON ALL REASONABLE GROUNDS THAT THE |
| IRS SHOWN ARE ACCURATELY PLOTTED IN |
| IN TO BOUNDARIES AS THIS INFORMATION HAS |
| OVIDED BY THE CONTRACTOR OF THE WORKS - |
| DINES LTD. |

| _ STEPHEN SMITH IZ CPENG | DATE |
|-----------------------------|---|
| | LEGEND |
| | MAJOR 5m CONTOUR MINOR 1m CONTOUR LAND STEEPER THAN 1:4 |

| | ISSUE STATUS: | | | | AS B | UILT |
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| | DRAWING NUMBER | 12670 |)1-13 | B-AB | 200rev | Α |
| \LOCAL\AUTODESK\C3D 2020\EN | J\TEMPLATE C:\DATA | A\POKENO\HIT(| CHEN\STAGE | 13\AS BUILTS | S\136701-13B | AB200.DWG |





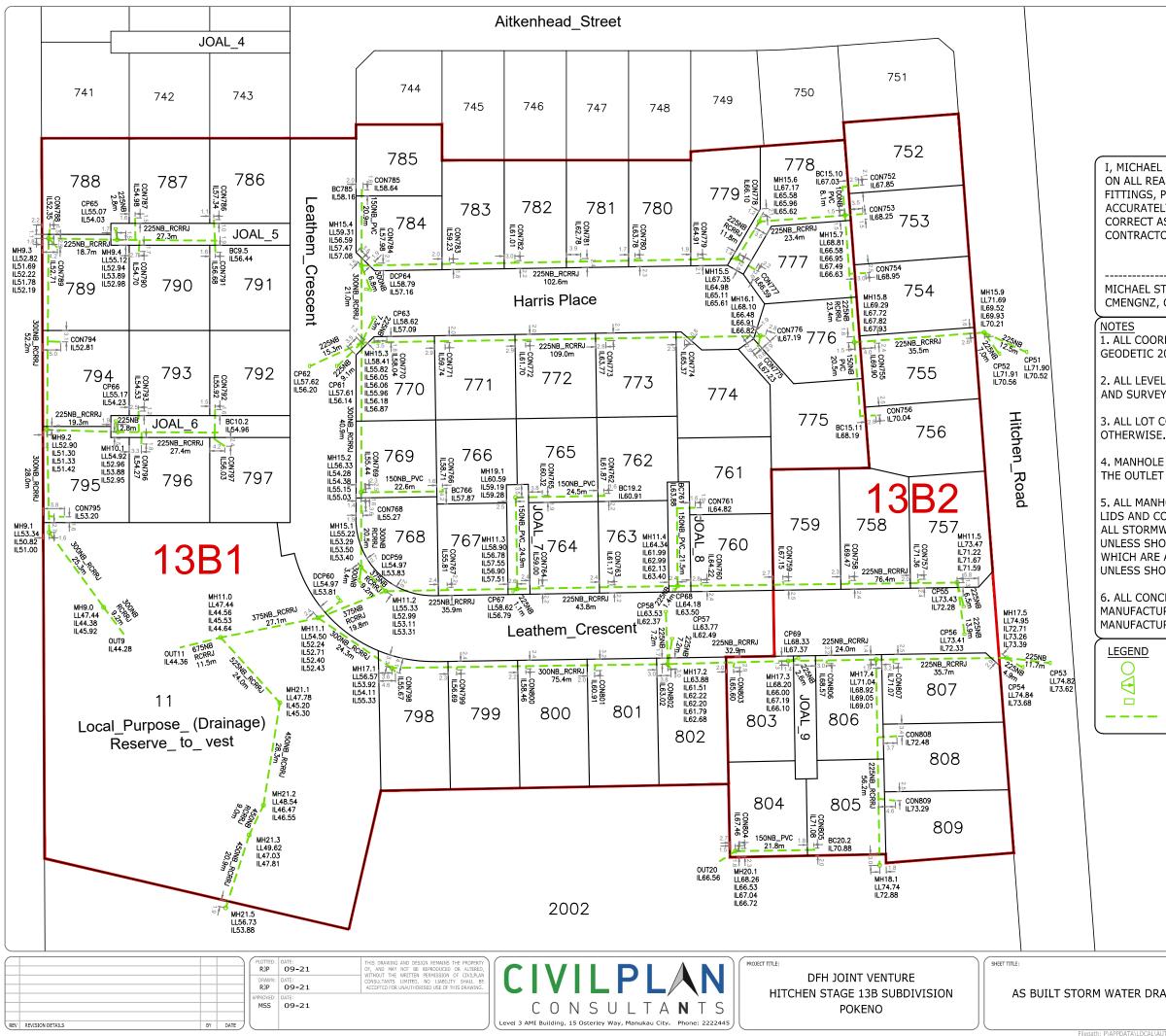
1. ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS & SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND 1945. 2. CONTOUR INTERVAL IS 1m 3. CONTOURS ACCURACY IS + OR - 0.5m

I, MICHAEL STEPHEN SMITH, CMENGNZ CPENG HEREBY BELIEVE ON ALL REASONABLE GROUNDS THAT THE CONTOURS SHOWN ARE ACCURATELY PLOTTED IN RELATION TO BOUNDARIES AS THIS INFORMATION HAS BEEN PROVIDED BY THE CONTRACTOR OF THE WORKS -KERRY DINES LTD.

_____ MICHAEL STEPHEN SMITH DATE

| LEGEND |
|----------------------|
| UNDERFILL DRAIN |
| MAJOR 5m CONTOUR |
| MINOR 1m CONTOUR |
| FILL AREAS |
| TUFF UNDERCUT AREA |
| TOTT UNDERCOT AREA |

| | ISSUE STATUS: | | | | AS B | UILT |
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I, MICHAEL STEPHEN SMITH, CMENGNZ CPENG HEREBY BELIEVE ON ALL REASONABLE GROUNDS THAT THE PIPE SIZES, FITTINGS, POSITIONS, COORDINATES AND LEVELS SHOWN ARE ACCURATELY PLOTTED IN RELATION TO BOUNDARIES AND IS CORRECT AS THIS INFORMATION HAS BEEN PROVIDED BY THE CONTRACTOR OF THE WORKS - KERRY DINES LTD.

MICHAEL STEPHEN SMITH DATE CMENGNZ, CPENG

1. ALL COORDINATES ARE IN METRES AND IN TERMS OF GEODETIC 2000 MT. EDEN

2. ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS AND SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND 1946

3. ALL LOT CONNECTIONS ARE 100NB PVC UNLESS SHOWN OTHERWISE.

4. MANHOLE INVERT LEVELS ARE LISTED CLOCKWISE FROM THE OUTLET PIPE.

5. ALL MANHOLES ARE 1050NB WITH STANDARD DUTY LIDS AND COVERS UNLESS SHOWN OTHERWISE. ALL STORMWATER PIPES 225NB AND OVER ARE RCRRJ CLASS 2 UNLESS SHOWN OTHERWISE EXCEPT FOR CATCHPIT LEADS WHICH ARE ALL CLASS 4. ALL CATCHPIT LEADS ARE 225NB UNLESS SHOWN OTHERWISE.

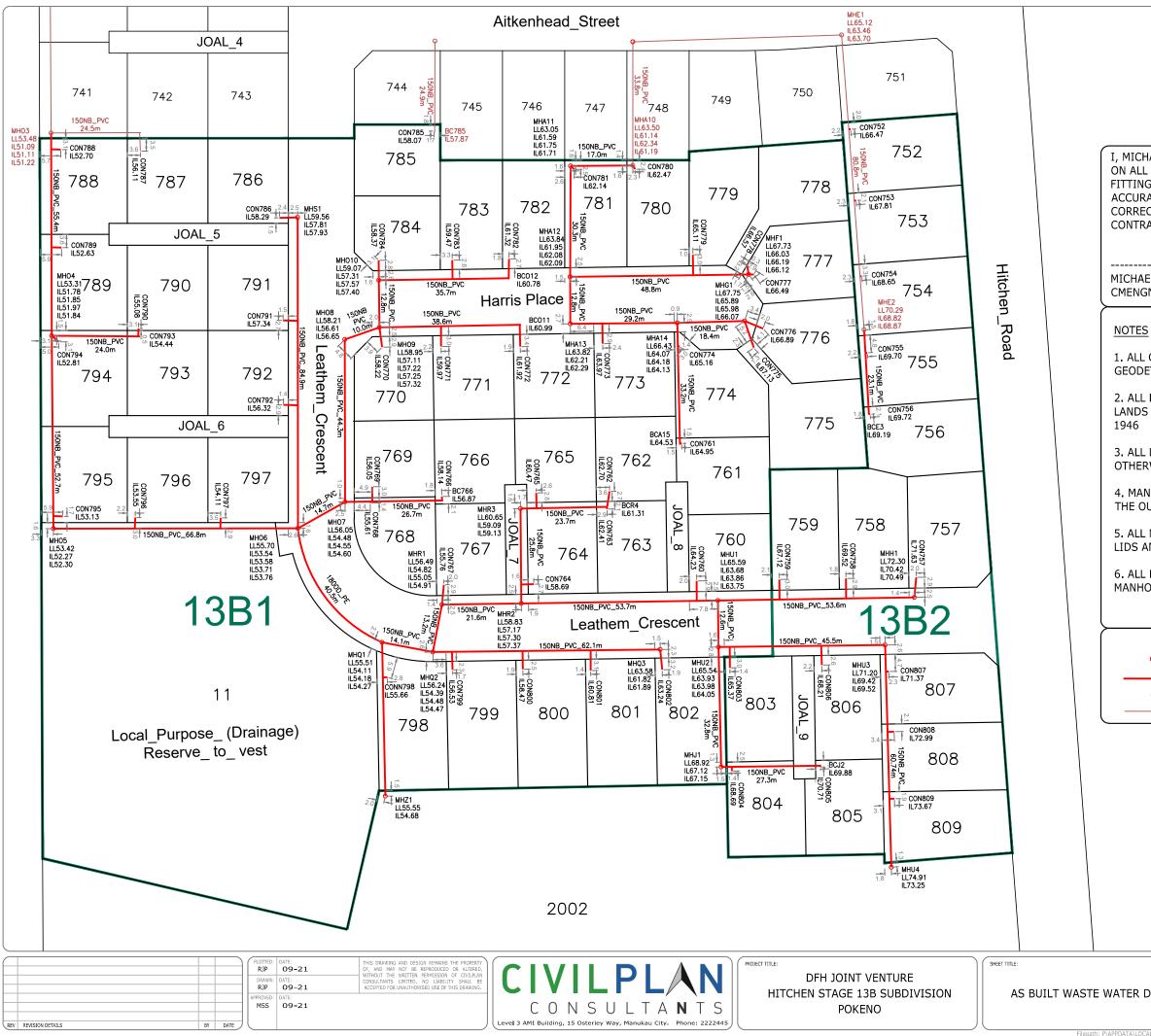
6. ALL CONCRETE PIPES AND STRUCTURES ARE MANUFACTURED BY HYNDS PIPES. ALL PLASTIC PIPES ARE MANUFACTURED BY MARLEY.

 \bigcirc

|) | MANHOLE |
|---|----------|
| 1 | OUTLET |
| | CATCHPIT |
| | SW LINE |
| | |

| EX.MANHOLE |
|----------------|
| EX.CATCHPIT |
| EX.SW LINE |
| |

| | ISSUE STATUS: | | | | AS B | UILT |
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I, MICHAEL STEPHEN SMITH, CMENGNZ CPENG HEREBY BELIEVE ON ALL REASONABLE GROUNDS THAT THE PIPE SIZES, FITTINGS, POSITIONS, COORDINATES AND LEVELS SHOWN ARE ACCURATELY PLOTTED IN RELATION TO BOUNDARIES AND IS CORRECT AS THIS INFORMATION HAS BEEN PROVIDED BY THE CONTRACTOR OF THE WORKS - KERRY DINES LTD.

_____ _ DATE MICHAEL STEPHEN SMITH CMENGNZ, CPENG

1. ALL COORDINATES ARE IN METRES AND IN TERMS OF GEODETIC 2000 MT. EDEN

2. ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS AND SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND

3. ALL LOT CONNECTIONS ARE 100NB PVC UNLESS SHOWN OTHERWISE.

4. MANHOLE INVERT LEVELS ARE LISTED CLOCKWISE FROM THE OUTLET PIPE.

5. ALL MANHOLES ARE 1050NB WITH STANDARD DUTY LIDS AND COVERS UNLESS SHOWN OTHERWISE.

6. ALL PIPES ARE MANUFACTURED BY MARLEY AND MANHOLES BY HYNDS PIPES.

| LEGEND | |
|--------|---------|
| 0 | MANHOLE |
| | WW LINE |
| 0 | EX.MANH |

| MANHOLL |
|------------|
| WW LINE |
| EX.MANHOLE |
| EX.WW LINE |

| | ISSUE STATUS: | | | | AS B | UILT |
|-----------------------------|--|-------------|---------------|---------|------|------|
| R DRAINAGE | SCALE: (A1/A3) SCALE BAR 0 1:1000@A3 | 1:500 10 | / 1:100 20 | 0 30 | 40 | 50m |
| \LOCAL\AUTODESK\C3D 2020\EN | DRAWING NUMBER | | | | | |



Appendix 2

Field Density Test Summary Sheets



Our Ref: 1009213.0657.0.0/3 Customer Ref: J00113 02 August 2021

Lander Geotechnical Itd PO Box 97 385 Manukau Auckland 2241

Attention: Shane Lander

Dear Shane

Hitchen Block, Pokeno - Stage 5

Site Report

Customer's Instructions

We were instructed to:

Complete nuclear densometer, shear vane and impact hammer testing at the above-mentioned site when requested and report the results.

Specifications

As per email from Mike (Lander Geotechnical) on the 08/01/2019 the specification for Earthwork's testing was as follows;

- Average air voids <10% with maximum air voids 12%.
- Average shear strength >140 kPa with minimum single value 120 kPa.
- Average value to be determined over 10 consecutive tests.

As per email from Mike (Lander Geotechnical) on 21/10/2019 the Earthworks specification for Pond Embankment areas was as follows;

- Average air voids <6% with maximum air voids 8%.
- Average shear strength >140 kPa with minimum single value 120 kPa.

As requested by Kyle (Lander Geotechnical) via email; on the 09/12/2020 the specification for hardfill testing required a minimum single density of 95% of the maximum dry density (MDD) and an impact hammer value of ≥ 20 .

As confirmed by Kyle (Lander Geotechnical) via email on the 09/12/2020 an MDD of 2.09 t/m³ was to be used for the material used on site. The MDD was determined by the Geotechnics plateau test carried out on the 12/02/2020 (Refer to Appendix C).

| Material Type | Maximum Dry Density t/m ³ | Optimum Water Content % | Solid Density Assumed t/m ³ | Report Reference Number/Supplier |
|---------------|--|-------------------------------|--|---|
| SPR | 2.09 | _ | - | Plateau Density Test 12/02/2020 (URN 33) |

Laboratory Determined Parameters of Material

Dates of Procedures

Testing was carried out from the 17/10/2019 to 10/05/2021.

Locations

Testing was carried out as instructed by the contractor on site. Individual test locations were selected on site by the Geotechnics technician on behalf of the customer. No formal testing frequency was provided by the customer.

The attached plans provide indicative locations only and are not to scale. All other information we provide regarding location should be referenced to the asset owner.

Samples

Samples taken for moisture content verification purposes were disposed of 24 hours after testing.

Methods

NZGS 8:2001 - Test method for determining the vane shear strength of a cohesive soil using a hand held shear vane.

NZS 4407:2015 Test 4.2 - Method using a nuclear surface moisture-density gauge (Direct Transmission Mode) – NDM

NZS 4407:2015 Test 3.1 - Determination of water content

NZS 4407:2015 Test 4.3 Method using a nuclear surface moisture density-gauge (Backscatter mode) - NDM

ASTM D 5874-16 - Standard test method for determination of the impact value (IV) of a soil – Impact Test

Material Description

Material descriptions are provided in the attached results. All descriptions were provided by the customer.

Results

The following is attached:

Appendix A - Earthworks testing results.

Appendix B - Hardfill testing results.

Appendix C - Plateau density test.

Appendix D- Test location plans.

Test Remarks

Shear Vane

Shear Vane tests are potentially unsuitable for material described in the Earthworks summary as 'Clay SILT with Gravels, 'Sandy SILT with Clay and Gravels', 'Clay SILT with Sand'. Tests in these materials may not be compliant with the stated test method and results are therefore not covered under the IANZ endorsement of this report. Results are provided for your own interpretation and inference.

NDM – Direct Transmission

The test method may not be appropriate for materials containing a nominal maximum particle size of >40 mm.

Nuclear densometers are calibrated for a bulk density range of 1,728 kg/m³ to 2,756 kg/m³. Test results outside of these bulk density limits are not covered under the IANZ endorsement of this report.

An assumed solid density value of 2.70 t/m³ was agreed with the customer. We do not take responsibility for misrepresentation or misinterpretation arising from the use of this assumed value to calculate air voids.

Where oven calculated air voids are negatives, these have been reported as zero.

The calculation of air voids is based on wet density (measured by the nuclear densometer), moisture content (measured by oven drying) and solid density (either assumed or measured by laboratory testing). Negative air voids may be caused by incorrect assumed solid density or due to the variability of onsite material when compared to that tested in a laboratory.

Determination of Water Content

Samples used for the determination of the water content were sampled in conjunction with nuclear densometer testing and disposed of after 24 hours.

NDM – Backscatter

The test method may not be appropriate for materials containing a nominal maximum particle size of >40 mm.

The wet density and moisture content were measured by the nuclear densometer.

The calculation of percentage compaction is obtained from NDM density values and customer provided maximum dry density (MDD) target.

Impact Value

Field procedure A was used. The test method is appropriate for materials having a maximum particle size of less than 37.5 mm. Results of tests that were performed on materials containing larger particle sizes, are not covered under the IANZ endorsement of this report.

Calculations of equivalent CBR are based on CBR = $IV^2x0.07$. This calculation is not covered under the IANZ endorsement of this report.

Pass/Fail Criteria

We accept no liability for any circumstances that may arise due to the inclusion of the pass/fail criteria or the use of this information by third parties. Pass/fail criteria are based solely on numerical values with no consideration given to uncertainty and are not covered under the IANZ endorsement of these results.

General Remarks

This report has been prepared for the benefit of Lander Geotechnical, with respect to the particular brief given to us and it cannot be relied upon in other contexts or for any other purpose without our prior review and agreement.

The inherent uncertainties of site investigation work, mean the nature and continuity of subsoil away from the test location could vary from the data logged.

Material descriptions are included for information only and are not covered under the IANZ endorsement of this report.

Sample(s) not destroyed during testing will be retained for one month from the date of this report before being discarded.

Please reproduce this report in full when transmitting to others or including in internal reports.

If we can be of any further assistance, feel free to get in touch. Contact details are provided at the bottom of the letterhead page.

GEOTECHNICS LTD

Report prepared by:

David Sayers CMT Field Technician

Authorised for Geotechnics by:

Steven Anderson Project Director Approved Signatory

Report checked by:

Daniel Brasting Project Manager

REDITED

Test results indicated as not accredited are outside the scope of the laboratory's accreditation

3-Aug-21

\\ttgroup.local\corporate\GeotechnicsGroup\Projects\1009213\1009213.0657 - Hitchen Block, Pokeno\IssuedDocuments\Stage 5 Report\20210728 Hitchen block, Pokeno Stage 5 Site Report.docx



Client: Lander Geotechnical Ltd.

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/SEBA/FRHA |
| Checked By | JRA/DASA/SEBA |
| Approved By | SJA |

| | | | | | | Test Type | | NDM 00 | | | NDM 90 ⁰ | | A | VERAGE ND | м | Solid | _ | Final Co | rrected | | | s | hear Vane | Reading (I | kPa) | | | PASS / FAIL | |
|------|-------|------------|----------------------------------|--------------------------------|---------------------|-----------|--------------------------------|----------------|-----------|--------------------------------|---------------------|-----------|--------------------------------|----------------|-----------|--------------------------------|---------------------------------|--------------------------------|------------------|--------------------------------------|--------|-----------|-----------|------------|-------------------|-------------------|------------|-------------|---|
| URN | Tech. | Date | Location | Layer | Material | | Wet | Moisture | Air Voids | Wet | Moisture | Air Voids | Wet | Moisture | Air Voids | Density (t/m ³) | Oven Moisture content (%) | Oven Dry | | Average Air Voids (10 X Tests) | Readin | g Reading | Reading | Reading | Average | Average | Retest URN | (P) Pass | Comments |
| | | | | | | NDM / SV | Density (t/m ³) | Content (%) | (%) | Density (t/m ³) | Content (%) | (%) | Density (t/m ³) | Content (%) | (%) | Assumed | content (70) | Density (t/m ³) | Air Voids (%) | (10 x 10303) | 1 | 2 | 3 | 4 | SV (4 x Tests) | SV (10 X Sets) | | (F) Fail | 1 |
| 3.1 | JRA | 21/10/2019 | Fill F - Pond Area | ~0.5m Above | Clay SILT | NDM / SV | 1.74 | 45.3 | 1.6 | 1.75 | 42.7 | 1.9 | 1.75 | 44.0 | 1.8 | 2.70 | 41.2 | 1.24 | 3.3 | - | 124 | 121 | 121 | 124 | 123 | - | - | Р | |
| 3.2 | 5101 | 11/10/2015 | Thirt Fold Area | Underfill Drain | city sici | NDM / SV | 1.80 | 40.6 | 0.6 | 1.79 | 40.1 | 1.2 | 1.80 | 40.4 | 0.9 | 2.70 | 35.8 | 1.32 | 3.6 | - | 135 | 124 | 145 | 161 | 141 | - | - | Р | |
| 4.1 | JRA | 30/10/2019 | Fill F - Pond Area | ~1m below FL | Clay SILT | NDM / SV | 1.82 | 37.2 | 1.5 | 1.82 | 36.9 | 1.7 | 1.82 | 37.1 | 1.6 | 2.70 | 36.1 | 1.34 | 2.2 | - | 150 | 148 | 150 | 145 | 148 | - | - | Р | |
| 4.2 | | | | ~FL | | NDM / SV | 1.82 | 32.4 | 4.5 | 1.82 | 32.1 | 4.7 | 1.82 | 32.3 | 4.6 | 2.70 | 32.0 | 1.38 | 4.7 | - | 185 | 185 | 174 | 142 | 172 | - | | Р | |
| 5.1 | JRA | 1/11/2019 | Fill E - See Site Plan | ~0.5 - 1m placed | Clay SILT | SV | - | - | - | - | - | - | - | - | - | - | | - | - | - | 87 | 95 | 113 | 106 | 100 | - | 6.1 | F | |
| 5.2 | | | Pidli | | | SV | - | - | - | - | - | - | - | - | - | - | | - | - | - | 103 | 119 | 95 | 79 | 99 | - | 6.2 | F | |
| 6.1 | JRA | 2/11/2019 | Fill E - See Site Plan | ~0.5 - 1m placed | Clay SILT w gravels | NDM / SV | 1.89 | 28.6 | 3.5 | 1.90 | 27.3 | 4.1 | 1.89 | 28.0 | 3.8 | 2.70 | 30.5 | 1.45 | 2.0 | - | >191 | >191 | >191 | >191 | >191 | - | - | Р | Retest of URN 5.1 |
| 6.2 | | | Fidit | | | NDM / SV | 1.84 | 32.0 | 3.9 | 1.83 | 32.3 | 4.0 | 1.84 | 32.2 | 3.9 | 2.70 | 34.1 | 1.37 | 2.7 | • | >191 | 174 | 180 | >191 | >184 | - | - | Р | Retest of URN 5.2 |
| 7.1 | JRA | 4/11/2019 | Fill E - See Site Plan | ~RL 54.70 | Clay SILT | NDM / SV | 1.88 | 34.1 | 0.2 | 1.89 | 35.2 | 0.0 | 1.88 | 34.7 | 0.0 | 2.70 | 35.3 | 1.39 | 0.0 | - | >191 | 161 | 166 | 177 | 174 | - | | Р | |
| 7.2 | | | Pidii | ~RL 55.20 | | NDM / SV | 1.73 | 42.1 | 3.7 | 1.73 | 41.3 | 3.9 | 1.73 | 41.7 | 3.8 | 2.70 | 44.5 | 1.20 | 2.3 | - | 147 | 155 | >191 | 150 | >160 | 149 | - | Р | |
| 8.1 | JRA | 7/11/2019 | Fill E - See Site Plan | ~RL 55.50 | Clay SILT | NDM / SV | 1.78 | 35.9 | 4.3 | 1.78 | 35.5 | 4.7 | 1.78 | 35.7 | 4.5 | 2.70 | 30.9 | 1.36 | 7.6 | - | >191 | 122 | 133 | 150 | >149 | 151 | - | Р | |
| 8.2 | | | r (dl) | ~RL 57.00 | | NDM / SV | 1.83 | 36.0 | 1.9 | 1.82 | 38.6 | 0.7 | 1.82 | 37.3 | 1.3 | 2.70 | 33.0 | 1.37 | 4.0 | 3.2 | 125 | 133 | >191 | 139 | >147 | 152. | - | Р | |
| 9.1 | JRA | 13/11/2019 | Fill K - Pond Bund Embankment | ~0.5m Below FL of Pond Bund | Clay SILT | NDM / SV | 1.76 | 35.6 | 5.9 | 1.76 | 33.3 | 7.1 | 1.76 | 34.5 | 6.5 | 2.70 | 34.3 | 1.31 | 6.6 | 3.6 | >191 | >191 | >191 | >191 | >191 | 157 | - | Р | Air Void Average for 2 x tests @ Pond Embankment = 5.6% |
| 9.2 | | | Embankment | or Pond Bund | | NDM / SV | 1.80 | 36.3 | 3.3 | 1.80 | 36.5 | 3.1 | 1.80 | 36.4 | 3.2 | 2.70 | 34.1 | 1.34 | 4.6 | 3.7 | 152 | 169 | 161 | 150 | 158 | 155 | - | Р | |
| 11.1 | JRA | 22/11/2019 | Fill E - See Site Plan | ~1m Below FL | Clay SILT | NDM / SV | 1.85 | 32.2 | 3.3 | 1.85 | 32.3 | 2.8 | 1.85 | 32.3 | 3.1 | 2.70 | 30.4 | 1.42 | 4.3 | 3.9 | 171 | >187 | >187 | >187 | 183 | 164 | | Р | |
| 11.2 | | | Fidit | | | NDM / SV | 1.87 | 28.9 | 4.2 | 1.87 | 27.7 | 5.0 | 1.87 | 28.3 | 4.6 | 2.70 | 23.6 | 1.52 | 8.1 | 4.2 | 128 | 147 | 166 | 144 | 146 | 168 | | Р | |
| 12.1 | JRA | 25/11/2019 | Fill E - See Site Plan | ~0.5m Below FL | Clay SILT | NDM / SV | 1.84 | 30.6 | 4.9 | 1.84 | 29.1 | 5.9 | 1.84 | 29.9 | 5.4 | 2.70 | 34.9 | 1.36 | 2.0 | 4.2 | >187 | >187 | >187 | >187 | >187 | 168 | | Р | |
| 12.2 | | _ | Pidli | | | NDM / SV | 1.87 | 30.8 | 3.1 | 1.87 | 29.7 | 4.0 | 1.87 | 30.3 | 3.5 | 2.70 | 29.8 | 1.44 | 3.8 | 4.3 | >187 | >187 | >187 | >187 | >187 | 168 | - | Р | |
| 13.1 | | | Fill E - See Site Plan | ~FL | Clay SILT | NDM / SV | 1.93 | 31.7 | 0.0 | 1.93 | 32.6 | 0.0 | 1.93 | 32.2 | 0.0 | 2.70 | 31.3 | 1.47 | 0.0 | 4.3 | >187 | >187 | >187 | >187 | >187 | 170 | | Р | |
| 13.2 | JRA | 27/11/2019 | Fidit | | | NDM / SV | 1.88 | 32.5 | 1.5 | 1.88 | 32.9 | 1.2 | 1.88 | 32.7 | 1.3 | 2.70 | 28.4 | 1.46 | 4.3 | 4.5 | >187 | >187 | >187 | >187 | >187 | 172 | | Р | |
| 13.3 | | | Fill O - Pond Bund Embankment | ~1m Below FL | Clay SILT w Gravels | NDM / SV | 1.86 | 31.4 | 3.0 | 1.86 | 31.6 | 3.1 | 1.86 | 31.5 | 3.1 | 2.70 | 25.4 | 1.48 | 7.4 | 4.5 | 131 | 176 | 158 | 160 | 156 | 173 | • | Р | |
| 13.4 | | _ | embankment | | | NDM / SV | 1.78 | 37.4 | 3.5 | 1.78 | 34.6 | 5.1 | 1.78 | 36.0 | 4.3 | 2.70 | 32.8 | 1.34 | 6.3 | 4.7 | 176 | >187 | 150 | 142 | 164 | 175 | | Р | |
| 14.1 | JRA | 29/11/2019 | Fill O - Pond Bund Embankment | ~FL | Clay SILT | NDM / SV | 1.72 | 34.6 | 8.6 | 1.81 | 34.6 | 3.7 | 1.76 | 34.6 | 6.1 | 2.70 | 33.9 | 1.32 | 6.6 | 4.7 | >187 | >187 | >187 | >187 | >187 | 174 | - | Р | |
| 14.2 | | _ | Embankment | | | NDM / SV | 1.82 | 34.9 | 2.8 | 1.82 | 34.7 | 3.1 | 1.82 | 34.8 | 2.9 | 2.70 | 33.5 | 1.36 | 3.7 | 4.7 | >187 | 171 | 182 | >187 | 182 | 177 | - | Р | |
| 15.1 | - | | Fill H - See Site Plan | | | NDM / SV | 1.76 | 40.7 | 2.7 | 1.76 | 39.6 | 3.3 | 1.76 | 40.2 | 3.0 | 2.70 | 43.2 | 1.23 | 1.3 | 4.4 | >187 | >187 | >187 | >187 | >187 | 177 | - | Р | |
| 15.2 | JRA | 2/12/2019 | Pidli | ~FL | Clay SILT | NDM / SV | 1.76 | 39.7 | 3.2 | 1.75 | 40.3 | 3.5 | 1.76 | 40.0 | 3.4 | 2.70 | 39.8 | 1.26 | 3.5 | 3.9 | >187 | >187 | >187 | >187 | >187 | 181 | - | Р | |
| 15.3 | 4 | | Fill O - Pond Floor Area | | | NDM / SV | 1.81 | 35.0 | 3.4 | 1.81 | 34.6 | 3.5 | 1.81 | 34.8 | 3.5 | 2.70 | 36.1 | 1.33 | 2.7 | 4.0 | 123 | 147 | 163 | 139 | 143 | 177 | • | Р | |
| 15.4 | | | Area | | | NDM / SV | 1.79 | 34.5 | 4.8 | 1.79 | 35.5 | 4.3 | 1.79 | 35.0 | 4.6 | 2.70 | 33.7 | 1.34 | 5.4 | 4.1 | 150 | 136 | 139 | 131 | 139 | 172 | • | Р | |
| 16.1 | FRHA | 13/12/2019 | Fill A - See site plan | ~3m placed | Clay SILT w gravels | NDM / SV | 1.87 | 32.3 | 2.2 | 1.87 | 31.9 | 2.2 | 1.87 | 32.1 | 2.2 | 2.70 | 32.1 | 1.41 | 2.2 | 4.3 | 147 | 147 | 184 | >187 | 166 | 170 | - | Р | |
| 16.2 | - | | Pidli | | | NDM / SV | 1.82 | 34.3 | 3.1 | 1.84 | 32.6 | 3.4 | 1.83 | 33.5 | 3.2 | 2.70 | 33.5 | 1.37 | 3.2 | 4.2 | 158 | 176 | >187 | >187 | 177 | 169 | - | Р | |
| 17.1 | 4 | | | | | NDM / SV | 1.77 | 37.9 | 3.7 | 1.78 | 35.6 | 4.8 | 1.78 | 36.8 | 4.2 | 2.70 | 42.4 | 1.25 | 1.0 | 3.6 | >187 | >187 | UTP | UTP | >187 | 172 | - | Р | |
| 17.2 | FRHA | 6/01/2020 | Fill A - See site plan | ~4m placed | Clay SILT | NDM / SV | 1.83 | 34.5 | 2.5 | 1.84 | 35.3 | 1.5 | 1.84 | 34.9 | 2.0 | 2.70 | 33.2 | 1.38 | 3.1 | 3.3 | >187 | >187 | >187 | UTP | >187 | 174 | - | Р | |
| 17.3 | 4 | | pian | | | NDM / SV | 1.82 | 35.1 | 2.6 | 1.83 | 35.2 | 2.5 | 1.82 | 35.2 | 2.5 | 2.70 | 32.6 | 1.38 | 4.2 | 3.0 | >187 | >187 | >187 | >187 | >187 | 174 | - | Р | |
| 17.4 | | | | | | NDM / SV | 1.85 | 33.9 | 2.0 | 1.85 | 35.7 | 0.8 | 1.85 | 34.8 | 1.4 | 2.70 | 33.0 | 1.39 | 2.6 | 2.9 | 158 | 147 | >187 | >187 | 170 | 173 | - | Р | |
| 18.1 | FRHA | 8/01/2020 | Fill A - See site | ~4.5m placed | Clay SILT w gravels | NDM / SV | 1.80 | 36.8 | 2.8 | 1.81 | 37.1 | 2.2 | 1.80 | 37.0 | 2.5 | 2.70 | 33.8 | 1.35 | 4.5 | 3.2 | 147 | >191 | 169 | >191 | 175 | 172 | - | Р | |
| 18.2 | | | plan | | | NDM / SV | 1.82 | 37.1 | 1.8 | 1.83 | 34.7 | 2.7 | 1.82 | 35.9 | 2.2 | 2.70 | 37.3 | 1.33 | 1.4 | 3.0 | 139 | 147 | 150 | >191 | 157 | 169 | - | Р | |
| 19.1 | FRHA | 10/01/2020 | Fill A - See site | ~5m placed | Clay SILT w gravels | NDM / SV | 1.90 | 26.5 | 4.8 | 1.89 | 25.6 | 5.9 | 1.89 | 26.1 | 5.3 | 2.70 | 24.4 | 1.52 | 6.6 | 3.4 | >191 | >191 | UTP | >191 | >191 | 174 | - | Р | |
| 19.2 | | | plan | | | NDM / SV | 1.88 | 26.2 | 5.8 | 1.89 | 25.8 | 5.8 | 1.88 | 26.0 | 5.8 | 2.70 | 26.4 | 1.49 | 5.5 | 3.4 | '>191 | 122 | >191 | >191 | 168 | 176 | | Р | |
| 20.1 | FRHA | 13/01/2020 | Fill A - See site | ~5.5m placed | Clay SILT w gravels | NDM / SV | 1.84 | 28.1 | 6.7 | 1.83 | 30.0 | 5.6 | 1.83 | 29.1 | 6.1 | 2.70 | 35.8 | 1.35 | 1.7 | 3.4 | 180 | >191 | >191 | >191 | 188 | 179 | - | Р | |
| 20.2 | | | plan | | | NDM / SV | 1.79 | 22.6 | 12.9 | 1.80 | 22.4 | 12.8 | 1.79 | 22.5 | 12.9 | 2.70 | 27.0 | 1.41 | 9.6 | 4.0 | >191 | >191 | >191 | >191 | >191 | 180 | 21.1 | F | Dylan informed of fail and a retest was scheduled after he has put some damper fill in and reworked. |



Client: Lander Geotechnical Ltd.

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/SEBA/FRHA |
| Checked By | JRA/DASA/SEBA |
| Approved By | SJA |

| | | | | | | Test Type | | NDM 0 ⁰ | | | NDM 90 ⁰ | | A | VERAGE ND | м | Solid | Oven | Final Co | orrected | A | | Sh | iear Vane R | teading (H | kPa) | | | PASS / FAIL | |
|--------------|-------|------------|---------------------------|------------------------|------------|-----------|---------------------|--------------------|-----------|---------------------|---------------------|------------|---------------------|----------------|------------|--------------------------------|-------------------------|---------------------|------------------|-----------------------|-------------|------------|-------------|------------|-------------------|-------------------|------------|-------------|--|
| URN | Tech. | Date | Location | Layer | Material | 10010 | Wet Density | Moisture | Air Voids | Wet Density | Moisture | Air Voids | Wet Density | Moisture | Air Voids | Density (t/m ³) | Moisture content (%) | Oven Dry Density | | Voids (10 X Tests) | Reading | Reading | Reading R | Reading | Average | Average | Retest URN | (P) Pass | Comments |
| | | | | | | NDM / SV | (t/m ³) | Content (%) | (%) | (t/m ³) | Content (%) | (%) | (t/m ³) | Content (%) | (%) | Assumed | content (10) | (t/m ³) | Air Voids (%) | (10 x 10303) | 1 | 2 | 3 | 4 | SV (4 x Tests) | SV (10 X Sets) | | (F) Fail | |
| 21.1 | FRHA | 14/01/2020 | Fill A - See site | ~5.5m placed | Clay SILT | NDM / SV | 1.73 | 40.5 | 4.5 | 1.73 | 39.6 | 4.8 | 1.73 | 40.1 | 4.7 | 2.70 | 37.4 | 1.26 | 6.2 | 4.5 | 150 | '>191 | '>191 | '>191 | 150 | 176 | - | Р | Retest of URN 20.2 |
| 21.2 | FKHA | 14/01/2020 | plan | "5.5m placed | Clay SILT | NDM / SV | 1.71 | 37.6 | 7.1 | 1.71 | 38.5 | 6.6 | 1.71 | 38.1 | 6.8 | 2.70 | 39.8 | 1.23 | 5.9 | 4.8 | 180 | 147 | '>191 | '>191 | 164 | 174 | - | Р | |
| 22.1 | FRHA | 20/01/2020 | Fill A - See site | ~0.5m Placed | Clay SILT | NDM / SV | 1.81 | 34.1 | 3.9 | 1.80 | 36.2 | 3.4 | 1.80 | 35.2 | 3.7 | 2.70 | 39.4 | 1.29 | 1.1 | 4.5 | 180 | >191 | 177 | 166 | 179 | 173 | - | Ρ | |
| 22.2 | Turr | 20/01/2020 | plan | 0.5mm lacca | city sici | NDM / SV | 1.82 | 35.0 | 3.1 | 1.82 | 35.4 | 2.6 | 1.82 | 35.2 | 2.9 | 2.70 | 37.0 | 1.33 | 1.8 | 4.4 | >191 | >191 | >191 | >191 | >191 | 175 | - | Р | |
| 23.1 | FRHA | 22/01/2020 | Fill A - See site | ~1m Placed | Clay SILT | NDM / SV | 1.79 | 32.5 | 5.9 | 1.81 | 30.8 | 6.3 | 1.80 | 31.7 | 6.1 | 2.70 | 31.8 | 1.37 | 6.0 | 4.6 | 150 | >191 | >191 | >191 | 181 | 176 | - | Р | |
| 23.2 | FKHA | 22/01/2020 | plan | TW Maced | Clay SIL1 | NDM / SV | 1.81 | 36.4 | 2.5 | 1.82 | 36.6 | 1.9 | 1.82 | 36.5 | 2.2 | 2.70 | 35.8 | 1.34 | 2.6 | 4.7 | 163 | 174 | >191 | >191 | 180 | 178 | - | Р | |
| 24.1 | 50114 | 24/04/2020 | Fill A - See site | and Car Diaco d | dia su z | NDM / SV | 1.76 | 31.0 | 8.5 | 1.77 | 32.7 | 6.8 | 1.77 | 31.9 | 7.6 | 2.70 | 35.6 | 1.30 | 5.3 | 4.6 | >191 | >191 | >191 | >191 | >191 | 178 | - | Р | |
| 24.2 | FRHA | 24/01/2020 | plan | ~1.5m Placed | Clay SILT | NDM / SV | 1.73 | 33.6 | 8.4 | 1.74 | 35.1 | 7.1 | 1.74 | 34.4 | 7.8 | 2.70 | 32.2 | 1.31 | 9.1 | 4.9 | >191 | >191 | >191 | >191 | >191 | 180 | - | Р | |
| 25.1 | FRHA | 20/04/2020 | Fill A - See site | RL 57.8 | dia cita | NDM / SV | 1.79 | 29.4 | 8.0 | 1.79 | 30.3 | 7.4 | 1.79 | 29.9 | 7.7 | 2.70 | 29.8 | 1.38 | 7.7 | 5.5 | >191 | >191 | >191 | >191 | >191 | 181 | - | Р | |
| 25.2 | FKHA | 29/01/2020 | plan | RL 50.04 | Clay SILT | NDM / SV | 1.81 | 35.2 | 3.1 | 1.81 | 36.6 | 2.2 | 1.81 | 35.9 | 2.6 | 2.70 | 34.0 | 1.35 | 3.8 | 5.0 | >191 | >191 | >191 | >191 | >191 | 181 | - | Р | |
| 26.1 | FRHA | 31/01/2020 | Fill A - See site | ~RL 50.54 | Clay SILT | NDM / SV | 1.93 | 29.4 | 0.8 | 1.93 | 24.4 | 4.7 | 1.93 | 26.9 | 2.7 | 2.70 | 23.2 | 1.57 | 5.6 | 4.9 | >191 | >191 | >191 | >191 | >191 | 185 | - | Р | |
| 26.2 | INDA | 31/01/2020 | plan | ~6m Placed | Ciay SIL1 | NDM / SV | 1.88 | 26.0 | 6.0 | 1.88 | 25.5 | 6.6 | 1.88 | 25.8 | 6.3 | 2.70 | 25.5 | 1.5 | 6.5 | 5.0 | >191 | >191 | >191 | >191 | >191 | 187 | - | Р | |
| 27.1 | FRHA | 3/02/2020 | Fill A - See site | ~RL 51.04 | Clay SILT | NDM / SV | 1.78 | 32.8 | 6.2 | 1.78 | 33.8 | 5.9 | 1.78 | 33.3 | 6.0 | 2.70 | 35.4 | 1.3 | 4.7 | 5.3 | >191 | >191 | >191 | >191 | >191 | 189 | - | Р | |
| 27.2 | | | plan | ~RL 58.3 | | NDM / SV | 1.77 | 28.7 | 9.4 | 1.77 | 31.1 | 8.0 | 1.77 | 29.9 | 8.7 | 2.70 | 38.3 | 1.3 | 3.5 | 5.5 | >191 | >191 | >191 | >191 | >191 | 189 | - | Р | |
| 28.1 | FRHA | 5/02/2020 | Fill A - See site | RL 56.10 | Clay SILT | NDM / SV | 1.76 | 30.6 | 8.8 | 1.76 | 29.8 | 9.4 | 1.76 | 30.2 | 9.1 | 2.70 | 29.3 | 1.4 | 9.7 | 5.9 | >191 | >191 | >191 | >191 | >191 | 190 | - | Р | |
| 28.2 | | | pian | RL 51.85 | | NDM / SV | | 24.6 | 5.8 | 1.89 | 29.0 | 3.3 | 1.90 | 26.8 | 4.5 | 2.70 | 24.8 | 1.5 | 6.0 | 6.2 | >191 | >191 | | >191 | >191 | 191 | - | Р | |
| 29.1 | FRHA | 10/02/2020 | Fill A - See site plan | RL 50.17 | Clay SILT | NDM / SV | | 31.9 | 4.0 | 1.84 | 32.4 | 3.6 | 1.84 | 32.2 | 3.8 | 2.70 | 28.6 | 1.4 | 6.2 | 6.3 | 150 | >191 | | 166 | 174 | 189 | - | Р | |
| 29.2 | | | plan | RL 58.35 | | NDM / SV | 1.82 | 33.8 | 3.4 | 1.84 | 33.1 | 2.9 | 1.83 | 33.5 | 3.2 | 2.70 | 39.3 | 1.3 | 0.0 | 5.4 | >191 | >191 | | >191 | >191 | 189 | - | Р | |
| 30.1 | FRHA | 12/02/2020 | Fill A - See site plan | ~RL 56.15 | Clay SILT | NDM / SV | | 28.8 | 3.0 | 1.9 | 29.9 | 1.7 | 1.90 | 29.4 | 2.4 | 2.70 | 28.5 | 1.5 | 3.0 | 4.9 | 120 | >191 | | 188 | 157 | 186 | - | Р | |
| 30.2 | | | | ~RL 50.22 | | NDM / SV | | 29.7 | 3.4 | 1.87 | 30.0 | 3.5 | 1.87 | 29.9 | 3.4 | 2.70 | 32.9 | 1.4 | 1.4 | 4.7 | 150 | >191 | | 163 | 174 | 184 | - | P | |
| 31.1 31.2 | FRHA | 14/02/2020 | Fill A - See site plan | ~RL 58.55 ~RL 50.67 | Clay SILT | NDM / SV | | 29.7 | 8.2 | 1.79 | 28.2 | 9.1 3.2 | 1.79 | 29.0 | 8.6 | 2.70 | 31.8 | 1.4 | 6.7 | 4.8 | UTP | UTP 174 | | >191 | >191 | 184 | - | P | |
| 31.2 | | | | ~RL 50.67 | | NDM / SV | | 31.9 33.4 | 3.3 | 1.86 | 31.5 32.4 | 4.3 | 1.85 | 31.7 32.9 | 3.2 3.8 | 2.70 | 31.2 31.7 | 1.4 | 4.6 | 4.5 | 128 >191 | >191 | | >191 | >191 | 181 | - | P | |
| 32.1 | FRHA | 17/02/2020 | Fill A - See site plan | ~RL 58.00 | Clay SILT | NDM / SV | 1.05 | 21.2 | 8.6 | 1.89 | 22.7 | 4.5 8.0 | 1.85 | 22.0 | 8.3 | 2.70 | 37.2 | 1.4 | 0.0 | 4.5 | 139 | 122 | | >191 | 151 | 177 | | P | |
| 33.1 | | | | ~RL 59.00 | | NDM / SV | | 30.0 | 7.7 | 1.77 | 28.9 | 9.4 | 1.78 | 29.5 | 8.6 | 2.70 | 32.0 | 1.3 | 6.9 | 3.8 | >191 | >191 | | >191 | >191 | 177 | | Р | |
| 33.2 | FRHA | 19/02/2020 | Fill A - See site plan | ~RL 58.50 | Clay SILT | NDM / SV | - | 29.0 | 6.3 | 1.82 | 29.1 | 6.7 | 1.83 | 29.1 | 6.5 | 2.70 | 30.4 | 1.4 | 5.6 | 3.8 | >191 | >191 | | >191 | >191 | 177 | - | P | |
| 35.9 | | | 511 A . 6 | | | NDM / SV | 1.80 | 35.9 | 3.6 | 1.79 | 34.6 | 4.9 | 1.79 | 35.3 | 4.3 | 2.70 | 32.8 | 1.3 | 5.8 | 3.8 | >191 | >191 | | >191 | >191 | 179 | - | Р | |
| 35.10 | FRHA | 21/02/2020 | Fill A - See site plan | ~RL 50.85 | Clay SILT | NDM / SV | 1.89 | 26.5 | 4.9 | 1.90 | 25.7 | 5.2 | 1.90 | 26.1 | 5.0 | 2.70 | 31.7 | 1.4 | 1.0 | 3.9 | 133 | 150 | 174 | 152 | 152 | 175 | - | Р | |
| 39.1 | | | Fill A - See site | | | NDM / SV | 1.81 | 31.6 | 5.6 | 1.80 | 32.3 | 5.8 | 1.80 | 32.0 | 5.7 | 2.70 | 35.5 | 1.3 | 3.4 | 3.9 | 136 | >191 | >191 | >191 | 177 | 177 | - | Р | |
| 39.2 | FRHA | 26/02/2020 | plan | ~RL 51.35 | Clay SILT | NDM / SV | 1.80 | 36.2 | 3.3 | 1.80 | 37.8 | 2.2 | 1.80 | 37.0 | 2.7 | 2.70 | 38.8 | 1.3 | 1.7 | 3.9 | 177 | >191 | >191 | >191 | >187.5 | 178 | - | Р | |
| 42.9 | | | Fill A - See site | RL 63.65 | | NDM / SV | 1.95 | 30.6 | 0.0 | 1.95 | 31.4 | 0.0 | 1.95 | 31.0 | 0.0 | 2.70 | 37.3 | 1.4 | 0.0 | 3.3 | >191 | >191 | >191 | >191 | >191 | 178 | - | Р | |
| 42.10 | FRHA | 2/03/2020 | plan | RL 61.82 | Clay SILT | NDM / SV | 1.90 | 35.1 | 0.0 | 1.90 | 34.6 | 0.0 | 1.90 | 34.9 | 0.0 | 2.70 | 34.7 | 1.4 | 0.0 | 2.9 | >191 | >191 | >191 | >191 | >191 | 181 | - | Р | |
| 43.1 | | - / / | Fill A - See site | RL 59.16 | | NDM / SV | 1.86 | 28.7 | 5.2 | 1.86 | 29.2 | 4.7 | 1.86 | 29.0 | 4.9 | 2.70 | 36.4 | 1.4 | 0.0 | 2.4 | 106 | 95 | 95 | 120 | 104 | 173 | - | F | Shear Vanes < 120, Kyle and Dylan informed, no retest requested. |
| 43.2 | FRHA | 9/03/2020 | plan | RL 57.56 | Clay SILT | NDM / SV | 1.87 | 30.3 | 3.6 | 1.86 | 30.2 | 4.1 | 1.86 | 30.3 | 3.8 | 2.70 | 32.4 | 1.4 | 2.4 | 2.7 | >191 | 188 | >191 | 180 | >187.5 | 181 | - | Р | |
| 44.9 | FRHA | 13/03/2020 | Fill A - See site | RL 58.94 | Clau SII T | NDM / SV | 1.79 | 26.5 | 9.9 | 1.79 | 27.3 | 9.6 | 1.79 | 26.9 | 9.8 | 2.70 | 32.4 | 1.4 | 6.1 | 2.6 | >191 | >191 | >191 | >191 | >191 | 185 | - | Р | |
| 44.10 | FRHA | 13/03/2020 | plan | RL 47.90 | Clay SILT | NDM / SV | 1.83 | 29.4 | 6.1 | 1.81 | 34.0 | 4.0 | 1.82 | 31.7 | 5.1 | 2.70 | 36.0 | 1.3 | 2.3 | 2.3 | >191 | >191 | >191 | >191 | >191 | 185 | - | Р | |
| 45.11 | FRHA | 16/03/2020 | Fill A - See site | ~RL 48.40 | Clay SILT | NDM / SV | 1.88 | 27.4 | 5.1 | 1.87 | 28.6 | 4.6 | 1.87 | 28.0 | 4.9 | 2.70 | 30.5 | 1.4 | 3.1 | 2.0 | >191 | >191 | >191 | >191 | >191 | 185 | • | Р | |
| 45.12 | 1010 | 10/03/2020 | plan | ~RL 59.44 | | NDM / SV | 1.88 | 24.5 | 7.0 | 1.88 | 23.7 | 7.7 | 1.88 | 24.1 | 7.3 | 2.70 | 22.1 | 1.54 | 8.9 | 2.8 | 180 | 139 | 128 | 120 | 142 | 180 | - | Ρ | |
| 46.1 | FRHA | 18/03/2020 | Fill A - See site | ~RL 48.30 | Clav SILT | NDM / SV | 1.84 | 31.8 | 3.9 | 1.84 | 31.7 | 4.2 | 1.84 | 31.8 | 4.0 | 2.70 | 32.6 | 1.39 | 3.4 | 2.8 | >191 | >191 | >191 | >191 | >191 | 184 | - | Ρ | |
| 46.2 | THUR | 13,03,2020 | plan | ~RL 48.90 | City Sici | NDM / SV | 1.86 | 32.1 | 2.6 | 1.86 | 29.7 | 4.2 | 1.86 | 30.9 | 3.4 | 2.70 | 32.2 | 1.41 | 2.5 | 2.9 | >191 | >191 | >191 | 161 | 184 | 184 | - | Р | |



Client: Lander Geotechnical Ltd.

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/SEBA/FRHA |
| Checked By | JRA/DASA/SEBA |
| Approved By | SJA |

| | | | | | | Test Type | | NDM 0 ⁰ | | | NDM 90 ⁰ | | , | VERAGE N | м | Solid | Oven | Final Co | rrected | | | s | hear Vane | Reading (| kPa) | | | PASS / FAIL | |
|------|---------------|------------|------------------------------|---------------------------------------|-----------------------|-----------|---------------------|--------------------|-----------|---------------------|---------------------|-----------|---------------------|---------------------|-----------|--------------------------------|-------------------------|---------------------|----------------------|-----------------------|------|---------|-----------|-----------|--------------------|---------------|------------|--------------|---|
| URN | Tech. | Date | Location | Layer | Material | NDM / SV | Wet Density | Moisture | Air Voids | Wet Density | Moisture | Air Voids | Wet Density | Moisture Content | Air Voids | Density (t/m ³) | Moisture content (%) | Oven Dry Density | Average Air Voids | Voids (10 X Tests) | | Reading | Reading | Reading | Average SV (4 x | Average SV | Retest URN | (P) Pass | Comments |
| | | | | | | NDNI/ SV | (t/m ³) | (%) | (%) | (t/m ³) | (%) | (%) | (t/m ³) | (%) | (%) | Assumed | | (t/m ³) | (%) | | 1 | 2 | 3 | 4 | Tests) | (10 X Sets) | | (F) Fail | |
| 47.1 | | | Fill A - See site | ~RL 59.45 | | NDM / SV | 1.78 | 33.1 | 6.1 | 1.8 | 36.2 | 4.9 | 1.77 | 34.7 | 5.5 | 2.70 | 41.4 | 1.26 | 1.5 | 3.0 | 125 | 122 | 131 | 131 | 127 | 180 | - | Р | |
| 47.2 | FRHA | 1/05/2020 | plan | ~RL 49.00 | Clay SILT | NDM / SV | 1.81 | 31.9 | 5.3 | 1.8 | 32.9 | 4.4 | 1.81 | 32.4 | 4.9 | 2.70 | 31.0 | 1.38 | 5.8 | 3.6 | 147 | 136 | 174 | 133 | 148 | 176 | - | P | |
| 47.3 | | | Fill F - Drainage line | ~0.5m to FL | | NDM / SV | 1.78 | 32.9 | 6.4 | 1.78 | 29.9 | 8.4 | 1.78 | 31.4 | 7.4 | 2.70 | 28.8 | 1.38 | 9.1 | 4.5 | >191 | >191 | >191 | >191 | >191 | 176 | - | P Testing re | requested along filled in trench line where drainage has been installed |
| 49.1 | FRHA | 15/05/2020 | Fill E - Storm water Line | ~FL | Clay SILT | NDM / SV | 1.92 | 20.3 | 8.6 | 1.92 | 20.0 | 8.9 | 1.92 | 20.2 | 8.8 | 2.70 | 23.4 | 1.55 | 6.1 | 4.9 | 152 | 122 | 161 | 163 | 150 | 165 | - | P | |
| 49.2 | | | Fill E - Sewer line | | | NDM / SV | 1.95 | 26.8 | 1.9 | 1.94 | 28.6 | 1.0 | 1.94 | 27.7 | 1.5 | 2.70 | 20.0 | 1.62 | 7.6 | 5.0 | 122 | 120 | 139 | 150 | 133 | 159 | - | р | |
| 52.1 | JRA | 4/09/2020 | Fill A - REB | ~1m above Shear | Clay SILT w Gravels | NDM / SV | 1.88 | 35.8 | 0.0 | 1.88 | 35.8 | 0.0 | 1.88 | 35.8 | 0.0 | 2.70 | 38.3 | 1.36 | 0.0 | 4.8 | 163 | 176 | 174 | 174 | 172 | 165 | - | P | |
| 52.1 | | ,, | | Key Hardfill | | NDM / SV | 1.92 | 30.7 | 0.7 | 1.92 | 30.1 | 0.7 | 1.92 | 30.4 | 0.7 | 2.70 | 29.4 | 1.48 | 1.4 | 4.6 | >187 | 163 | 158 | >187 | 174 | 159 | - | р | |
| 53.1 | JRA | 22/09/2020 | Fill A - REB | ~5m above Shear Key Hardfill - 1st | Clay SILT w Gravels | NDM / SV | 1.87 | 29.5 | 4.1 | 1.87 | 31.1 | 2.6 | 1.87 | 30.3 | 3.4 | 2.70 | 33.1 | 1.40 | 1.5 | 3.9 | 171 | 123 | 131 | 147 | 143 | 165 | - | Р | |
| 53.2 | 5101 | 11/03/2020 | THIN THE | Bench | city size workers | NDM / SV | 1.84 | 39.3 | 0.0 | 1.85 | 31.4 | 3.8 | 1.84 | 35.4 | 1.5 | 2.70 | 32.0 | 1.40 | 3.7 | 3.9 | 126 | 144 | >187 | 171 | 157 | 156 | - | P | |
| 54.1 | SEBA | 5/10/2020 | Fill A - REB | ~5m above Shear | Clay SILT w Gravels | NDM / SV | 1.90 | 31.3 | 1.1 | 1.89 | 31.6 | 1.5 | 1.89 | 31.5 | 1.3 | 2.70 | 28.9 | 1.47 | 3.1 | 4.3 | 189 | 189 | 189 | 189 | 189 | 158 | - | Р | |
| 54.2 | SEDM | 3/10/2020 | FIII A - NEB | Key Hardfill - 1st Bench | Ciay SILT W GraVels | NDM / SV | 1.91 | 32.2 | 0.0 | 1.90 | 31.5 | 1.1 | 1.90 | 31.9 | 0.5 | 2.70 | 28.9 | 1.48 | 2.6 | 3.9 | 189 | 189 | 189 | 189 | 189 | 164 | - | Р | |
| 55.1 | STFB | 7/10/2020 | Fill A - REB | Lower Shear Key RL 43.00 | Clay SILT w Gravels | NDM / SV | 1.84 | 32.8 | 3.0 | 1.85 | 28.4 | 5.8 | 1.85 | 30.6 | 4.4 | 2.70 | 33.1 | 1.39 | 2.7 | 2.7 | 156 | 184 | 170 | 173 | 171 | 167 | - | Р | |
| 55.2 | 316 | //10/2020 | FILLA - NED | Upper Shear Key RL 56.37 | Clay SILT W Gravers | NDM / SV | 1.86 | 25.4 | 7.5 | 1.86 | 27.2 | 5.9 | 1.86 | 26.3 | 6.7 | 2.70 | 27.8 | 1.46 | 5.6 | 4.2 | 178 | 161 | 156 | 167 | 166 | 164 | - | Р | |
| 56.1 | PEFE | 9/10/2020 | Fill A - REB | Lower Shear Key | Clay SILT w Gravels | NDM / SV | 1.83 | 33.3 | 3.3 | 1.80 | 32.8 | 5.1 | 1.82 | 33.1 | 4.2 | 2.70 | 30.2 | 1.40 | 6.1 | 3.4 | 172 | 164 | 160 | 156 | 163 | 166 | - | Р | |
| 56.2 | PEFE | 9/10/2020 | FILLA - NED | RL 43.8 | Clay SILT W Gravels | NDM / SV | 1.85 | 34.3 | 1.6 | 1.84 | 35.4 | 1.6 | 1.85 | 34.9 | 1.6 | 2.70 | 30.6 | 1.41 | 4.4 | 3.1 | 164 | 174 | 200 | 170 | 177 | 170 | - | Р | |
| 57.1 | JRA | 23/10/2020 | Fill A - REB | RL 45.40 | Clay SILT w Gravels | NDM / SV | 1.83 | 37.3 | 0.9 | 1.83 | 35.6 | 1.9 | 1.83 | 36.5 | 1.4 | 2.70 | 34.1 | 1.36 | 2.9 | 3.4 | 144 | 126 | 160 | 131 | 140 | 167 | - | Р | |
| 57.2 | 2101 | 23/10/2020 | THIN NED | 112 43.40 | city size workers | NDM / SV | 1.92 | 29.8 | 1.3 | 1.92 | 28.6 | 2.0 | 1.92 | 29.2 | 1.6 | 2.70 | 27.9 | 1.50 | 2.6 | 3.5 | >187 | >187 | >187 | >187 | >187 | 168 | - | Р | |
| 58.1 | IRA | 28/10/2020 | Fill A - REB | RL 46.20 | Clay SILT w Gravels | NDM / SV | 1.81 | 32.6 | 4.9 | 1.81 | 32.4 | 5.1 | 1.81 | 32.5 | 5.0 | 2.70 | 38.2 | 1.31 | 1.4 | 3.5 | 136 | 134 | 147 | 144 | 140 | 168 | - | Р | |
| 58.2 | | | | | | NDM / SV | 1.90 | 29.3 | 2.4 | 1.91 | 28.9 | 2.2 | 1.91 | 29.1 | 2.3 | 2.70 | 27.8 | 1.49 | 3.2 | 3.5 | >187 | >187 | >187 | >187 | >187 | 171 | - | Р | |
| 59.1 | IRA | 30/10/2020 | Fill A - REB | RL 47.00 | Clay SILT w Gravels | NDM / SV | 1.84 | 31.9 | 3.7 | 1.85 | 35.3 | 1.3 | 1.84 | 33.6 | 2.5 | 2.70 | 34.7 | 1.37 | 1.8 | 3.3 | >187 | >187 | >187 | >187 | >187 | 171 | - | Р | |
| 59.2 | | | | | | NDM / SV | 1.73 | 36.4 | 7.0 | 1.74 | 36.4 | 6.2 | 1.73 | 36.4 | 6.6 | 2.70 | 34.2 | 1.29 | 7.9 | 3.9 | 144 | 163 | 158 | 136 | 150 | 167 | - | Р | |
| 60.1 | JRA | 4/11/2020 | Fill A - REB | RL 48.20 | Sandy SILT w clay and | NDM / SV | 1.75 | 31.9 | 8.6 | 1.75 | 31.9 | 8.7 | 1.75 | 31.9 | 8.6 | 2.70 | 39.1 | 1.26 | 4.3 | 4.0 | >187 | >187 | 131 | 128 | 158 | 166 | - | р | |
| 60.2 | | | | RL 48.50 | gravels | NDM / SV | 1.75 | 28.7 | 10.8 | 1.77 | 29.6 | 8.8 | 1.76 | 29.2 | 9.8 | 2.70 | 44.5 | 1.22 | 0.7 | 3.5 | 174 | 123 | 126 | 126 | 137 | 163 | - | р | |
| 61.1 | JRA | 16/11/2020 | Fill A - REB | RL 49.20 | Sandy SILT w clay and | NDM / SV | 1.80 | 37.1 | 2.9 | 1.81 | 39.2 | 0.9 | 1.80 | 38.2 | 1.9 | 2.70 | 44.6 | 1.25 | 0.0 | 2.9 | 170 | 126 | 151 | 151 | 150 | 161 | - | р | |
| 61.2 | | | | | gravels | NDM / SV | 1.76 | 42.1 | 2.0 | 1.76 | 43.9 | 0.9 | 1.76 | 43.0 | 1.4 | 2.70 | 45.8 | 1.21 | 0.0 | 2.5 | 188 | >216 | 166 | 182 | 188 | 162 | - | P | |
| 62.1 | | | | | | NDM / SV | 1.88 | 29.9 | 3.3 | 1.88 | 32.1 | 1.8 | 1.88 | 31.0 | 2.5 | 2.70 | 30.9 | 1.43 | 2.6 | 2.5 | 182 | 130 | 139 | 159 | 153 | 164 | - | р | |
| 62.2 | JRA | 4/12/2020 | Fill A - REB | RL 52.60 | Sandy SILT w clay and | NDM / SV | 1.90 | 28.5 | 3.0 | 1.92 | 27.3 | 3.1 | 1.91 | 27.9 | 3.0 | 2.70 | 31.7 | 1.45 | 0.3 | 2.2 | 139 | 139 | 159 | 153 | 148 | 160 | - | P | |
| 62.3 | | | | | gravels | NDM / SV | 1.90 | 27.8 | 3.6 | 1.90 | 27.3 | 3.9 | 1.90 | 27.6 | 3.7 | 2.70 | 29.0 | 1.47 | 2.7 | 2.4 | 133 | 179 | 184 | 170 | 167 | 162 | - | Р | |
| 62.4 | | | | | | NDM / SV | 1.89 | 28.0 | 3.9 | 1.89 | 27.1 | 4.6 | 1.89 | 27.6 | 4.3 | 2.70 | 31.2 | 1.44 | 1.7 | 2.2 | >199 | >199 | >199 | >199 | >199 | 164 | - | Р | |
| 63.1 | | | | RL 50.90 | | NDM / SV | 1.95 | 24.3 | 3.8 | 1.95 | 23.6 | 4.4 | 1.95 | 24.0 | 4.1 | 2.70 | 20.9 | 1.61 | 6.6 | 2.7 | >199 | >199 | >199 | >199 | >199 | 165 | - | Р | |
| 63.2 | | | | RL 51.90 | | NDM / SV | 1.86 | 34.7 | 1.2 | 1.86 | 34.6 | 0.8 | 1.86 | 34.7 | 1.0 | 2.70 | 30.6 | 1.42 | 3.7 | 2.3 | 176 | >199 | 153 | 156 | 171 | 167 | - | Р | |
| 63.3 | JRA | 7/12/2020 | Fill A - REB | RL 52.00 | Sandy SILT w clay and | NDM / SV | 1.91 | 28.7 | 2.6 | 1.91 | 28.3 | 2.9 | 1.91 | 28.5 | 2.7 | 2.70 | 25.4 | 1.52 | 5.0 | 2.3 | 156 | 122 | 125 | 139 | 136 | 165 | - | | Dug by Dines to test missed layers as requested by Kyle (Lander) / Trevor (Dines) - Testing |
| 63.4 | | | | RL 51.00 | gravels | NDM / SV | 1.90 | 29.7 | 2.4 | 1.89 | 30.4 | 2.4 | 1.89 | 30.1 | 2.4 | 2.70 | 28.9 | 1.47 | 3.2 | 2.6 | 128 | 153 | >199 | 133 | 153 | 166 | - | P | ed behind area of Geo Grid. |
| 63.5 | | | | RL 53.40 | | NDM / SV | | 24.4 | 8.2 | 1.87 | 25.7 | 6.9 | 1.86 | 25.1 | 7.5 | 2.70 | 23.8 | 1.50 | 8.5 | 3.4 | >199 | >199 | 153 | >199 | >187 | 170 | - | Р | |
| 63.6 | | | | RL 52.90 | | NDM / SV | 1.86 | 23.1 | 9.1 | 1.86 | 23.9 | 8.6 | 1.86 | 23.5 | 8.8 | 2.70 | 27.5 | 1.46 | 5.9 | 4.0 | >199 | >187 | 184 | >187 | 189 | 170 | - | Р | |
| 64.1 | | | Fill A - REB | RL 53.0 | | NDM / SV | 1.90 | 26.7 | 4.6 | 1.90 | 28.4 | 3.3 | 1.90 | 27.6 | 3.9 | 2.70 | 25.2 | 1.52 | 5.7 | 4.3 | >199 | >199 | UTP | UTP | >199 | 175 | - | Р | |
| 64.2 | JRA / DASA | 9/12/2020 | | | Clay SILT w gravels | NDM / SV | | 31.9 | 2.6 | 1.80 | 29.9 | 7.0 | 1.83 | 30.9 | 4.8 | 2.70 | 36.7 | 1.34 | 1.1 | 4.4 | 136 | 139 | >199 | 156 | 158 | 176 | - | Р | |
| 64.3 | DAJA | | Fill Area - 415/422 | ~1.5m below FL | | NDM / SV | | 35.2 | 1.3 | 1.85 | 34.1 | 1.6 | 1.85 | 34.7 | 1.5 | 2.70 | 37.5 | 1.35 | 0.0 | 4.1 | 139 | 136 | 153 | 159 | 147 | 174 | - | | ovided Area tested Reference of 415/422 - See Site plan for approximate testing location. |
| 64.4 | | | | | | NDM / SV | 1.83 | 28.3 | 7.0 | 1.80 | 33.7 | 4.6 | 1.81 | 31.0 | 5.7 | 2.70 | 37.3 | 1.32 | 1.7 | 4.1 | 133 | 139 | 145 | 139 | 139 | 168 | - | Р | |



Client: Lander Geotechnical Ltd.

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/SEBA/FRHA |
| Checked By | JRA/DASA/SEBA |
| Approved By | SJA |

| | | | | | | Test Type | | NDM 0 ⁰ | | | NDM 90 ⁰ | | A | VERAGE NI | м | Solid | Oven | Final Co | orrected | Avorago Air | | SI | iear Vane Re | ading (kPa) |) | | | PASS / FAIL | |
|-------|-------|------------|---------------------------|----------------|---------------------------|-----------|---------------------|---------------------|------------------|---------------------|---------------------|------------|---------------------|---------------------|------------------|---|-------------------------|---------------------|-----------|-----------------------|--------------|--------------|--------------|-------------|--------------|---------------|------------|-------------|---|
| URN | Tech. | Date | Location | Layer | Material | NDM / SV | | Moisture Content | Air Voids (%) | Wet Density | Moisture Content | Air Voids | Wet Density | Moisture Content | Air Voids (%) | Density (t/m ³) Assumed | Moisture content (%) | Density | Pui voida | Voids (10 X Tests) | Reading 1 | Reading 2 | | ading SV | V (4 x | Average SV | Retest URN | (P) Pass | Comments |
| | | | | | | | (t/m ³) | (%) | | (t/m ³) | (%) | | (t/m ³) | (%) | | | | (t/m ³) | (%) | | | | | | | 10 X Sets) | | (F) Fail | |
| 65.1 | | | | RL 54.0 | | NDM / SV | | 29.5 | 6.3 | 1.81 | 30.6 | 6.1 | 1.82 | 30.1 | 6.2 | 2.70 | 37.8 | 1.32 | 1.2 | 3.6 | 156 | 133 | | | 161 | 164 | - | Р | |
| 65.2 | JRA / | | Fill A - REB | | | NDM / SV | | 26.8 | 1.4 | 1.98 | 24.8 | 1.9 | 1.97 | 25.8 | 1.7 | 2.70 | 28.0 | 1.54 | 0.0 | 3.2 | UTP | UTP | | | 191.5 | 166 | | Р | |
| 65.3 | DASA | 14/12/2020 | | RL 53.0 | Clay SILT w gravels | NDM / SV | | 26.4 | 4.6 7.7 | 1.90 | 28.2 | 3.3 | 1.90 | 27.3 21.2 | 3.9 | 2.70 | 31.3 | 1.45 | 1.1 | 2.8 | 142 UTP | 156 | | | 170 | 169 | - | P | |
| 65.4 | - | | Fill Area - 415/422 | ~1m below FL | | | 1.91 | 22.0 | | 1.94 | 20.4 | 7.3 | 1.93 | | 7.5 | 2.70 | 20.7 | 1.60 | 7.9 | | | >199 | | | 196 | 174 175 | | P | |
| 66.1 | | | | | | NDM / SV | 1.99 1.93 | 24.3 25.4 | 1.6 4.1 | 1.99 1.93 | 23.9 25.0 | 2.2 | 1.99 1.93 | 24.1 25.2 | 1.9 4.1 | 2.70 2.70 | 25.6 31.2 | 1.59 | 0.7 | 2.5 | UTP >199 | >199 >199 | | | >199 >199 | 175 | - | P | |
| 66.2 | | | Fill Area - 415/422 | ~0.5m below FL | | NDM / SV | 1.93 | 23.4 | 4.1 | 1.95 | 23.0 | 4.1 0.8 | 1.95 | 23.2 | 4.1 | 2.70 | 29.9 | 1.47 | 0.0 | 1.9 | >199 | >199 | | | 199 | 176 | | P | |
| 66.3 | JRA | 16/12/2020 | | RL 53.20 | Clay SILT w gravels | NDM / SV | | 25.6 | 5.0 | 1.94 | 28.0 | 3.9 | 1.94 | 26.4 | 4.4 | 2.70 | 32.9 | 1.49 | 0.0 | 1.4 | 182 | >199 | | | 199 | 178 | | P | |
| 66.4 | | | Fill A - REB | RL 53.20 | - | NDM / SV | | 27.8 | 0.0 | 1.90 | 29.0 | 0.0 | 1.99 | 28.4 | 0.0 | 2.70 | 28.8 | 1.45 | 0.0 | 1.3 | >199 | >199 | | | >199 | 185 | | P | |
| 68.1 | | | | ~RL 65.00 | | NDM / SV | | 32.9 | 4.3 | 1.90 | 33.2 | 5.3 | 1.81 | 33.1 | 4.8 | 2.70 | 37.5 | 1.34 | 2.0 | 1.3 | 151 | >216 | | | 172 | 185 | | P | |
| 68.2 | 1 | | _ | ~RL 60.00 | + | NDM / SV | | 36.2 | 4.3 6.0 | 1.80 | 35.8 | 6.1 | 1.01 | 36.0 | 4.0 6.0 | 2.70 | 34.9 | 1.31 | 6.7 | 1.5 | >216 | >216 | | | >216 | 193 | | P | |
| 68.3 | DASA | 6/01/2021 | Fill A - See Site Plan | ~RL 53.00 | Clay SILT w gravels | NDM / SV | | 32.6 | 0.0 | 1.75 | 33.8 | 0.0 | 1.75 | 33.5 | 0.0 | 2.70 | 33.0 | 1.30 | 0.0 | 1.9 | >216 | >216 | | | 197 | 195 | | P | |
| 68.4 | 1 | | | ~RL 53.00 | ł | NDM / SV | | 29.6 | 4.1 | 1.91 | 29.0 | 4.1 | 1.91 | 29.3 | 4.1 | 2.70 | 32.5 | 1.45 | 1.9 | 1.9 | >216 | >216 | | | >216 | 194 | | P | |
| 70.5 | - | | | ~ RL 63 | | NDM / SV | | 25.0 | 7.5 | 1.86 | 25.2 | 7.5 | 1.85 | 25.6 | 7.5 | 2.70 | 22.3 | 1.41 | 10.0 | 2.1 | 46 | 65 | | | 66 | 195 | 74.8 | F | |
| 70.6 | DASA | 11/01/2021 | Fill A - See Site Plan | ~ RL 63 | Sandy SILT w some Clay | NDM / SV | | 22.4 | 12.6 | 1.75 | 22.9 | 14.5 | 1.78 | 22.7 | 13.5 | 2.70 | 23.8 | 1.44 | 12.7 | 3.3 | 52 | 46 | | | 47 | 170 | 74.9 | F | SV < 120 kPa & AV > 12% - Dines informed material would be removed and replaced |
| 70.7 | - | ,-, | Fill A - REB | ~ RL 53 | Clay SILT w Gravels | NDM / SV | | 32.5 | 7.2 | 1.77 | 34.0 | 6.1 | 1.77 | 33.3 | 6.6 | 2.70 | 31.4 | 1.35 | 7.8 | 4.1 | 170 | 154 | | | 171 | 167 | - | P | |
| 72.1 | | | | ~ RL 54 | | NDM / SV | 1.79 | 38.4 | 2.6 | 1.80 | 37.3 | 2.6 | 1.79 | 37.9 | 2.6 | 2.70 | 32.8 | 1.35 | 5.7 | 4.7 | 170 | 154 | | | 174 | 165 | - | P | |
| 72.2 | DASA | 12/01/2021 | Fill A - REB | ~ RL 55 | Clay SILT some Sand | NDM / SV | | 30.5 | 6.8 | 1.81 | 32.4 | 5.0 | 1.81 | 31.5 | 5.9 | 2.70 | 23.3 | 1.46 | 11.6 | 5.8 | 170 | 173 | | | 168 | 162 | | P | |
| 74.8 | | | | ~0.3m Below FL | | NDM / SV | | 36.0 | 2.6 | 1.81 | 36.0 | 2.6 | 1.81 | 36.0 | 2.6 | 2.70 | 35.0 | 1.34 | 3.2 | 6.2 | >199 | >199 | | | 199 | 162 | | Р | Retest of URN 70.5 |
| 74.9 | JRA | 13/01/2021 | Fill A - See Site Plan | ~FL | Clay SILT some Sand | NDM / SV | | 23.5 | 10.4 | 1.83 | 23.4 | 10.3 | 1.83 | 23.5 | 10.3 | 2.70 | 25.8 | 1.45 | 8.6 | 6.8 | >199 | >199 | | | 199 | 165 | | Р | Retest of URN 70.6 |
| 75.5 | | | | | | NDM / SV | | 34.4 | 1.3 | 1.86 | 34.3 | 1.1 | 1.86 | 34.4 | 1.2 | 2.70 | 32.1 | 1.41 | 2.6 | 6.4 | >216 | >216 | | | 212 | 165 | | Р | |
| 75.6 | DASA | 14/01/2021 | Fill A - REB | ~RL 54.5 | Clay SILT some Sand | NDM / SV | | 44.3 | 0.2 | 1.78 | 42.3 | 0.9 | 1.77 | 43.3 | 0.6 | 2.70 | 46.7 | 1.21 | 0.0 | 6.4 | 170 | 185 | | | 185 | 164 | | Р | |
| 77.1 | | | | RL 54.0 | | NDM / SV | | 37.9 | 1.1 | 1.83 | 36.6 | 1.6 | 1.82 | 37.3 | 1.4 | 2.70 | 43.9 | 1.27 | 0.0 | 6.2 | 154 | 139 | | | 147 | 157 | | Р | |
| 77.2 | DASA | 15/01/2021 | Fill A - REB | RL 56.40 | Clay SILT w Gravels | NDM / SV | 1.82 | 31.5 | 5.0 | 1.82 | 33.3 | 4.0 | 1.82 | 32.4 | 4.5 | 2.70 | 34.9 | 1.35 | 2.9 | 5.5 | >216 | >216 | >216 > | >216 > | >216 | 172 | | Р | |
| 79.10 | | | | | | NDM / SV | 1.70 | 37.0 | 7.9 | 1.70 | 38.4 | 7.2 | 1.70 | 37.7 | 7.5 | 2.70 | 45.7 | 1.17 | 3.2 | 4.6 | >199 | >199 | >199 > | >199 > | >199 | 187 | | Р | |
| 79.11 | JRA | 18/01/2021 | Fill A - REB | ~RL 56.7 | Clay SILT w Gravels | NDM / SV | 1.78 | 36.9 | 3.8 | 1.77 | 36.1 | 4.6 | 1.78 | 36.5 | 4.2 | 2.70 | 41.8 | 1.25 | 1.2 | 3.9 | 139 | 159 | >199 > | 199 1 | 174 | 187 | | Р | |
| 83.1 | 1 | | | | | NDM / SV | | 41.1 | 0.0 | 1.82 | 38.5 | 0.9 | 1.82 | 39.8 | 0.0 | 2.70 | 41.0 | 1.29 | 0.0 | 3.3 | >216 | >216 | | | >216 | 191 | - | Р | |
| 83.2 | DASA | 27/01/2021 | Fill A - REB | ~RL 56 | Clay SILT w Gravels | NDM / SV | 1.75 | 41.5 | 3.1 | 1.75 | 41.0 | 3.1 | 1.75 | 41.3 | 3.1 | 2.70 | 35.5 | 1.29 | 6.4 | 2.8 | 200 | 185 | 188 | 185 1 | 190 | 194 | | Р | |
| 84.1 | 1 | | | | | NDM / SV | 1.91 | 28.9 | 2.2 | 1.91 | 30.0 | 1.3 | 1.91 | 29.5 | 1.7 | 2.70 | 25.9 | 1.52 | 4.4 | 2.9 | 123 | 126 | 139 | 126 1 | 129 | 187 | | Р | |
| 84.2 | DASA | 29/01/2021 | Fill A - REB | ~RL 56.5 | Clay SILT w Gravels | NDM / SV | 1.68 | 52.2 | 1.6 | 1.68 | 49.4 | 2.8 | 1.68 | 50.8 | 2.2 | 2.70 | 40.2 | 1.20 | 7.5 | 2.8 | >216 | >216 | | | 204 | 187 | | Р | |
| 85.1 | 1 | | | RL 44.6 | | NDM / SV | 1.87 | 25.4 | 7.1 | 1.87 | 24.0 | 8.2 | 1.87 | 24.7 | 7.6 | 2.70 | 28.6 | 1.45 | 4.8 | 3.0 | 170 | 185 | 173 | 188 1 | 179 | 184 | | Р | |
| 85.2 | DASA | 2/02/2021 | Fill B | RL 46.6 | Clay SILT | NDM / SV | 1.80 | 31.4 | 6.5 | 1.80 | 30.3 | 7.0 | 1.80 | 30.9 | 6.7 | 2.70 | 32.7 | 1.35 | 5.5 | 3.6 | 185 | 188 | >216 > | >216 | 201 | 185 | | Р | |
| 85.3 | 1 | | | RL 48.9 | 1 | NDM / SV | 1.78 | 28.3 | 9.5 | 1.77 | 28.4 | 9.6 | 1.78 | 28.4 | 9.6 | 2.70 | 29.5 | 1.37 | 8.8 | 4.5 | >216 | 188 | 200 | 200 2 | 201 | 191 | | Р | |
| 86.1 | 1 | | | ~ FL | | NDM / SV | 1.79 | 45.7 | 0.0 | 1.79 | 45.7 | 0.0 | 1.79 | 45.7 | 0.0 | 2.70 | 38.1 | 1.29 | 2.7 | 4.5 | >216 | >216 | >216 > | 216 > | 216 | 191 | - | Р | |
| 86.2 | DASA | 3/02/2021 | Fill C | ~ RL 78 | Clay SILT | NDM / SV | 1.83 | 30.3 | 5.3 | 1.83 | 29.7 | 5.8 | 1.83 | 30.0 | 5.5 | 2.70 | 31.3 | 1.40 | 4.6 | 4.6 | >216 | >216 | >216 > | >216 > | 216 | 193 | - | Р | |
| 88.11 | | | | | _ | NDM / SV | 1.79 | 39.6 | 1.6 | 1.80 | 38.3 | 2.0 | 1.80 | 39.0 | 1.8 | 2.70 | 36.0 | 1.32 | 3.6 | 4.8 | >216 | >216 | >216 > | >216 > | >216 | 197 | - | Р | |
| 88.12 | DASA | 5/02/2021 | Fill C | 1m placed | Clay SILT | NDM / SV | 1.77 | 36.6 | 4.8 | 1.77 | 37.0 | 4.1 | 1.77 | 36.8 | 4.5 | 2.70 | 36.1 | 1.30 | 4.9 | 5.3 | >216 | >216 | >216 > | >216 > | 216 | 197 | - | Р | |
| 91.1 | | | | 1m placed | | NDM / SV | 1.75 | 32.5 | 8.4 | 1.75 | 31.6 | 8.7 | 1.75 | 32.1 | 8.6 | 2.70 | 30.8 | 1.34 | 9.4 | 5.6 | >216 | >216 | >216 | UTP > | >216 | 199 | - | Р | |
| 91.2 | DASA | 12/02/2021 | Fill B | 0.5m placed | Clay SILT | NDM / SV | 1.61 | 28.6 | 17.8 | 1.61 | 26.0 | 19.4 | 1.61 | 27.3 | 18.6 | 2.70 | 27.9 | 1.26 | 18.2 | 7.0 | >216 | >216 | >216 > | >216 > | 216 | 208 | 93.1 | F | |
| L | + | | I | | 1 | | | | · · · · · · | | | | | | | | | | | | | ı – I | I | | | | | | 4 |



Client: Lander Geotechnical Ltd.

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/SEBA/FRHA |
| Checked By | JRA/DASA/SEBA |
| Approved By | SJA |

| | | | | | | Test Type NDM 0 ⁰ | | | NDM 90 ⁰ | | | AVERAGE NDM | | | Solid | 0.00 | Final Co | rrected | .ed | | SI | hear Vane F | eading (kP | Pa) | | | PASS / FAIL | | |
|--------------|-----------------------|----------------------|--------------------------|--------------------|-----------|------------------------------|--------------------------------|----------------|---------------------|---------------------|----------------|-------------|---------------------|----------------|------------|--------------------------------|-------------------------|--------------------------------|------------------|-----------------------|--------------|--------------|------------|------|---------------------|------------------|-------------|----------|--------------------------|
| URN | Tech. | Date | Location | Layer | Material | 1014 (5) | Wet | Moisture | Air Voids | Wet Density | Moisture | Air Voids | Wet Density | Moisture | Air Voids | Density (t/m ³) | Moisture content (%) | Oven Dry | Average | Voids (10 X Tests) | Reading | Reading | Reading F | | Average | Average | Retest URN | (P) Pass | Comments |
| | | | | | | NDM / SV | Density (t/m ³) | Content (%) | (%) | (t/m ³) | Content (%) | (%) | (t/m ³) | Content (%) | (%) | Assumed | content (70) | Density (t/m ³) | Air Voids (%) | (10 x 10303) | 1 | 2 | 3 | | SV (4 x Tests) (| SV 10 X Sets) | | (F) Fail | 1 |
| 92.1 | | | | 2m placed | | NDM / SV | 1.72 | 34.1 | 8.8 | 1.73 | 32.1 | 9.5 | 1.72 | 33.1 | 9.2 | 2.70 | 35.1 | 1.28 | 8.0 | 7.1 | 154 | 170 | 157 | 173 | 164 | 204 | - | Р | |
| 92.2 | DASA | 17/02/2021 | 17/02/2021 Fill B | 1.5m placed | Clay SILT | NDM / SV | 1.57 | 31.9 | 17.9 | 1.57 | 34.3 | 16.6 | 1.57 | 33.1 | 17.2 | 2.70 | 29.1 | 1.22 | 19.5 | 8.5 | >216 | >216 | >216 | >216 | >216 | 208 | 93.1 | F | |
| 92.3 | DASA | 17/02/2021 | | 1.5m placed | Ciay Sici | NDM / SV | 1.70 | 38.3 | 7.1 | 1.70 | 35.9 | 8.7 | 1.70 | 37.1 | 7.9 | 2.70 | 37.1 | 1.24 | 7.9 | 8.8 | 170 | 188 | 173 | 176 | 177 | 205 | - | Р | |
| 92.4 | | | | 2m placed | | NDM / SV | 1.79 | 38.3 | 2.3 | 1.80 | 28.6 | 8.3 | 1.80 | 33.5 | 5.2 | 2.70 | 24.2 | 1.45 | 11.5 | 9.0 | 170 | 173 | 176 | 173 | 173 | 203 | | Р | |
| 93.1 | DASA | 18/02/2021 | Fill B | 2.5m placed | Clay SILT | NDM / SV | 1.81 | 31.9 | 5.3 | 1.81 | 34.6 | 3.6 | 1.81 | 33.3 | 4.4 | 2.70 | 32.0 | 1.37 | 5.2 | 9.3 | >216 | >216 | 203 | 185 | 205 | 201 | | Р | Retest of URN 91.2, 92.2 |
| 94.1 | | | | ~RL 45.2 | | NDM / SV | 1.69 | 35.7 | 9.3 | 1.70 | 34.7 | 9.5 | 1.70 | 35.2 | 9.4 | 2.70 | 34.1 | 1.26 | 10.0 | 9.8 | 185 | >191 | >216 | 185 | 194 | 199 | | Р | |
| 94.2 | DASA | 19/02/2021 Fill B | Fill B | ~REL 45.2 | Clay SILT | NDM / SV | 1.79 | 38.4 | 2.5 | 1.78 | 39.1 | 2.6 | 1.78 | 38.8 | 2.6 | 2.70 | 38.7 | 1.29 | 2.6 | 9.7 | 170 | 173 | 173 | 182 | 175 | 195 | - | Р | |
| 94.3 | | | | ~RL 48 | | NDM / SV | 1.80 | 38.3 | 1.7 | 1.80 | 37.5 | 2.3 | 1.80 | 37.9 | 2.0 | 2.70 | 48.6 | 1.21 | 0.0 | 9.2 | >216 | >216 | >216 | >216 | >216 | 195 | - | Р | |
| 94.4 | | | | ~RL 53 | | NDM / SV | | 39.4 | 3.9 | 1.76 | 40.6 | 2.8 | 1.76 | 40.0 | 3.4 | 2.70 | 37.0 | 1.28 | 5.1 | 8.8 | 170 | 185 | | >191 | 180 | 191 | • | Р | |
| 95.1 | | | | RL 54.2 | | NDM / SV | | 32.0 | 6.6 | 1.79 | 33.4 | 5.5 | 1.79 | 32.7 | 6.1 | 2.70 | 33.0 | 1.34 | 5.9 | 7.6 | 154 | 173 | 170 | 154 | 163 | 186 | - | Р | |
| 95.2 | DASA | 22/02/2021 | Fill B | RL 53.0 | Clay SILT | NDM / SV | 1.81 | 38.0 | 1.7 | 1.81 | 37.4 | 2.0 | 1.81 | 37.7 | 1.9 | 2.70 | 51.7 | 1.19 | 0.0 | 6.8 | >216 | >216 | >216 | | >216 | 191 | - | Р | |
| 95.3 | | | | RL 51.0 | | NDM / SV | 1.86 | 33.1 | 2.2 | 1.85 | 33.2 | 2.3 | 1.85 | 33.2 | 2.2 | 2.70 | 49.2 | 1.24 | 0.0 | 4.8 | 188 | 154 | 154 | | 167 | 187 | - | Р | |
| 96.1 | DASA | 23/02/2021 | REB 1 | 600mm placed | Clay SILT | NDM / SV | 1.74 | 31.5 | 9.1 | 1.76 | 31.0 | 8.6 | 1.75 | 31.3 | 8.9 | 2.70 | 31.0 | 1.34 | 9.0 | 4.9 | >216 | >216 | | _ | >216 | 190 | - | Р | |
| 97.1 | - | | | RL 56.6 | | NDM / SV | 1.82 | 39.3 | 0.1 | 1.81 | 39.5 | 0.5 | 1.82 | 39.4 | 0.3 | 2.70 | 38.1 | 1.32 | 1.1 | 3.9 | >216 | >216 | | | >216 | 195 | - | P | |
| 97.2 97.3 | DASA | 24/02/2021 | REB 2 | RL 54.5 | Clay SILT | NDM / SV | 1.85 | 30.0 35.5 | 4.7 | 1.86 | 26.5 37.7 | 6.5 | 1.86 | 28.3 | 5.6 | 2.70 | 37.4 | 1.35 | 0.0 | 3.4 | >216 >216 | >216 >216 | >216 | _ | >216 | 196 198 | - | Р | |
| 97.3 | | | | RL 46.6 | | NDM / SV | 1.83 | 35.5 | 2.2 | 1.83 | 37.7 | 0.9 2.5 | 1.83 | 36.6 32.4 | 1.6 2.6 | 2.70 | 32.4 36.7 | 1.38 | 4.2 0.0 | 2.8 | >216 | >216 | | | 214 >216 | 202 | - | P | |
| 97.4 | | | | RL 46.6 | | NDM / SV | 1.85 | 32.2 | 0.0 | 1.86 | 32.5 | 0.3 | 1.86 | 32.4 | 0.0 | 2.70 | 36.7 | 1.36 | 0.0 | 2.5 | 185 | 170 | 185 | | 178 | 198 | | P | |
| 98.2 | DASA | 26/02/2021 | /2021 REB 2 | RL 56.0 | Clay SILT | NDM / SV | | 40.0 | 0.0 | 1.83 | 43.4 | 0.0 | 1.84 | 41.7 | 0.0 | 2.70 | 43.1 | 1.30 | 0.0 | 2.0 | >216 | >216 | 185 | _ | 205 | 201 | | P | |
| 98.3 | - Chart | 20/02/2021 | | RL 46.8 | | NDM / SV | 1.69 | 39.0 | 7.6 | 1.69 | 40.9 | 6.3 | 1.69 | 40.0 | 7.0 | 2.70 | 38.0 | 1.23 | 8.0 | 2.2 | >216 | >216 | >216 | | >216 | 206 | | Р | |
| 99.1 | | | | | | NDM / SV | 1.92 | 34.9 | 0.0 | 1.92 | 35.3 | 0.0 | 1.92 | 35.1 | 0.0 | 2.70 | 34.6 | 1.43 | 0.0 | 2.2 | 170 | 173 | | | 194 | 204 | - | P | |
| 99.2 | | | | RL 56.5 RL 47.6 | Clay SILT | NDM / SV | 1.71 | 38.4 | 6.7 | 1.71 | 37.3 | 7.3 | 1.71 | 37.9 | 7.0 | 2.70 | 40.5 | 1.22 | 5.5 | 2.8 | >216 | >216 | | | >216 | 209 | | р | |
| 99.3 | DASA | DASA 1/03/2021 REB 2 | REB 2 | | | NDM / SV | 1.80 | 34.2 | 4.5 | 1.79 | 33.0 | 5.5 | 1.80 | 33.6 | 5.0 | 2.70 | 46.9 | 1.22 | 0.0 | 1.9 | 170 | 173 | >216 | >216 | 194 | 206 | | Р | |
| 99.4 | | | | | | NDM / SV | 1.79 | 33.2 | 5.5 | 1.77 | 31.3 | 8.0 | 1.78 | 32.3 | 6.8 | 2.70 | 41.0 | 1.26 | 1.5 | 1.9 | >216 | >216 | >216 | >216 | >216 | 206 | | Р | |
| 100.1 | | | | RL 48.4 | | NDM / SV | 1.75 | 40.1 | 3.6 | 1.73 | 41.7 | 4.0 | 1.74 | 40.9 | 3.8 | 2.70 | 45.2 | 1.20 | 1.5 | 2.1 | 62 | 74 | 92 | 99 | 82 | 193 | 101.2 | F | SV < 120 kPa |
| 100.2 | | | | RL 49.20 | Clay SILT | NDM / SV | 1.66 | 44.0 | 6.9 | 1.64 | 46.1 | 6.6 | 1.65 | 45.1 | 6.7 | 2.70 | 43.6 | 1.15 | 7.5 | 2.4 | >216 | >216 | >216 | >216 | >216 | 193 | | Р | |
| 100.3 | DASA | A 8/03/2021 REB 2 | 050.2 | RL 48.4 | | NDM / SV | 1.66 | 46.1 | 5.3 | 1.67 | 46.7 | 4.7 | 1.67 | 46.4 | 5.0 | 2.70 | 46.2 | 1.14 | 5.1 | 2.9 | 170 | 173 | 170 | 182 | 174 | 189 | - | Р | |
| 100.4 | DASA | | NED 2 | RL 48.4 | | NDM / SV | 1.72 | 46.3 | 2.1 | 1.72 | 48.0 | 1.4 | 1.72 | 47.2 | 1.8 | 2.70 | 37.3 | 1.25 | 7.0 | 3.6 | 170 | 173 | 170 | 182 | 174 | 189 | • | Р | |
| 100.5 | | | | RL 58 | | NDM / SV | 1.76 | 42.0 | 2.1 | 1.77 | 41.8 | 1.6 | 1.76 | 41.9 | 1.9 | 2.70 | 42.2 | 1.24 | 1.7 | 3.8 | >216 | 216 | >216 | >216 | 167 | 185 | • | Р | |
| 100.6 | | | | RL 58.5 | | NDM / SV | 1.76 | 37.4 | 4.6 | 1.75 | 38.7 | 4.5 | 1.76 | 38.1 | 4.5 | 2.70 | 40.0 | 1.25 | 3.4 | 3.3 | 170 | 166 | 173 | 185 | 174 | 181 | - | Р | |
| 101.1 | | | REB 2 | RL 50 | | NDM / SV | 1.77 | 37.8 | 3.7 | 1.78 | 37.9 | 3.4 | 1.78 | 37.9 | 3.5 | 2.70 | 44.4 | 1.23 | 0.0 | 3.3 | 173 | 167 | 170 | 196 | 177 | 179 | - | Р | |
| 101.2 | JRA | | | RL 48.4 Cl | Clay SILT | NDM / SV | 1.77 | 38.1 | 3.7 | 1.78 | 37.4 | 3.5 | 1.78 | 37.8 | 3.6 | 2.70 | 38.1 | 1.29 | 3.4 | 3.1 | >199 | >199 | >199 | >199 | >199 | 177 | - | Р | Retest of URN 100.1 |
| 101.2 | | | Slip" Area above Pond | RL 48.8 (~FL) | | NDM / SV | 1.74 | 37.8 | 5.6 | 1.72 | 41.3 | 4.4 | 1.73 | 39.6 | 5.0 | 2.70 | 35.6 | 1.28 | 7.3 | 3.8 | 156 | >199 | 179 | >199 | 183 | 176 | - | Р | |
| 102.1 | | | | | chu chi T | NDM / SV | 1.79 | 35.1 | 4.6 | 1.78 | 34.4 | 5.3 | 1.78 | 34.8 | 4.9 | 2.70 | 33.9 | 1.33 | 5.5 | 4.2 | >216 | >216 | >216 | >216 | >216 | 176 | - | Р | |
| 102.2 | | | | RL 50.40 | | NDM / SV | 1.80 | 31.6 | 6.1 | 1.81 | 30.4 | 6.4 | 1.81 | 31.0 | 6.2 | 2.70 | 39.6 | 1.29 | 0.9 | 4.2 | >216 | >216 | >216 | >216 | >216 | 190 | - | Р | |
| 102.3 | DASA 12/03/2021 REB 2 | REB 2 | RL 57.20 | Clay SILT | NDM / SV | 1.84 | 35.4 | 1.3 | 1.82 | 36.8 | 2.0 | 1.83 | 36.1 | 1.7 | 2.70 | 42.8 | 1.28 | 0.0 | 3.4 | >216 | >216 | >216 | >216 | >216 | 190 | - | Р | | |
| 102.4 | | | | RL 59.00 | | NDM / SV | 1.82 | 38.2 | 0.9 | 1.78 | 37.7 | 3.4 | 1.80 | 38.0 | 2.1 | 2.70 | 37.3 | 1.31 | 2.5 | 3.2 | >216 | UTP | >216 | UTP | >216 | 194 | - | Р | |
| 103.1 | DASA | 16/03/2021 | 21 REB 2 | RL 51.60 | Clay SILT | NDM / SV | 1.87 | 33.8 | 1.2 | 1.87 | 33.8 | 1.1 | 1.87 | 33.8 | 1.2 | 2.70 | 39.1 | 1.34 | 0.0 | 2.5 | 154 | 170 | 166 | 163 | 163 | 193 | - | Р | |
| 103.2 | DAJA | 10/03/2021 | NED Z | NE 51.00 | Clay SILI | NDM / SV | 1.67 | 47.6 | 4.4 | 1.65 | 47.7 | 5.2 | 1.66 | 47.7 | 4.8 | 2.70 | 52.5 | 1.09 | 2.5 | 2.6 | >216 | >216 | >216 | >216 | >216 | 198 | | Р | |
| 104.1 | | | | RL 51.59 | | NDM / SV | 1.78 | 34.2 | 5.5 | 1.78 | 34.2 | 5.7 | 1.78 | 34.2 | 5.6 | 2.70 | 37.3 | 1.30 | 3.7 | 2.6 | 170 | 173 | >216 | >216 | 194 | 199 | - | Р | |
| 104.2 | DASA | SA 18/03/2021 REB2 | RL 51.93 | Clay SILT | NDM / SV | 1.76 | 33.4 | 7.1 | 1.76 | 32.1 | 8.1 | 1.76 | 32.8 | 7.6 | 2.70 | 38.9 | 1.26 | 3.9 | 3.0 | 108 | 111 | 99 | 108 | 107 | 193 | 105.1 | F | | |
| 104.3 | | | ~RL 58.75 | city size | NDM / SV | 1.80 | 36.6 | 2.9 | 1.79 | 37.8 | 2.8 | 1.80 | 37.2 | 2.9 | 2.70 | 42.8 | 1.26 | 0.0 | 2.6 | 108 | 92 | 111 | 114 | 106 | 183 | 105.2 | F | 4 | |
| 104.4 | | | | | NDM / SV | 1.84 | 28.6 | 6.3 | 1.84 | 30.5 | 4.9 | 1.84 | 29.6 | 5.6 | 2.70 | 33.6 | 1.38 | 2.9 | 2.2 | 154 | >216 | >216 | 188 | 194 | 184 | - | Р | | |



Job: Hitchens Block Stage 5

Client: Lander Geotechnical Ltd.

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/SEBA/FRHA |
| Checked By | JRA/DASA/SEBA |
| Approved By | SJA |

| | | | | | | Test Type | | NDM 00 | | | NDM 90 ⁰ | | A | VERAGE ND | м | Solid | Oven | Final (| Corrected | Average Air | | s | hear Vane | Reading (| (kPa) | | | PASS / FAIL | |
|-------|-------|------------|------------|----------------|-----------|-----------|---------------------------------------|----------------------------|------------------|---------------------------------------|----------------------------|------------------|---------------------------------------|----------------------------|------------------|---|-------------------------|--|-----------------------------|-------------|--------------|--------------|--------------|--------------|------------------------------|------------------------------|------------|----------------------|--|
| URN | Tech. | Date | Location | Layer | Material | NDM / SV | Wet Density (t/m ³) | Moisture Content (%) | Air Voids (%) | Wet Density (t/m ³) | Moisture Content (%) | Air Voids (%) | Wet Density (t/m ³) | Moisture Content (%) | Air Voids (%) | Density (t/m ³) Assumed | Moisture content (%) | Oven Dry Density (t/m ³) | Average Air Voids (%) | | Reading 1 | Reading 2 | Reading 3 | Reading 4 | Average SV (4 x Tests) | Average SV (10 X Sets) | Retest URN | (P) Pass (F) Fail | Comments |
| 105.1 | | | | RL 51.93 | | NDM / SV | 1.53 | 23.0 | 25.3 | 1.55 | 23.8 | 23.9 | 1.54 | 23.4 | 24.6 | 2.70 | 24.3 | 1.24 | 24.0 | - | 154 | 166 | 157 | 170 | 162 | 179 | 106.1 | F | Retest of URN 104.2 |
| 105.2 | DASA | 19/03/2021 | REB2 | RL 51.38 | Clay SILT | NDM / SV | 1.82 | 37.0 | 1.5 | 1.81 | 35.5 | 2.9 | 1.82 | 36.3 | 2.2 | 2.70 | 38.1 | 1.32 | 1.1 | 4.1 | >216 | >216 | >216 | >216 | >216 | 179 | - | Р | Retest of URN 105.1 |
| 105.3 | | | | RL 58.75 | | NDM / SV | 1.74 | 46.9 | 0.7 | 1.74 | 44.7 | 1.9 | 1.74 | 45.8 | 1.3 | 2.70 | 52.2 | 1.14 | 0.0 | 4.1 | 170 | 166 | 157 | 160 | 163 | 174 | - | Р | |
| 106.1 | JRA | 22/03/2021 | REB 2 | RL 51.6 | Clay SILT | NDM / SV | 1.82 | 35.1 | 2.6 | 1.82 | 36.1 | 2.2 | 1.82 | 35.6 | 2.4 | 2.70 | 33.2 | 1.37 | 3.9 | 4.2 | 159 | >199 | 173 | 167 | 175 | 169 | - | Р | |
| 107.1 | | | | | | NDM / SV | 1.78 | 35.8 | 4.3 | 1.77 | 34.0 | 6.2 | 1.78 | 34.9 | 5.2 | 2.70 | 39.8 | 1.27 | 2.3 | 4.4 | 185 | 182 | 170 | >191 | 182 | 171 | - | Р | |
| 107.2 | DASA | 24/03/2021 | REB2 | RL ~53.2 | Clay SILT | NDM / SV | 1.75 | 42.7 | 2.3 | 1.74 | 43.2 | 2.8 | 1.74 | 43.0 | 2.5 | 2.70 | 37.8 | 1.26 | 5.4 | 4.7 | >216 | >216 | >216 | >216 | >216 | 171 | - | Р | |
| 109.1 | | / / | | RL 54.10 | | NDM / SV | 1.78 | 36.4 | 4.0 | 1.77 | 36.4 | 4.7 | 1.78 | 36.4 | 4.3 | 2.70 | 33.2 | 1.33 | 6.3 | 5.0 | >216 | >216 | >216 | >216 | >216 | 174 | - | Р | |
| 109.2 | DASA | 26/03/2021 | REB2 | RL 54.00 | Clay SILT | NDM / SV | 1.80 | 33.4 | 5.1 | 1.80 | 31.7 | 6.3 | 1.80 | 32.6 | 5.7 | 2.70 | 45.2 | 1.24 | 0.0 | 4.6 | 166 | 170 | 148 | 163 | 162 | 179 | - | Р | |
| 110.1 | | | | | | NDM / SV | 1.73 | 35.0 | 7.5 | 1.74 | 36.1 | 6.3 | 1.74 | 35.6 | 6.9 | 2.70 | 37.1 | 1.27 | 6.0 | 5.2 | >216 | >216 | >216 | >216 | >216 | 190 | - | Р | |
| 110.2 | | - / / | | ~RL 54.70 | | NDM / SV | 1.77 | 31.2 | 7.7 | 1.76 | 31.3 | 8.4 | 1.77 | 31.3 | 8.1 | 2.70 | 39.1 | 1.27 | 3.3 | 5.2 | >216 | >216 | >216 | >216 | >216 | 192 | - | Р | |
| 110.3 | DASA | 7/04/2021 | | -01.57.40 | Clay SILT | NDM / SV | 1.67 | 41.0 | 7.6 | 1.64 | 41.4 | 9.0 | 1.66 | 41.2 | 8.3 | 2.70 | 47.3 | 1.12 | 5.2 | 3.4 | >216 | >216 | 170 | 173 | 194 | 196 | - | Р | |
| 110.4 | | | | ~RL 57.40 | | NDM / SV | 1.77 | 38.6 | 3.4 | 1.747 | 37.6 | 5.2 | 1.76 | 38.1 | 4.3 | 2.70 | 42.1 | 1.24 | 2.0 | 3.4 | >216 | >216 | >216 | >216 | >216 | 196 | - | Р | |
| 111.1 | | | | | | NDM / SV | 1.78 | 29.9 | 8.2 | 1.761 | 29.5 | 9.5 | 1.77 | 29.7 | 8.8 | 2.70 | 42.7 | 1.24 | 1.0 | 3.5 | >216 | >216 | >216 | >216 | >216 | 201 | - | Р | |
| 111.2 | | | REB2 | ~1m placed | Clay SILT | NDM / SV | 1.87 | 32.4 | 2.0 | 1.861 | 33.0 | 2.0 | 1.87 | 32.7 | 2.0 | 2.70 | 48.5 | 1.26 | 0.0 | 3.2 | >216 | >216 | >216 | >216 | >216 | 205 | - | Р | |
| 111.3 | DASA | 9/04/2021 | | | | NDM / SV | 1.79 | 27.6 | 9.5 | 1.871 | 26.0 | 6.4 | 1.83 | 26.8 | 7.9 | 2.70 | 32.2 | 1.38 | 4.2 | 3.3 | 185 | 188 | 200 | 197 | 193 | 206 | - | Р | |
| 111.4 | | | | | | NDM / SV | 1.80 | 27.5 | 8.9 | 1.81 | 27.3 | 8.5 | 1.80 | 27.4 | 8.7 | 2.70 | 46.1 | 1.24 | 0.0 | 2.8 | >216 | >216 | >216 | >216 | >216 | 206 | - | Р | |
| 111.5 | | | Pond Fill | ~FL | Clay SILT | NDM / SV | 1.86 | 29.4 | 4.7 | 1.822 | 28.3 | 7.2 | 1.84 | 28.9 | 6.0 | 2.70 | 40.9 | 1.31 | 0.0 | 2.2 | >216 | >216 | >216 | >216 | >216 | 206 | - | Р | |
| 112.1 | | 40/04/2024 | 0500 | -000 | ci | NDM / SV | 1.92 | 30.7 | 0.4 | 1.92 | 31.2 | 0.1 | 1.92 | 31.0 | 0.3 | 2.70 | 33.1 | 1.44 | 0.0 | 2.2 | >216 | >216 | >216 | >216 | >216 | 211 | | Р | |
| 112.2 | DASA | 19/04/2021 | REB2 | ~800mm from FL | Clay SILT | NDM / SV | 1.90 | 31.4 | 1.0 | 1.877 | 30.4 | 2.9 | 1.89 | 30.9 | 1.9 | 2.70 | 33.0 | 1.42 | 0.5 | 1.6 | >216 | >216 | >216 | >216 | >216 | 211 | - | Р | |
| 113.1 | | 22/04/2024 | 0500 | ~RL 64 | ci | NDM / SV | 1.80 | 31.0 | 6.6 | 1.80 | 30.6 | 7.0 | 1.80 | 30.8 | 6.8 | 2.70 | 38.0 | 1.30 | 2.3 | 1.5 | 185 | 188 | >216 | >216 | 201 | 210 | - | Р | |
| 113.2 | DASA | 23/04/2021 | REB2 | ~RL 62 | Clay SILT | NDM / SV | 1.88 | 31.6 | 1.8 | 1.89 | 31.5 | 1.6 | 1.89 | 31.6 | 1.7 | 2.70 | 35.2 | 1.39 | 0.0 | 1.0 | >216 | >216 | >216 | >216 | >216 | 212 | - | Р | |
| 114.1 | DASA | 20/04/2024 | Pond | ~RL 61.0 | ci | NDM / SV | 1.76 | 33.9 | 6.7 | 1.76 | 31.7 | 8.3 | 1.76 | 32.8 | 7.5 | 2.70 | 36.2 | 1.29 | 5.4 | 1.3 | 185 | 173 | >216 | >216 | 198 | 210 | - | Р | Pond embankment specification provided by Chris (Lander) requires maximum air voids <8%, |
| 114.2 | DASA | 30/04/2021 | Embankment | ~RL 56.0 | Clay SILT | NDM / SV | 1.78 | 34.3 | 5.7 | 1.78 | 31.6 | 7.3 | 1.78 | 33.0 | 6.5 | 2.70 | 36.8 | 1.30 | 4.1 | 1.7 | 170 | 154 | 173 | 185 | 171 | 206 | - | Р | average less than 6%, Shear strength average >140 kPa with a minimum single value of >120 kPa. |
| 115.1 | | 1/05/2024 | 0500 | RL 48.68 | ci | NDM / SV | 1.72 | 35.5 | 7.7 | 1.74 | 33.3 | 8.4 | 1.73 | 34.4 | 8.0 | 2.70 | 39.3 | 1.24 | 5.2 | 2.2 | 170 | 188 | 191 | 200 | >187 | 203 | - | Р | |
| 115.2 | DASA | 4/05/2021 | REB2 | ~ RL 47 | Clay SILT | NDM / SV | 1.84 | 32.7 | 3.4 | 1.80 | 31.9 | 5.8 | 1.82 | 32.3 | 4.6 | 2.70 | 45.2 | 1.25 | 0.0 | 1.8 | 216 | 216 | 188 | 197 | 204 | 204 | - | Р | |



Job: Hitchens Block Stage 5

| Customer: Lander | Geotechnical Ltd |
|------------------|------------------|
|------------------|------------------|

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/FRHA |
| Checked By | SEBA/DASA/JRA |
| Approved By | SJA |

| | | | | | | | | | | | Nuclear De | nsity (Backscat | ter) | | | | | PASS / FAIL | |
|------|-------|--------------|------------------------|-----------------------------------|-----------------|----------|--------|----------------|--------------------------|----------------------------|---|--------------------------|---|--------------------|------------------|-------------------|------------|----------------------|---|
| URN | Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m ³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URN | (P) Pass (F) Fail | Comments |
| 1.1 | | | | | | - | - | - | - | - | - | - | - | - | - | 10 | 2.1 | F | |
| 1.2 | | | | | | - | - | - | - | - | - | - | - | - | - | 11 | 2.2 | F | |
| 1.3 | JRA | 47/40/2040 | 600 | | DI 40.70 | - | - | - | - | - | - | - | - | - | - | 10 | 2.3 | F | Specification of CIV \ge 20 provided by Kyle (Lander). CIV < 20 - Fail. Hardfill had only been track rolled - |
| 1.4 | JRA | 17/10/2019 | SPR | Fill F Toe Key - Hardfill | RL 40.78 | - | - | - | - | - | - | - | - | - | - | 12 | 2.4 | F | Kyle Informed & to be retested tomorrow. |
| 1.5 | | | | | | - | - | - | - | - | - | - | - | - | - | 10 | 2.5 | F | |
| 1.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 10 | 2.6 | F | |
| 2.1 | | | | | | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | Retest of URN 1.1 once compacted with plate compactor |
| 2.2 | | | | | | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | Retest of URN 1.2 once compacted with plate compactor |
| 2.3 | JRA | 18/10/2019 | SPR | Fill F Toe Key - Hardfill | RL 40.78 | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | Retest of URN 1.3 once compacted with plate compactor |
| 2.4 | JIVA | 10/10/2015 | JFK | rin r toe key - narunn | NL 40.78 | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | Retest of URN 1.4 once compacted with plate compactor |
| 2.5 | | | | | | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | Retest of URN 1.5 once compacted with plate compactor |
| 2.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | Retest of URN 1.6 once compacted with plate compactor |
| 10.1 | | | | | | - | - | - | - | - | - | - | - | - | - | 31 | - | Р | |
| 10.2 | | | | | | - | - | - | - | - | - | - | - | - | - | 25 | - | Р | |
| 10.3 | JRA | 21/11/2019 | SPR | Fill 0 Pond Toe Key - | Top of Hardfill | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | |
| 10.4 | 5101 | 21, 11, 2015 | 5.11 | Fill 0 Pond Toe Key - Hardfill | rop of Hardini | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 10.5 | | | | | | - | - | - | - | - | - | - | - | - | - | 29 | - | Р | |
| 10.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 34.1 | | | | | | - | - | 2.26 | 2.14 | 5.8 | 2.09 | 102.2% | - | - | - | 55 | - | Р | Backscatter specification of 95% MDD provided by Kyle (Lander) |
| 34.2 | | | | | | - | - | 2.20 | 2.12 | 4.1 | 2.09 | 101.2% | - | - | - | 48 | - | Р | |
| 34.3 | FRHA | 20/02/2020 | SPR | Fill A - Shear Key | 0.6m Placed | - | - | 2.20 | 2.09 | 5.1 | 2.09 | 99.9% | - | - | - | 28 | - | Р | |
| 34.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | |
| 34.5 | | | | | | - | - | - | - | - | - | - | - | - | - | 43 | - | Р | |
| 34.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 37 | - | Р | |
| 35.1 | | | | | | - | - | 2.21 | 2.06 | 7.3 | 2.09 | 98.7% | - | - | - | 28 | - | Р | |
| 35.2 | | | | | | - | - | 2.28 | 2.12 | 7.1 | 2.09 | 101.6% | - | - | - | 23 | - | Р | |
| 35.3 | | | | | | - | - | 2.20 | 2.10 | 4.7 | 2.09 | 100.5% | - | - | - | 45 | - | Р | |
| 35.4 | FRHA | 21/02/2020 | SPR Fill A - Shear Key | 1.2m Placed | - | - | 2.25 | 2.15 | 4.9 | 2.09 | 102.8% | - | - | - | 50 | - | Р | | |
| 35.5 | | .,, | | | | - | - | - | - | • | - | - | • | - | - | 26 | - | Р | |
| 35.6 | | | | | | - | - | - | - | | - | - | | - | - | 37 | - | Р | |
| 35.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 37 | - | Р | |
| 35.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 41 | - | Р | |



 Job : Hitchens Block Stage 5
 Job #
 1009213.0657.0.0/3

 ICS
 Entered By
 DASA/JRA/FRHA

 Customer: Lander Geotechnical Ltd
 Checked By
 SEBA/DASA/JRA

 Approved By
 SJA

| | | | | | | | | | | | Nuclear De | nsity (Backscat | ter) | | | | | PASS / FAIL | |
|------|-------|------------|------------------|------------------------|-------------|----------|--------|----------------|--------------------------|----------------------------|---|--------------------------|---|--------------------|------------------|-------------------|------------|----------------------|----------|
| URN | Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m ³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URN | (P) Pass (F) Fail | Comments |
| 36.1 | | | | | | - | - | 2.12 | 2.02 | 5.2 | 2.09 | 96.6% | - | - | - | 36 | - | Р | |
| 36.2 | | | | | | - | - | 2.15 | 2.05 | 5.0 | 2.09 | 98.0% | - | - | - | 31 | - | Р | |
| 36.3 | | | | | | - | - | 2.13 | 2.02 | 5.0 | 2.09 | 96.9% | - | - | - | 48 | - | Р | |
| 36.4 | FRHA | 24/02/2020 | SPR | Fill A - Shear Key | 1.8m Placed | - | - | 2.12 | 2.03 | 4.3 | 2.09 | 97.3% | - | - | - | 28 | - | Р | |
| 36.5 | _ | | | | | - | - | - | - | - | - | - | - | - | - | 52 | - | Р | |
| 36.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 61 | - | Р | |
| 36.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 42 | - | Р | |
| 36.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 37.1 | | | | | | - | - | 2.16 | 2.04 | 5.7 | 2.09 | 97.6% | - | - | - | 26 | - | Р | |
| 37.2 | | | | | | - | - | 2.20 | 2.06 | 6.6 | 2.09 | 98.6% | - | - | - | 30 | - | Р | |
| 37.3 | | | | | | - | - | 2.19 | 2.07 | 5.7 | 2.09 | 98.9% | - | - | - | 32 | - | Р | |
| 37.4 | FRHA | 25/02/2020 | SPR | Fill A - Shear Key | 2.4m Placed | - | - | 2.20 | 2.05 | 7.6 | 2.09 | 97.9% | - | - | - | 31 | - | Р | |
| 37.5 | | | | | | - | - | - | - | - | • | - | - | - | - | 32 | - | Р | |
| 37.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 37.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 41 | - | Р | |
| 37.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 38 | - | Р | |
| 38.1 | | | | | | - | - | 2.24 | 2.11 | 6.2 | 2.09 | 101.0% | - | - | - | 36 | - | Р | |
| 38.2 | | | | | | - | - | 2.26 | 2.12 | 6.9 | 2.09 | 101.3% | - | - | - | 31 | - | Р | |
| 38.3 | | | | | | - | - | 2.21 | 2.10 | 5.5 | 2.09 | 100.3% | - | - | - | 40 | - | Р | |
| 38.4 | FRHA | 25/02/2020 | SPR | Fill A - Shear Key | 3.0m Placed | - | - | 2.13 | 2.02 | 5.2 | 2.09 | 96.7% | - | - | - | 28 | - | Р | |
| 38.5 | _ | | | | | - | - | - | - | - | - | - | - | - | - | 29 | - | Р | |
| 38.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 33 | - | Р | |
| 38.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 38.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 40.1 | | | | | | - | - | 2.13 | 2.01 | 5.6 | 2.09 | 96.4% | - | - | - | 49 | - | Р | |
| 40.2 | | | | | | - | - | 2.21 | 2.08 | 6.2 | 2.09 | 99.4% | - | - | - | 57 | - | Р | |
| 40.3 | | | | | | - | - | 2.09 | 2.01 | 4.4 | 2.09 | 95.9% | - | - | - | 60 | - | Р | |
| 40.4 | FRHA | 26/02/2020 | SPR | SPR Fill A - Shear Key | 3.6m Placed | - | - | 2.16 | 2.05 | 5.3 | 2.09 | 98.2% | - | - | - | 30 | - | Р | |
| 40.5 | | | | | | - | - | - | - | - | - | - | - | - | - | 36 | - | Р | |
| 40.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 34 | - | Р | |
| 40.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 40 | - | Р | |
| 40.8 | | | | | | - | - | - | - | | - | - | - | - | - | 41 | - | Р | |



Job : Hitchens Block Stage 5 Job # 1009213.0657.0.0/3 CUstomer: Lander Geotechnical Ltd Entered By DASA/JRA/FRHA Checked By SEBA/DASA/JRA Approved By SJA

| | | | | | | | | | | | Nuclear De | nsity (Backsca | tter) | | | | | PASS / FAIL | |
|--------------|-------|------------|------------------|--------------------|-------------|----------|--------|----------------|--------------------------|----------------------------|---|--------------------------|----------------------------|--------------------|------------------|-------------------|------------|----------------------|----------|
| URN | Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URN | (P) Pass (F) Fail | Comments |
| 41.1 | 1 | | | | | - | - | 2.19 | 2.08 | 5.7 | 2.09 | 99.3% | - | - | - | 27 | - | Р | |
| 41.2 | | | | | | - | - | 2.22 | 2.05 | 8.5 | 2.09 | 98.0% | - | - | - | 27 | - | Р | |
| 41.3 | | | | | | - | - | 2.24 | 2.13 | 5.1 | 2.09 | 102.0% | - | - | - | 51 | - | Р | |
| 41.4 | FRHA | 27/02/2020 | SPR | Fill A - Shear Key | 4.2m Placed | - | - | 2.21 | 2.08 | 6.4 | 2.09 | 99.4% | - | - | - | 38 | - | Р | |
| 41.5 | 4 | | | , | | - | - | - | - | - | - | - | - | - | - | 40 | - | Р | |
| 41.6 | _ | | | | | - | - | - | - | - | - | - | - | - | - | 40 | - | Р | |
| 41.7 | 4 | | | | | - | - | - | - | - | - | - | - | - | - | 36 | - | Р | |
| 41.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 37 | - | Р | |
| 42.1 | | | | | | - | - | 2.14 | 2.01 | 6.2 | 2.09 | 96.4% | - | - | - | 32 | - | Р | |
| 42.2 | 4 | | | | | - | - | 2.12 | 2.01 | 5.5 | 2.09 | 96.2% | - | - | - | 20 | - | Р | |
| 42.3 | 4 | | | | | - | - | 2.23 | 2.08 | 7.1 | 2.09 | 99.5% | - | - | - | 23 | - | Р | |
| 42.4 | FRHA | 2/03/2020 | SPR | Fill A - Shear Key | 4.8m Placed | - | - | 2.22 | 2.09 | 6.2 | 2.09 | 99.9% | - | - | - | 28 | - | Р | |
| 42.5 | 4 | | | | | - | - | - | • | - | - | - | - | - | - | 28 | - | Р | |
| 42.6 | 4 | | | | | - | - | - | - | - | - | - | - | - | - | 29 | - | Р | |
| 42.7 | 4 | | | | | - | - | - | • | - | - | - | - | - | - | 27 | - | Р | |
| 42.8 | | | | | | - | - | - | • | • | - | - | - | - | - | 29 | - | Ρ | |
| 44.1 | 4 | | | | | - | - | 2.23 | 2.07 | 7.6 | 2.09 | 99.3% | - | - | - | 26 | - | Р | |
| 44.2 | 4 | | | | | - | - | 2.20 | 2.06 | 6.9 | 2.09 | 98.3% | - | - | - | 29 | • | Р | |
| 44.3 | - | | | | | - | - | 2.15 | 2.02 | 6.8 | 2.09 | 96.5% | - | - | - | 21 | • | Р | |
| 44.4 | FRHA | 13/03/2020 | SPR | Fill A - Shear Key | 3.6m Placed | - | - | 2.12 | 2.00 | 6.3 | 2.09 | 95.6% | - | - | - | 20 | • | Р | |
| 44.5 | 4 | | | | | - | - | - | - | • | - | - | • | - | - | 24 | - | P | |
| 44.6 | - | | | | | - | - | - | - | - | - | - | • | - | - | 37 | - | P | |
| 44.7 | - | | | | | - | - | - | - | • | - | - | • | - | - | 24 | - | | |
| 44.8 | | | | | | - | - | - | - | | - | - | • | - | - | 20 32 | - | P | |
| 45.1 | - | | | | | | | 2.16 | 2.03 | 6.2 | 2.09 | 97.3% | | | - | | | P | |
| 45.2 45.3 | - | | | | | - | - | 2.19 2.16 | 2.07 2.03 | 5.6 6.3 | 2.09 | 99.2% 97.2% | - | | - | 42 35 | - | Р Р | |
| 45.4 | - | | | | | | | | 2.03 | 6.8 | 2.09 | | | | | 30 | | Р Р | |
| 45.4 | - | | | | | - | - | 2.18 2.18 | 2.04 | 7.5 | 2.09 | 97.8% 96.8% | - | - | - | 43 | - | Р Р | |
| 45.5 | FRHA | 16/03/2020 | 020 SPR | Fill A - Shear Key | 5.4m Placed | - | - | 2.18 | 2.02 | 4.8 | 2.09 | 96.8% | - | - | - | 43 | - | P | |
| 45.7 | + | | | | | - | - | - | - | 4.0 | - | - | | | | 42 27 | | P | |
| 45.8 | + | | | | | - | - | | | | | - | | | | 35 | | Р Р | |
| 45.9 | 1 | | | | | - | - | - | | | - | - | | | | 33 | - | Р Р | |
| 45.9 | 1 | | | | | - | - | | | | - | - | | - | - | 40 | - | Р Р | |
| 45.10 | 1 | | | | | - | | - | | - | - | | - | - | - | 40 | | r | |



| | Job : Hitchens Block Stage 5 | Job # | 1009213.0657.0.0/3 |
|-------|-----------------------------------|-------------|--------------------|
| INICS | | Entered By | DASA/JRA/FRHA |
| | Customer: Lander Geotechnical Ltd | Checked By | SEBA/DASA/JRA |
| | | Approved By | SJA |

| | | | | | | | | | | | Nuclear De | nsity (Backsca | tter) | | | | | PASS / FAIL | | |
|-------|-------|------------|------------------|------------------------|--------------|------------|--------|----------------|--------------------------|----------------------------|---|--------------------------|---|--------------------|------------------|-------------------|------------|----------------------|---|--|
| URN | Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m ³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URN | (P) Pass (F) Fail | Comments | |
| 48.1 | | | | | | - | - | 2.17 | 2.04 | 6.5 | 2.09 | 97.5% | - | - | - | 27 | - | Р | | |
| 48.2 | | | | | | - | - | 2.34 | 2.15 | 8.7 | 2.09 | 103.1% | - | - | - | 35 | - | Р | | |
| 48.3 | | | | | | - | - | 2.13 | 2.03 | 4.7 | 2.09 | 97.1% | - | - | - | 23 | - | Р | | |
| 48.4 | | | | | | - | - | 2.16 | 2.00 | 7.6 | 2.09 | 95.9% | - | - | - | 25 | - | Р | | |
| 48.5 | FRHA | 13/05/2020 | SPR | Fill A - Shear Key | 6.0m Placed | - | - | 2.21 | 2.04 | 8.5 | 2.09 | 97.4% | - | - | - | 20 | - | Р | | |
| 48.6 | | | | | | - | - | 2.31 | 2.11 | 9.6 | 2.09 | 100.8% | - | - | - | 22 | - | Р | | |
| 48.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 23 | - | Р | | |
| 48.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | | |
| 48.9 | | | | | | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | | |
| 50.1 | | | | | | - | - | 2.12 | 2.02 | 4.8 | 2.09 | 96.8% | - | - | - | 28 | - | Р | | |
| 50.2 | | | | | | - | - | 2.28 | 2.16 | 5.5 | 2.09 | 103.5% | - | - | - | 28 | - | Р | | |
| 50.3 | | | | | | - | - | 2.21 | 2.13 | 4.0 | 2.09 | 101.7% | - | - | - | 38 | - | Р | | |
| 50.4 | | | | | | - | - | 2.15 | 2.06 | 4.0 | 2.09 | 98.8% | - | - | - | 36 | - | Р | | |
| 50.5 | | | | | | - | - | 2.09 | 2.00 | 4.2 | 2.09 | 95.9% | - | - | - | 24 | - | Р | | |
| 50.6 | | | | | | - | - | 2.19 | 2.08 | 5.0 | 2.09 | 99.6% | - | - | - | 28 | - | Р | | |
| 50.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 25 | - | Р | | |
| 50.8 | FRHA | 18/05/2020 | SPR | Fill A - Shear Key | 6.4m placed | - | - | - | - | - | - | - | - | - | - | 43 | - | Р | | |
| 50.9 | | | | | | - | - | - | - | - | - | - | - | - | - | 36 | - | Р | | |
| 50.1 | | | | | | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | | |
| 50.11 | | | | | | - | - | - | - | - | - | - | - | - | - | 24 | - | Р | | |
| 50.12 | | | | | | - | - | - | - | - | - | - | - | - | - | 36 | - | Р | | |
| 50.13 | | | | | | - | - | - | - | - | - | - | - | - | - | 33 | - | Р | | |
| 50.14 | | | | | | - | - | - | - | - | - | - | - | - | - | 33 | - | Р | | |
| 50.15 | | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | | |
| 51.1 | | | | | | - | - | - | - | - | - | - | - | - | - | 34 | - | Р | | |
| 51.2 | | | | | | - | - | - | - | - | - | - | - | - | - | 38 | - | Р | | |
| 51.3 | FRHA | 17/06/2020 | GAP65 | P65 Fill A - Shear Key | ~7.4m placed | - | - | - | - | - | - | - | - | - | | 26 | - | Р | CIV Value of >20 requested by Kyle (Lander) on the 16/06/2020 | |
| 51.4 | 1 | | | | | - | - | - | - | - | - | - | - | | | 30 | - | Р | | |
| 67.1 | | | | | | - | - | 2.23 | 2.07 | 7.9 | 2.09 | 99.0% | - | - | - | 23 | - | Р | | |
| 67.2 | 1 | | | | ~1m placed | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | | |
| 67.3 | JRA | 17/12/2020 | SPR | Fill B - REB Shear Key | | - | - | 2.18 | 2.02 | 7.7 | 2.09 | 96.8% | - | - | - | 26 | - | Р | | |
| 67.4 | 1 | | | FIII B - KEB Snear Key | | ~2m placed | - | - | 2.24 | 2.10 | 6.7 | 2.09 | 100.5% | - | - | - | 34 | - | Р | |
| 67.5 | | | | | | - | - | 2.24 | 2.07 | 8.5 | 2.09 | 98.9% | - | - | - | 31 | - | Р | | |



| | Job : Hitchens Block Stage 5 | Job # | 1009213.0657.0.0/3 |
|-------|-----------------------------------|-------------|--------------------|
| INICS | | Entered By | DASA/JRA/FRHA |
| | Customer: Lander Geotechnical Ltd | Checked By | SEBA/DASA/JRA |
| | | Approved By | SJA |

| | | | | | | | | | | | Nuclear De | nsity (Backsca | ter) | | | | | PASS / FAIL | |
|-------|-------|------------|------------------|------------------------|------------------------|----------|--------|----------------|--------------------------|----------------------------|---|--------------------------|---|--------------------|------------------|-------------------|-------------|----------------------|---------------------------|
| URN | Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m ³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URN | (P) Pass (F) Fail | Comments |
| 69.1 | | | | | | - | - | 2.30 | 2.16 | 6.4 | 2.09 | 103.3% | - | - | - | 21 | - | Р | |
| 69.2 | | | | | | - | - | 2.23 | 2.05 | 8.6 | 2.09 | 98.2% | - | - | - | 23 | - | Р | |
| 69.3 | | | | | | - | - | - | - | - | - | - | - | - | - | 27 | - | Р | |
| 69.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 18 | | F | |
| 69.5 | DASA | 11/01/2021 | SPR | Fill B - REB Shear Key | ~3m placed | - | - | - | - | - | - | - | - | - | - | 16 | 70.1 - 70.4 | F | |
| 69.6 | | ,, | | | | - | - | - | - | - | - | - | - | - | - | 14 | | F | |
| 69.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 14 | | F | |
| 69.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | |
| 69.9 | | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 69.10 | | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 70.1 | | | | | | - | - | 2.24 | 2.07 | 8.0 | 2.09 | 99.3% | - | - | - | 25 | - | Р | |
| 70.2 | DASA | 11/01/2021 | SPR | Fill B - REB Shear Key | ~3m placed | - | - | 2.35 | 2.17 | 8.3 | 2.09 | 103.9% | - | - | - | 30 | - | Р | Retest of URN 69.4 - 69.7 |
| 70.3 | | | | Fill B - REB Shear Key | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 70.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 71.1 | | | | | | - | - | 2.09 | 1.92 | 8.9 | 2.09 | 91.7% | - | - | - | - | 71.2 | F | |
| 71.2 | | | | | | - | - | 2.23 | 2.05 | 8.7 | 2.09 | 98.1% | - | - | - | - | - | Р | Retest of URN 71.1 |
| 71.3 | DASA | 12/01/2021 | SPR | Fill B - REB Shear Key | ~2.6 below FL | - | - | 2.22 | 2.03 | 9.8 | 2.09 | 96.9% | - | - | - | - | - | Р | |
| 71.4 | | | | | | - | - | 2.13 | 1.95 | 9.3 | 2.09 | 93.3% | - | - | - | - | 71.5 | F | |
| 71.5 | | | | | | - | - | 2.23 | 2.06 | 8.2 | 2.09 | 98.7% | - | - | - | - | - | Р | Retest of URN 71.4 |
| 72.3 | | | | | | - | - | 2.03 | 1.87 | 8.8 | 2.09 | 89.4% | - | - | - | 21 | | F | |
| 72.4 | | | | | | - | - | 2.10 | 1.91 | 9.8 | 2.09 | 91.5% | - | - | - | 22 | 72.6 - 72.8 | F | |
| 72.5 | | | | | | - | - | 2.08 | 1.92 | 8.0 | 2.09 | 91.9% | - | - | - | 21 | | F | |
| 72.6 | | | | | ~0.5m placed | - | - | 1.70 | 1.52 | 11.8 | 2.09 | 72.8% | - | - | - | 24 | | F | |
| 72.7 | DASA | 12/01/2021 | SPR | Fill B - REB Shear Key | (Hardfill extension | - | - | 1.79 | 1.63 | 10.0 | 2.09 | 77.9% | - | - | - | 27 | 73.1 - 73.2 | F | |
| 72.8 | 5, GA | 12,01/2021 | 5. K | The shee shear key | where old sump was) | - | - | 1.83 | 1.68 | 8.9 | 2.09 | 80.3% | - | - | - | 21 | | F | |
| 72.9 | | | | | , | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 72.10 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 72.11 | | | | | | - | - | - | - | - | - | - | - | - | - | 27 | - | Р | |
| 72.12 | | | | | | | - | - | - | - | - | - | - | - | - | 30 | - | Р | |



Job: Hitchens Block Stage 5

| Customer: Lander Geotechnical Ltd | |
|-----------------------------------|--|
|-----------------------------------|--|

| Job # | 1009213.0657.0.0/3 |
|-------------|--------------------|
| Entered By | DASA/JRA/FRHA |
| Checked By | SEBA/DASA/JRA |
| Approved By | SJA |

| | | | | | | | | | | | Nuclear De | nsity (Backscat | ter) | | | | | PASS / FAIL | |
|------|--------|------------|------------------|------------------------|-------------------------|----------|--------|----------------|--------------------------|----------------------------|---|--------------------------|---|--------------------|------------------|-------------------|------------|----------------------|---------------------------|
| URN | Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m ³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URN | (P) Pass (F) Fail | Comments |
| 73.1 | | | | | ~0.5m placed | - | - | 2.17 | 2.04 | 6.7 | 2.09 | 97.4% | - | - | - | 22 | - | Р | Retest of URN 72.3 - 72.8 |
| 73.2 | | | | | (Hardfill extension | - | - | 2.17 | 2.04 | 6.6 | 2.09 | 97.4% | - | - | - | 24 | - | Р | |
| 73.3 | | | | | where old sump was) | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | |
| 73.4 | DASA | 13/01/2021 | SPR | Fill B - REB Shear Key | , | - | - | - | - | - | - | - | - | - | - | 27 | - | Р | |
| 73.5 | 571571 | 10/01/2021 | 5111 | The shear key | | - | - | 2.18 | 2.05 | 6.5 | 2.09 | 98.1% | - | - | - | 30 | - | Р | |
| 73.6 | | | | | ~2.1m below FL | - | - | 2.32 | 2.16 | 7.4 | 2.09 | 103.5% | - | - | - | 28 | - | Р | |
| 73.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 73.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 74.1 | | | | | | - | - | 2.28 | 2.10 | 8.2 | 2.09 | 100.7% | - | - | - | 34 | - | Р | |
| 74.2 | | | | | ~2.1m below FL | - | - | 2.40 | 2.24 | 7.3 | 2.09 | 107.2% | - | - | - | 40 | - | Р | |
| 74.3 | | | | | | - | - | 2.35 | 2.16 | 9.1 | 2.09 | 103.2% | - | - | - | 36 | - | Р | |
| 74.4 | JRA | 13/01/2021 | SPR | Fill B - REB Shear Key | ~1m placed (lower | - | - | 2.35 | 2.20 | 7.0 | 2.09 | 105.2% | - | - | - | 36 | - | Р | |
| 74.5 | | | | | extension area) | - | - | 2.23 | 2.06 | 8.3 | 2.09 | 98.6% | - | - | - | 30 | - | Р | |
| 74.6 | | | | | ~1.5m placed (lower | - | - | 2.31 | 2.14 | 7.8 | 2.09 | 102.5% | - | - | - | 32 | - | Р | |
| 74.7 | | | | | extension area) | - | - | 2.36 | 2.21 | 7.1 | 2.09 | 105.5% | - | - | - | 39 | - | Р | |
| 75.1 | | | | | | - | - | 2.22 | 2.07 | 7.4 | 2.09 | 99.1% | - | - | - | 28 | - | Р | |
| 75.2 | DASA | 14/01/2021 | SPR | Fill B - REB Shear Key | ~2.6m below FL | - | - | 2.24 | 2.05 | 9.1 | 2.09 | 98.0% | - | - | - | 30 | - | Р | |
| 75.3 | DASA | 14/01/2021 | JFK | FILD - KED SHEAL KEY | (Extension area) | - | - | - | - | - | - | - | - | - | - | 27 | - | Р | |
| 75.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 76.1 | | | | | | - | - | 2.31 | 2.14 | 7.8 | 2.09 | 102.4% | - | - | - | 32 | - | Р | |
| 76.2 | DASA | 14/01/2021 | SPR | Fill B - REB Shear Key | ~2.1m below FL | - | - | 2.29 | 2.10 | 9.1 | 2.09 | 100.6% | - | - | - | 28 | - | Р | |
| 76.3 | DASA | 14/01/2021 | SPR | FIII B - REB SHear Key | (Extension Area) | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 76.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 78.1 | | | | | | - | - | 2.32 | 2.10 | 10.5 | 2.09 | 100.3% | - | - | - | 23 | - | Р | |
| 78.2 | JRA | 18/01/2021 | SPR | Fill B - REB Shear Key | ~1m Placed (Ex | - | - | 2.21 | 2.02 | 9.7 | 2.09 | 96.5% | - | - | - | 20 | - | Р | |
| 78.3 | JKA | 18/01/2021 | SPR | FIII B - REB Shear Key | Ramp Extension Area) | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | |
| 78.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | |
| 79.1 | | | | | | - | - | 2.24 | 2.05 | 9.0 | 2.09 | 98.2% | - | - | - | 25 | - | Р | |
| 79.2 | | | | | ~1.5m Placed (Ex | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 79.3 | | | | | Ramp Extension Area) | - | - | 2.24 | 2.06 | 8.6 | 2.09 | 98.5% | - | - | - | 30 | - | Р | |
| 79.4 | 1 | | | Fill B - REB Shear Key | | - | - | - | - | - | - | - | - | - | - | 25 | - | Р | |
| 79.5 | JRA | 18/01/2021 | SPR | | ~0.7m below Hardfill | - | - | 2.21 | 2.03 | 8.8 | 2.09 | 97.0% | - | - | - | 32 | - | Р | |
| 79.6 | 1 | | | | FL | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | |
| 79.7 | 1 | | | | | - | - | 2.22 | 2.05 | 8.6 | 2.09 | 97.9% | - | - | - | 36 | - | Р | |
| 79.8 | 1 | | | | ~0.5m below Hardfill | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | |
| 79.9 | 1 | | | | FL | - | - | 2.23 | 2.03 | 9.8 | 2.09 | 97.2% | - | - | - | 32 | - | Р | |



 Job : Hitchens Block Stage 5
 Job #
 1009213.0657.0.0/3

 Entered By
 DASA/JRA/FRHA

 Customer: Lander Geotechnical Ltd
 Checked By
 SEBA/DASA/JRA

 Approved By
 SJA

| | | | | | | | | | | | Nuclear De | nsity (Backsca | ter) | | | | | PASS / FAIL | |
|-------|-------|------------|------------------|------------------------|----------------------------------|----------|--------|----------------|--------------------------|----------------------------|---|--------------------------|---|--------------------|------------------|-------------------|------------|----------------------|----------|
| URN | Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m ³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URN | (P) Pass (F) Fail | Comments |
| 80.1 | | | | | | - | - | 2.31 | 2.09 | 10.5 | 2.09 | 100.1% | - | - | - | 21 | - | Р | |
| 80.2 | JRA | 19/01/2021 | SPR | Fill B - REB Shear Key | ~2m Placed (Ex Ramp Extension | - | - | 2.56 | 2.35 | 9.0 | 2.09 | 112.3% | - | - | - | 25 | - | Р | |
| 80.3 | 5101 | 13/01/2021 | 5.11 | The bolication | Area) | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 80.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 81.1 | | | | | | - | - | 2.24 | 2.06 | 8.6 | 2.09 | 98.6% | - | - | - | 24 | - | Р | |
| 81.2 | | | | | 0 Far halan haadfill | - | - | 2.22 | 2.05 | 8.2 | 2.09 | 98.2% | - | - | - | 25 | - | Р | |
| 81.3 | DASA | 21/01/2021 | SPR | Fill B - REB Shear Key | 0.5m below hardfill FL | - | - | - | - | - | - | - | - | - | - | 27 | - | Р | |
| 81.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | |
| 81.5 | | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 82.1 | | | | | | - | - | 2.22 | 2.04 | 8.6 | 2.09 | 97.6% | - | - | - | 28 | - | Р | |
| 82.2 | | | | | | - | - | 2.23 | 2.04 | 9.0 | 2.09 | 97.7% | - | - | - | 30 | - | Р | |
| 82.3 | | | | | | - | - | 2.24 | 2.06 | 8.9 | 2.09 | 98.6% | - | - | - | 33 | - | Р | |
| 82.4 | | | | | | - | - | 2.25 | 2.06 | 9.0 | 2.09 | 98.8% | - | - | - | 34 | - | Р | |
| 82.5 | | | | | | - | - | 2.24 | 2.05 | 9.2 | 2.09 | 97.9% | - | - | - | 27 | - | Р | |
| 82.6 | DASA | 22/01/2021 | SPR | Fill B - REB Shear Key | FL | - | - | - | - | - | - | - | - | - | - | 31 | - | Р | |
| 82.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | |
| 82.8 | | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 82.9 | | | | | | - | - | - | - | - | - | - | - | - | - | 24 | - | Р | |
| 82.10 | | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 82.11 | | | | | | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | |
| 82.12 | | | | | | - | - | - | - | - | - | - | - | - | - | 34 | - | Р | |
| 87.1 | | | | | | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | |
| 87.2 | | | | | | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | |
| 87.3 | | | | | | - | - | - | - | - | - | - | - | - | - | 24 | - | Р | |
| 87.4 | | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 87.5 | | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 87.6 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 87.7 | | | | | | - | - | - | - | - | - | - | - | - | - | 24 | - | Р | |
| 87.8 | DASA | 4/02/2021 | GAP65 | Fill B - REB Shear Key | 500mm placed | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 87.9 | | , , , , , | | , | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 87.10 | | | | | | - | - | - | - | - | - | - | - | | - | 24 | - | Р | |
| 87.11 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 87.12 | | | | | | - | - | - | - | - | - | - | - | - | - | 25 | - | Р | |
| 87.13 | | | | | | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | |
| 87.14 | | | | | | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | |
| 87.15 | | | | | | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | |
| 87.16 | | | | | | - | - | - | - | - | - | - | - | - | | 24 | - | Р | |

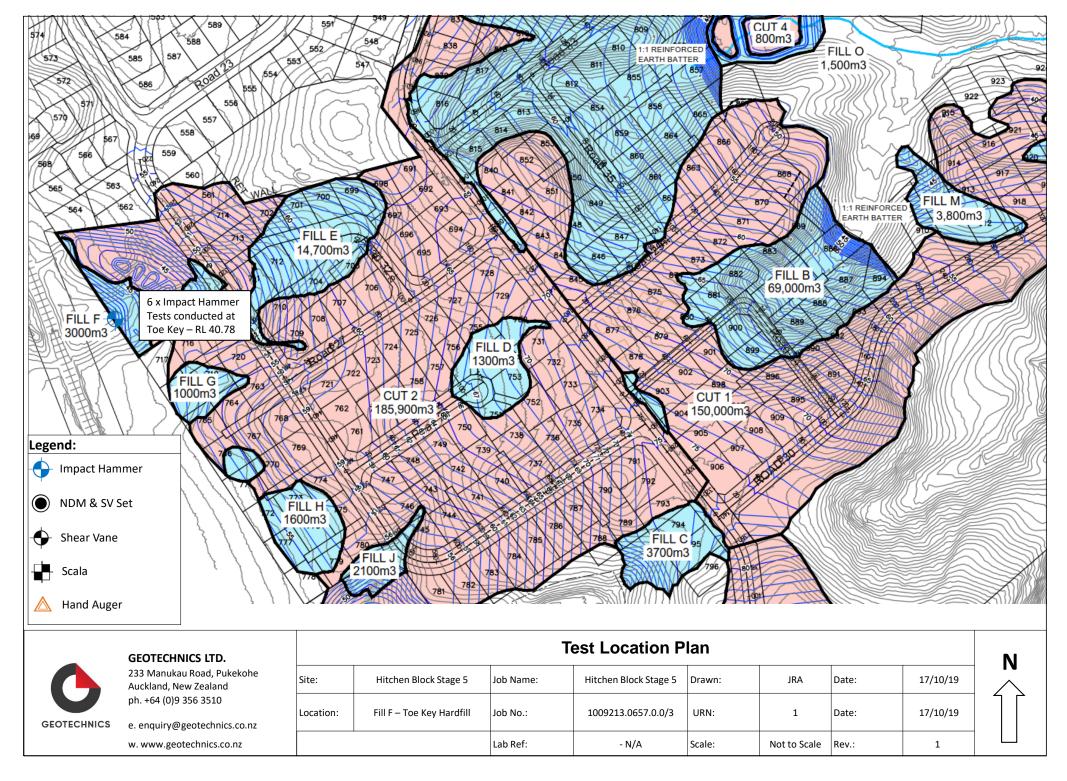


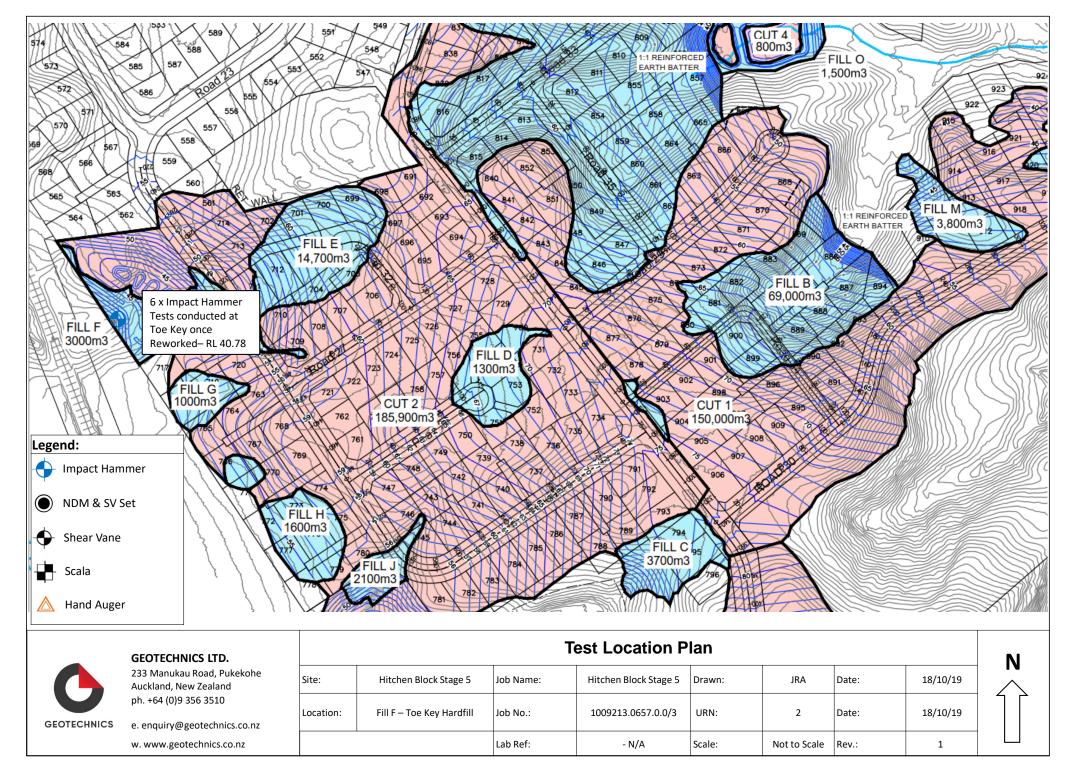
| | Job : Hitchens Block Stage 5 | Job # | 1009213.0657.0.0/3 |
|-------|-----------------------------------|-------------|--------------------|
| HNICS | | Entered By | DASA/JRA/FRHA |
| писз | Customer: Lander Geotechnical Ltd | Checked By | SEBA/DASA/JRA |
| | | Approved By | SJA |

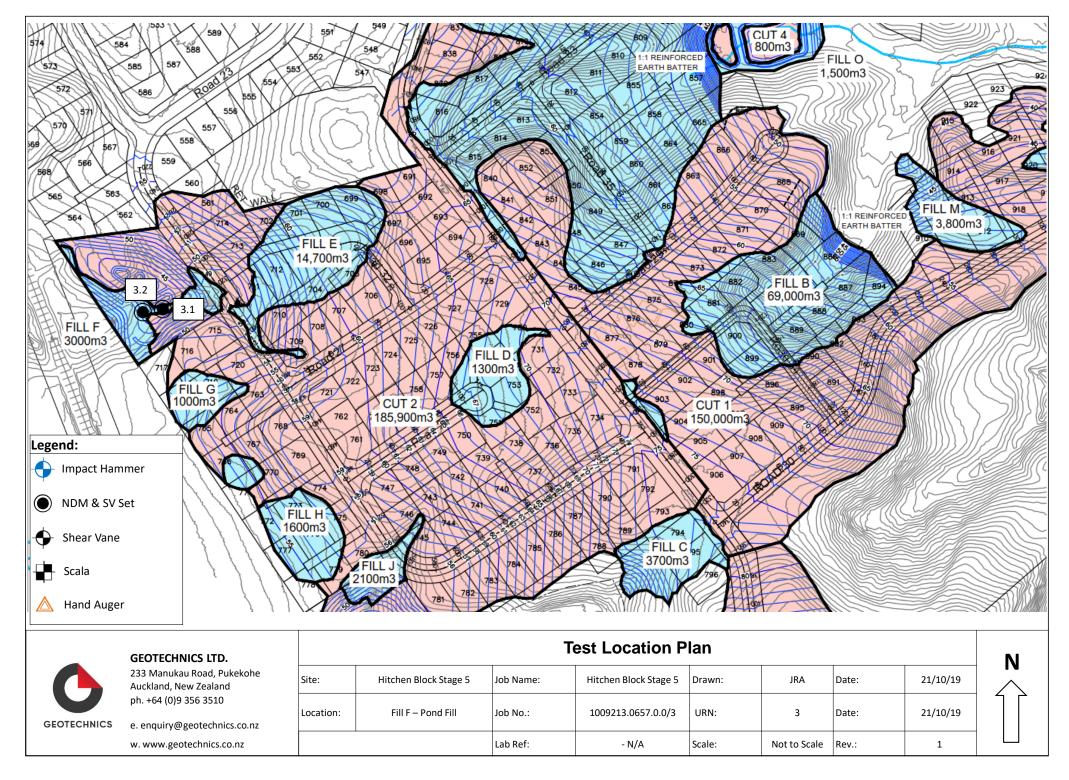
| | | | | | | | | | | | Nuclear De | nsity (Backsca | tter) | | | | | PASS / FAIL | |
|----|----------|--------------|------------------|------------------------|--------------|----------|--------|----------------|--------------------------|----------------------------|---|--------------------------|---|--------------------|------------------|-------------------|------------|------------------------|-----------------|
| U | N Tech. | Date | Material Type | Location | Layer | Chainage | Offset | Wet Density | Dry Density (t/m³) | Moisture Content (%) | Maximum Dry Density (t/m ³) | % Maximum Dry Density | Solid Density (t/m ³) | % Solid Density | % Total Voids | Impact Value 1 | Retest URI | l (P) Pass (F) Fail | Comments |
| 88 | 8.1 | | | | | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | |
| 88 | 3.2 | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 88 | 1.3 | | | | | - | - | - | - | - | - | - | - | - | - | 24 | - | Р | |
| 88 | 3.4 | | | | | - | - | - | - | - | - | - | - | - | - | 27 | - | Р | |
| 88 | DASA | 5/02/2021 | GAP65 | Fill B - REB Shear Key | 500mm placed | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | |
| 88 | .6 | | | , | | - | - | - | - | - | - | - | - | - | - | 21 | - | Р | |
| 88 | | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | |
| 88 | | | | | | - | - | - | - | - | - | - | - | - | - | 27 | - | Р | |
| 88 | 8.9 | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 88 | .10 | | | | | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | |
| 89 | 0.1 | | | | | - | - | - | - | - | - | - | - | - | - | 22 | - | Р | |
| 89 | | | | | | - | - | - | - | - | - | - | - | - | - | 24 | - | Р | |
| 89 | 0.3 | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 89 | 0.4 | | GAP65 | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 89 | DASA | 9/02/2021 | | Fill B - REB Shear Key | FL | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 89 | 0.6 | | | | | - | - | - | - | - | - | - | - | - | - | 24 | - | Р | |
| 89 | .7 | | | | | - | - | - | - | - | - | - | - | - | - | 18 | 90.1 | F | |
| 89 | .8 | | | | | - | - | - | - | - | - | - | - | - | - | 16 | 90.2 | F | |
| 89 | 0.9 | | | | | - | - | - | - | - | - | - | - | - | - | 14 | 90.3 | F | |
| 89 | .10 | | | | | - | - | - | - | - | - | - | - | - | - | 14 | 90.4 | F | |
| 90 | 0.1 | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | Retest of 89.7 |
| 90 | 0.2 | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | Retest of 89.8 |
| 90 | 0.3 | | | | | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | Retest of 89.9 |
| 90 | 0.4 | | | | | - | - | - | - | - | - | - | - | - | - | 26 | - | Р | Retest of 89.10 |
| 90 | 0.5 DASA | 10/02/2021 | GAP65 | Fill B - REB Shear Key | FL | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 90 | 0.6 | 10, 02, 2021 | 0, 05 | The billed her | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 90 |).7 | | | | | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | |
| 90 | 0.8 | | | | | - | - | - | - | - | - | - | - | - | - | 34 | - | Р | |
| 90 | .9 | | | | | - | - | - | - | - | - | - | - | - | - | 30 | - | Р | |
| 90 | .10 | | | | | - | - | - | - | - | - | - | - | - | - | 28 | - | Р | |
| 10 | 8.1 | | | | | - | - | 2.21 | 2.00 | 10.6 | 2.09 | 95.7% | - | - | - | 20 | - | Р | |
| 10 | 8.2 | 25/03/2021 | SPR | Pond Fill | 51 | - | - | - | - | - | - | - | - | - | - | 20 | - | Р | |
| 10 | 8.3 | 25/05/2021 | SPR | PUILU FIII | FL | - | - | - | - | - | - | - | - | - | - | 29 | - | Р | |
| 10 | 8.4 | | | | | - | - | - | - | - | - | - | - | - | - | 32 | - | Р | |

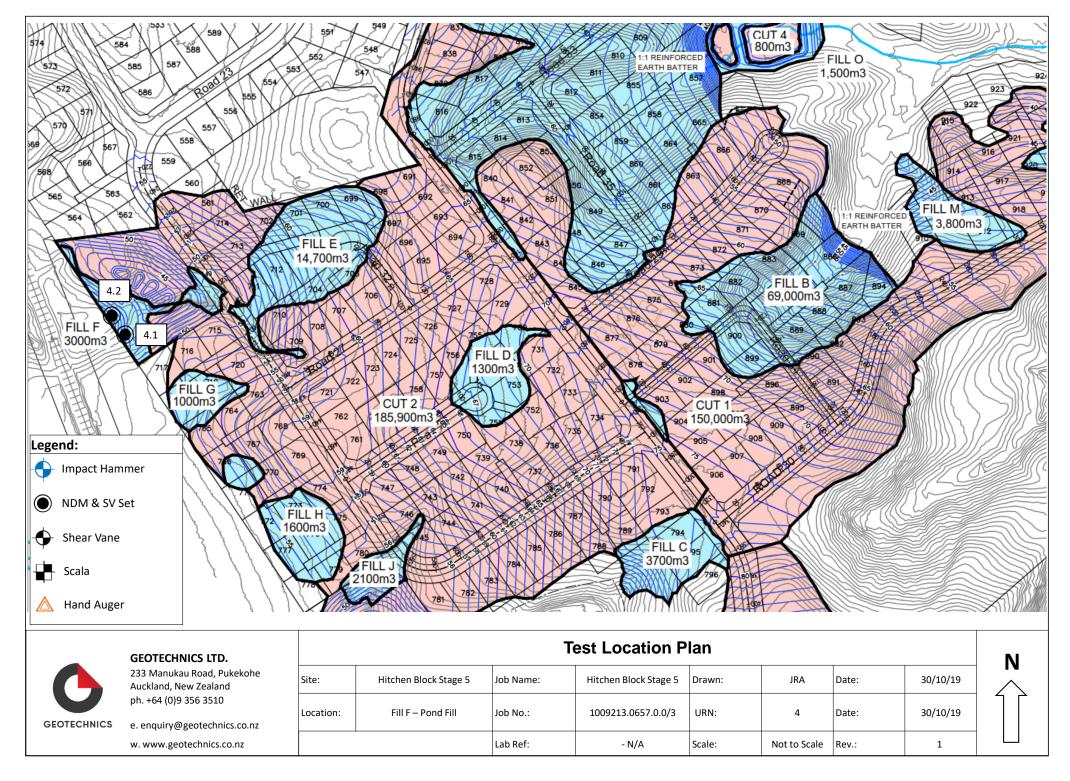
| | | GE | от | ECHI | | S | | Lander Ge | lock Stage otechnical ensity Test | | | | | | | | Job # Entered By: Checked By: Approved By: | 1009213.0657.0.0/3 FRHA SEBA SJA |
|----------------------------|--------|------------|-----------|------------------|--------|-------------------|-------------------------|---------------------------------|---|---|---------------------------------|--|---|---------------------------------------|---------------------------|--------------|---|--|
| URN | Tech. | Date | Location | Location (A / B) | Layer | Material Type | Number (Cumu VIBE | of Passes Ilative) STATIC | Nuclear Wet Density (t/m ³) | Nuclear Dry Density (t/m ³) | Nuclear water content (%) | Field Maximum Dry Density (MDD) (t/m ³) | Percentage maximum Dry Density MDD (%) | Percentage of Solid Density (%) | Impact Value 1 (IV) | Pass / Fail | Retest URN | Comments |
| 33.3 | | | | | | | 1 | STATIC | 1.77 | 1.65 | 7.4 | 2.09 | 78.7% | - | 20 | - | - | |
| 33.5 | | | | | | | 2 | | 1.89 | 1.79 | 5.7 | 2.09 | 85.5% | - | 13 | - | - | |
| 33.7 | | | | | | | 3 | | 1.78 | 1.67 | 6.2 | 2.09 | 80.0% | - | 14 | - | - | |
| 33.9 | | | | | | | | 5 | 2.09 | 1.95 | 7.1 | 2.09 | 93.5% | - | 23 | - | - | |
| 33.1 | FRHA | 19/02/2020 | Shear Key | Location A | ~300mm | SPR | | 7 | 2.13 | 1.99 | 6.7 | 2.09 | 95.4% | - | 28 | - | - | |
| 33.13 | | | | | placed | | | 9 | 2.15 | 2.04 | 5.5 | 2.09 | 97.5% | - | 31 | - | - | |
| 33.15 | | | | | | | | 11 | 2.17 | 2.04 | 6.4 | 2.09 | 97.6% | - | 35 | - | - | |
| 33.17 | | | | | | | | 13 | 2.19 | 2.08 | 5.2 | 2.09 | 99.4% | - | 35 | - | - | |
| 33.19 | | | | | | | | 15 | 2.21 | 2.09 | 5.7 | 2.09 | 100.0% | - | 35 | - | - | Maximum field dry density achieved of 2.09 t/m ³ |
| 33.21 | | | | | | | | 17 | 2.21 | 2.07 | 7.0 | 2.09 | 98.9% | - | 34 | - | - | |
| | | | | | | Density vs. C | umulative N | umber Of Pa | sses | | | | | | | Plateau Te | st Details | |
| | 2.20 | | | | | | | | | | | | | | | Plant | | 17 Tonne Single Drum Roller |
| | | | | | | | | | | | | | | | | Material Typ | pe | SPR |
| (e | 2.10 | | | | | | | | 2.08 | 2.09 | | 2.07 | | | | Supplier | | Smythe's Quarry |
| :y (t/m | 2.00 | | | | | .99 | | 2.04 | | | 2 | | | | | Layer Thickr | iess | ~300mm |
| Densit | 1.90 | | | 1.95 | 1 | | | | | | | | - E Vibratio | on 🖛 Static | | Subgrade Ty | pe | Unknown |
| Nuclear Dry Density (t/m³) | 1.80 - | | | | | | | | | | | | | | | Definition o | f # of Passes | 1 Pass = 2 passes above the test area with plant (i.e. there & back) |
| Nuc | | / | 1.79 | / | | | | | | | | | | | | Comments | | |
| | 1.70 | 1 65 | 1.67 | 1.67 | 1 | | | 1 | | 1 | 1 | | | | | | | |
| | 0 | | 2 | 4 | 6 | 8 | 10 | 12 | | 14 | 16 | 18 | | | | | | |
| | | | | | | Cumulative Number | of Passes (#) | | | | | | | | | | | |

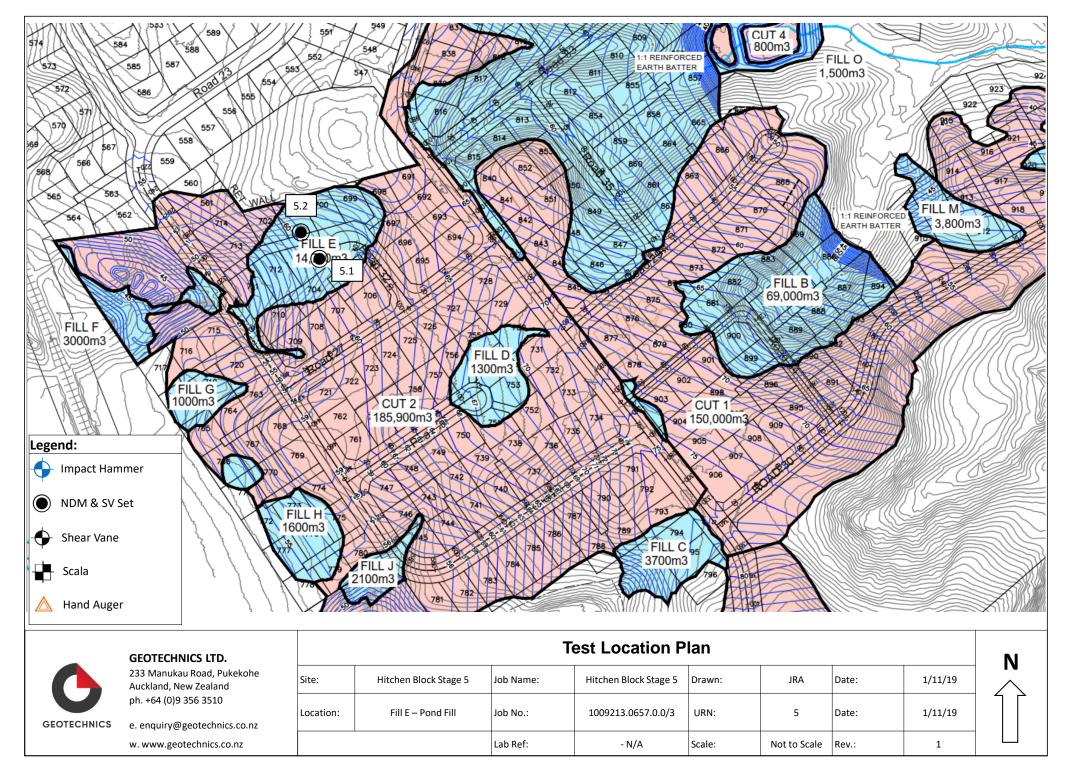
| | 3 | GE | ΟΤΕ | CHN | ICS | | Client: | Lander G | Block Stag eotechnical eensity Test | | | | | | | - | Job # Entered By: Checked By: Approved By: | 1009213.0657.0.0/3 FRHA SEBA SJA |
|----------------------------|-------|------------|------------|------------------|------------------|-------------------|-----------------|------------|---|---|---------------------------------|--|---|---------------------------------------|---------------------------|---------------|---|--|
| URN | Tech. | Date | Location | Location (A / B) | Layer | Material Type | Number (Cumu | ilative) | Nuclear Wet Density (t/m ³) | Nuclear Dry Density (t/m ³) | Nuclear water content (%) | Field Maximum Dry Density (MDD) (t/m ³) | Percentage maximum Dry Density MDD (%) | Percentage of Solid Density (%) | Impact Value 1 (IV) | Pass / Fail | Retest URN | Comments |
| 33.4 | | | | | | | VIBE 1 | STATIC | 1.70 | 1.62 | 4.9 | 2.09 | 77.7% | - | 11 | - | - | |
| 33.6 | | | | | | | 2 | | 1.78 | 1.70 | 4.9 | 2.09 | 81.2% | - | 14 | - | - | |
| 33.8 | | | | | | | 3 | | 1.75 | 1.66 | 5.4 | 2.09 | 79.5% | - | 16 | - | - | |
| 3.10 | | | | | | | | 5 | 1.94 | 1.84 | 5.5 | 2.09 | 87.9% | - | 24 | - | - | |
| 3.12 | FRHA | 19/02/2020 | Shear Key | Location B | ~300mm placed | SPR | | 7 | 2.01 | 1.92 | 4.6 | 2.09 | 91.9% | - | 23 | - | - | |
| 3.14 | FNDA | 19/02/2020 | Sileal Key | LOCATION B | | | | 9 | 2.03 | 1.91 | 6.2 | 2.09 | 91.5% | - | 24 | - | - | |
| 3.16 | | | | | | | | 11 | 2.06 | 1.96 | 4.9 | 2.09 | 93.8% | - | 27 | - | - | |
| 3.18 | | | | | | | | 13 | 2.10 | 2.01 | 4.6 | 2.09 | 96.1% | - | 28 | - | - | |
| 3.20 | | | | | | | | 15 | 2.07 | 1.98 | 4.7 | 2.09 | 94.8% | - | 27 | - | - | |
| 33.22 | | | | | | | | 17 | 2.07 | 1.98 | 4.7 | 2.09 | 94.6% | - | 27 | - | - | |
| | | | | | | Density vs. Cu | imulative Nu | mber Of Pa | isses | | | | | | | Plateau Te | st Details | |
| | 2.05 | | | | | | | | | | | | | | | Plant | | 17 Tonne Single Drum Roller |
| | 2.00 | | | | | | | | 2.01 | | | | | | | Material Typ | e | SPR |
| (_e u | 1.95 | | | | | \sim | 1 | .96 | | 1.98 | 1 | .98 | | | | Supplier | | Smythe's Quarry |
| ity (t/r | 1.90 | | | | 1. | 92 1.91 | | | | | | | | | | Layer Thickn | ess | ~300mm |
| Nuclear Dry Density (t/m³) | 1.85 | | | 1.84 | | | | | | | | | | on 🖛 Static | | Subgrade Ty | ре | Unknown |
| lear Dr | 1.80 | 1.04 | | | | | | | | | | | | | | Definition of | # of Passes | 1 Pass = 2 passes above the test area with plant (i. there & back) |
| Nuc | 1.70 | | 1.70 | / | | | | | | | | | | | | Comments | | |
| | 1.65 | 1.62 | 1.66 | .66 | | | | | | | | | | | | | | |
| | 1.60 | | 2 | 4 | 6 | 8 | 10 | 12 | | 14 | 16 | 18 | | | | | | |
| | | | | | | Cumulative Number | of Passes (#) | | | | | | | | | | | |

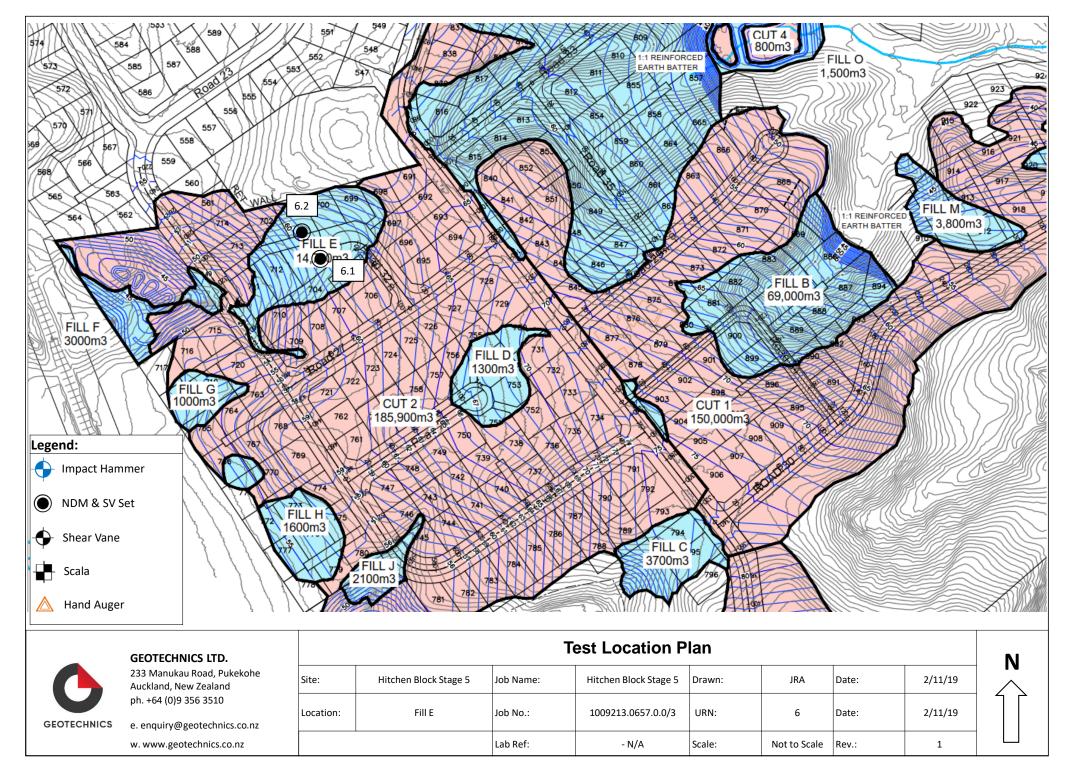


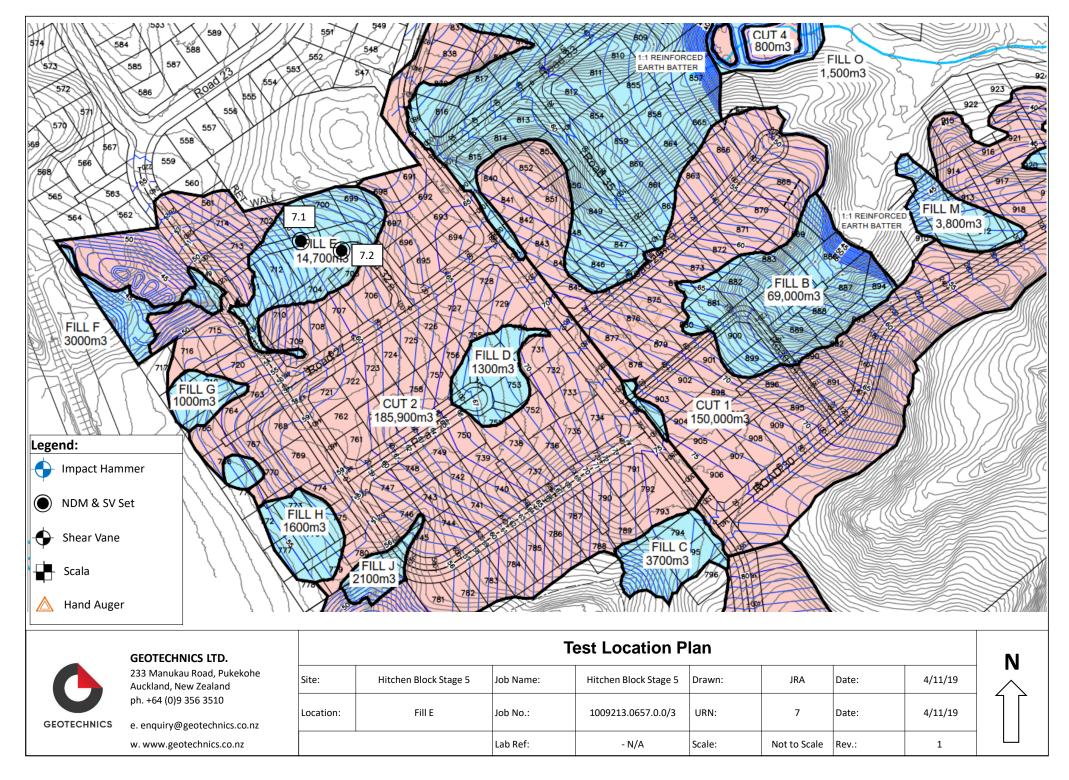


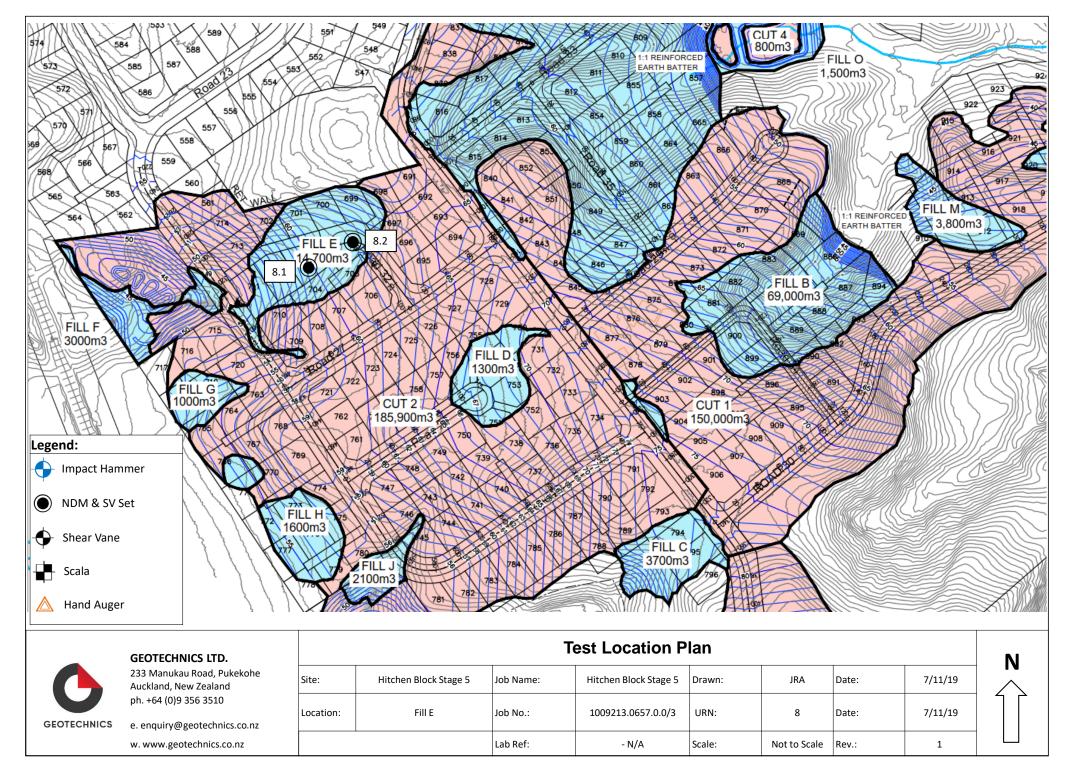


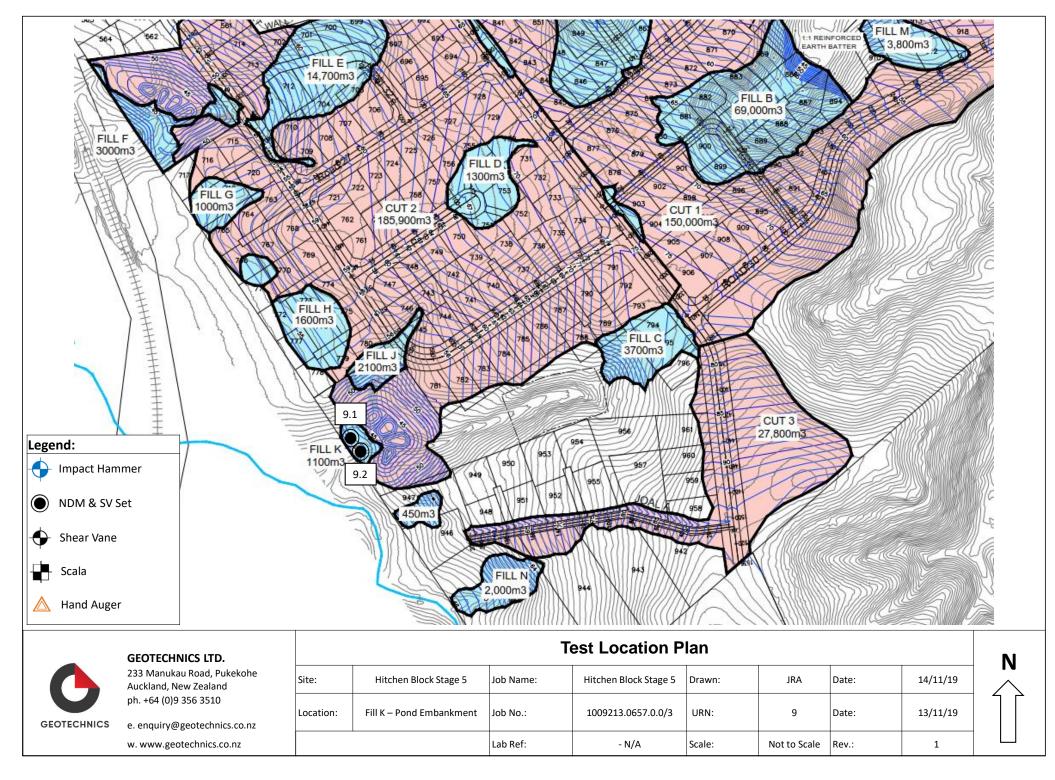


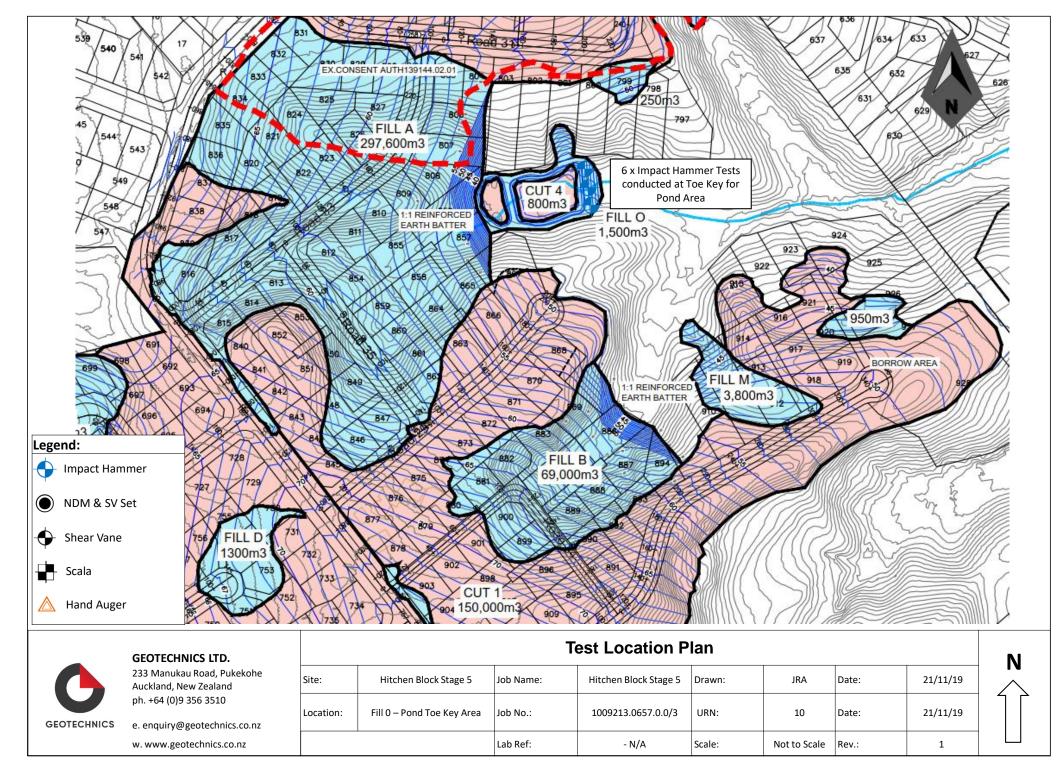


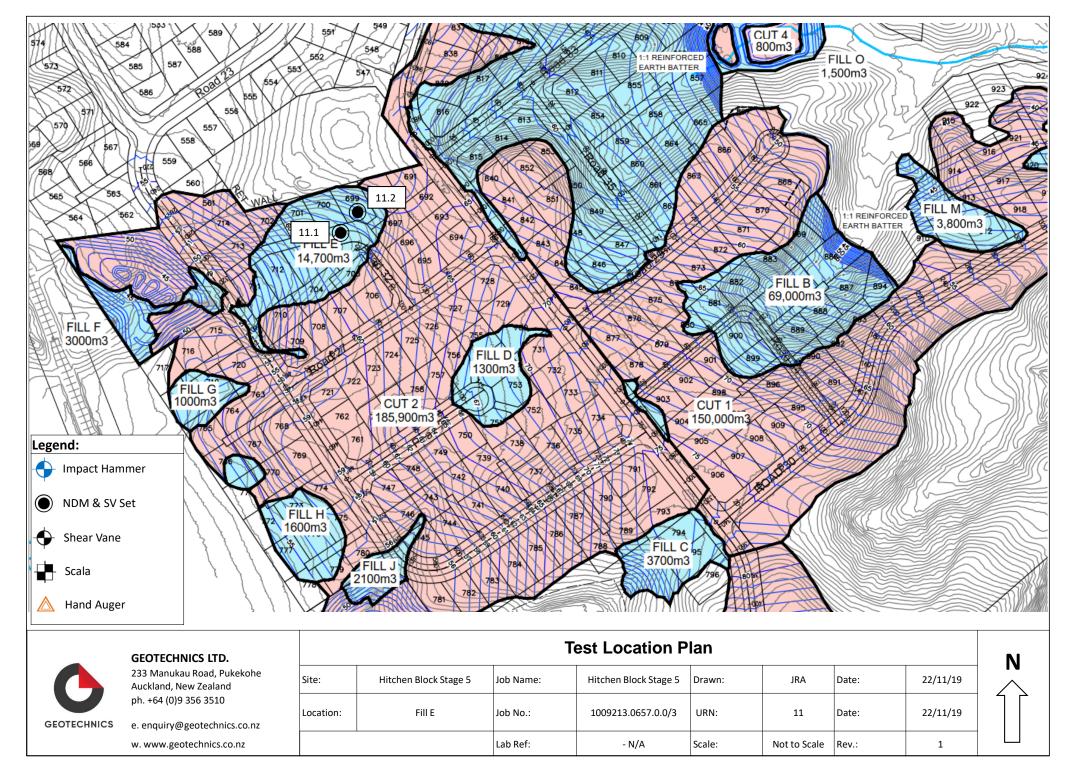


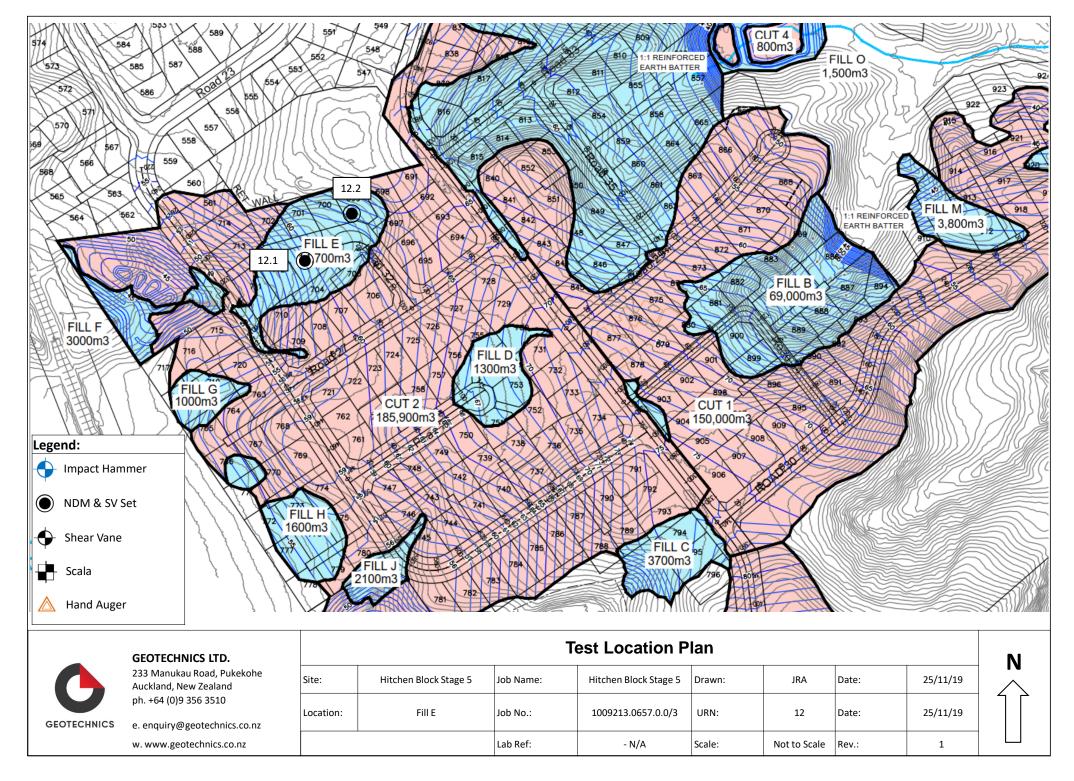


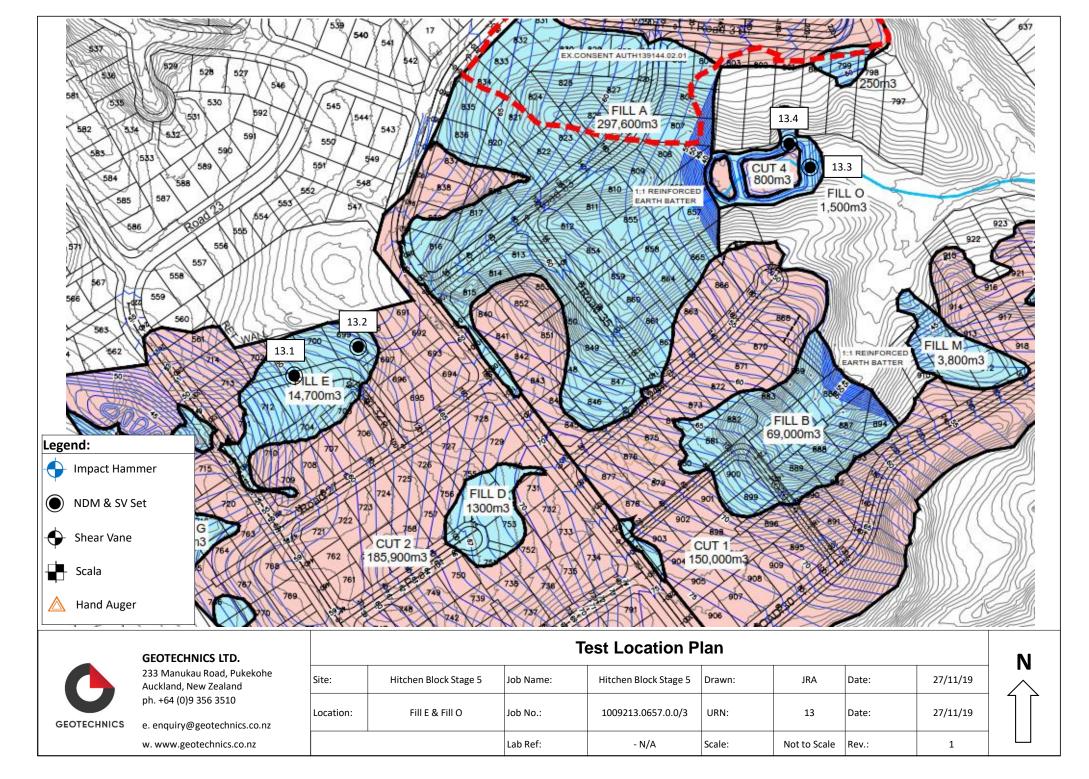


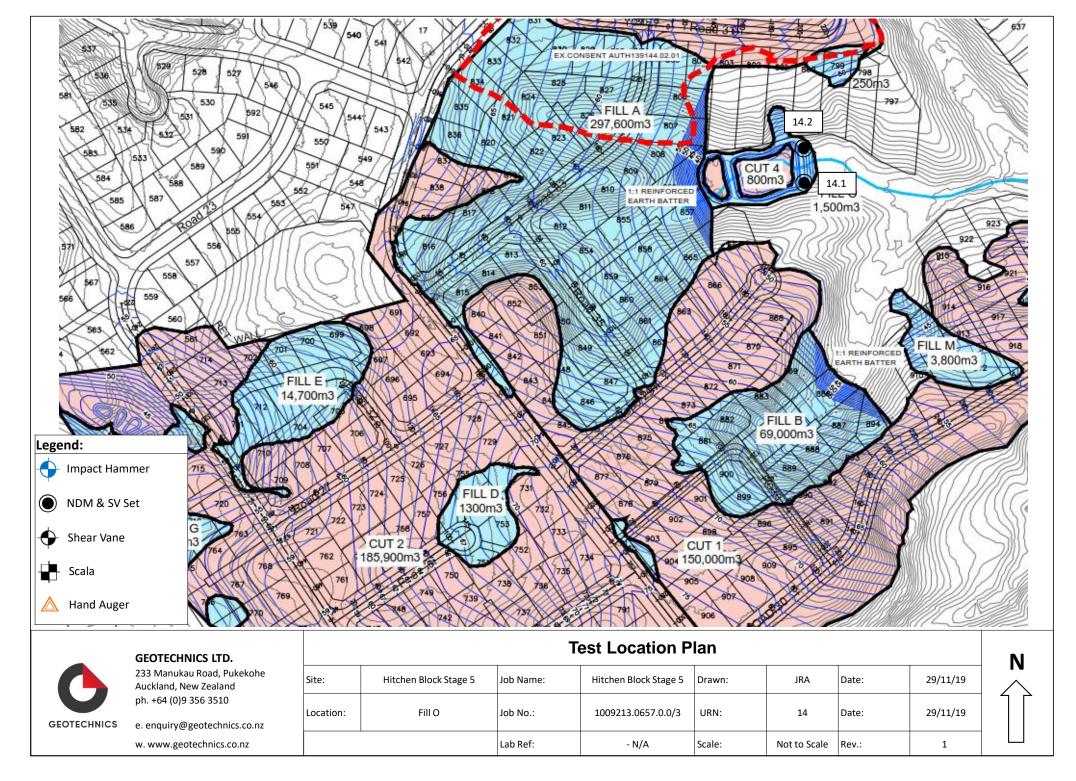


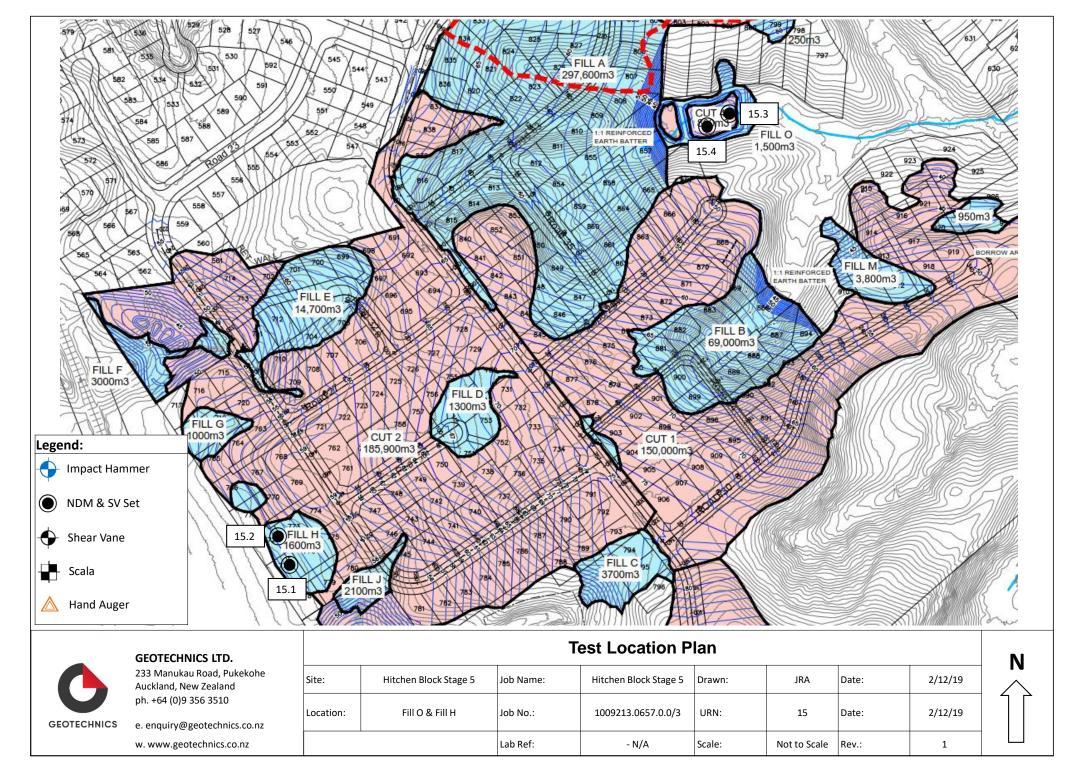


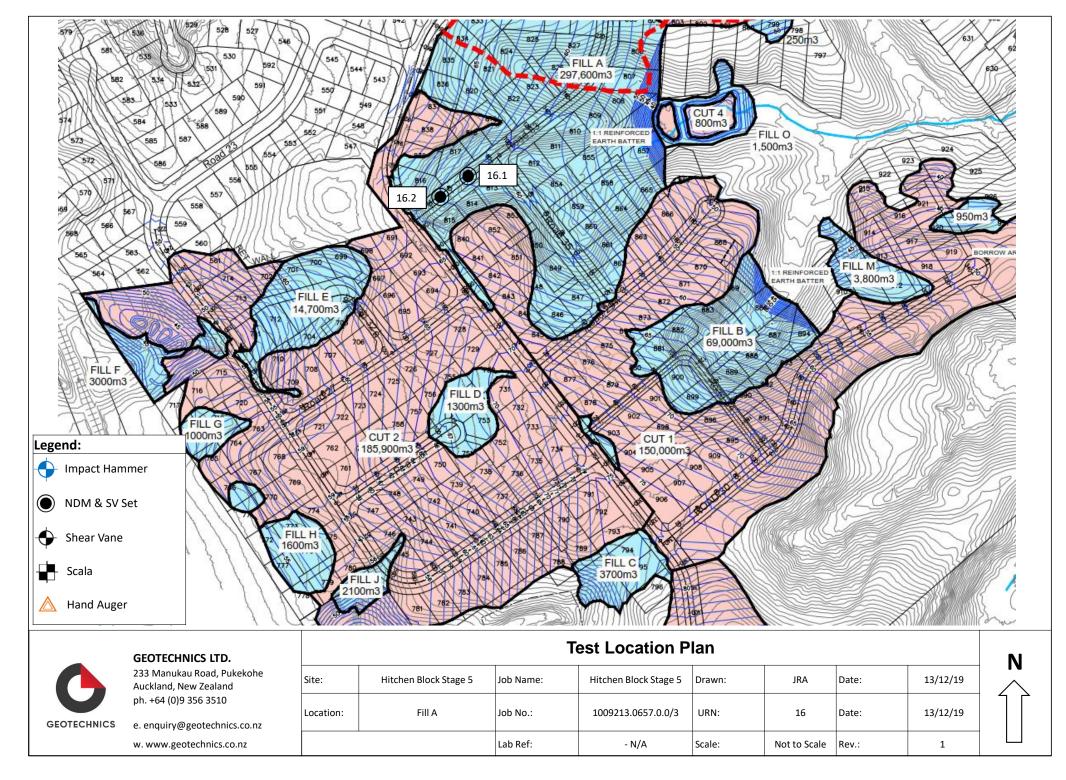


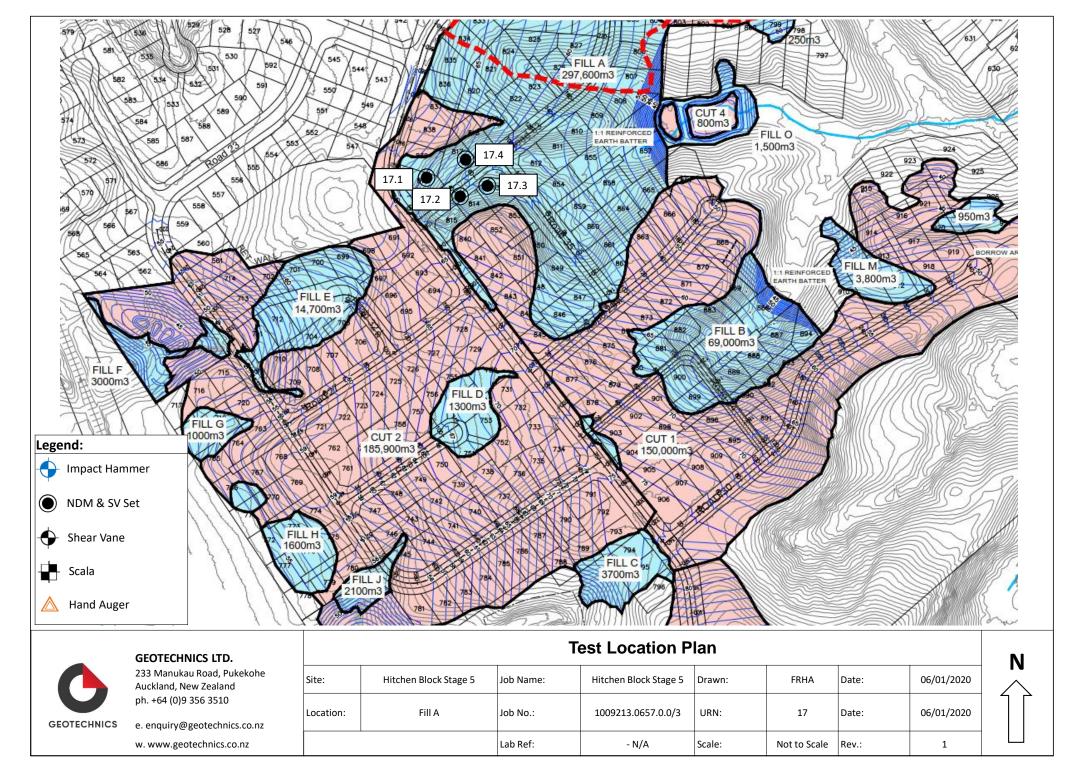


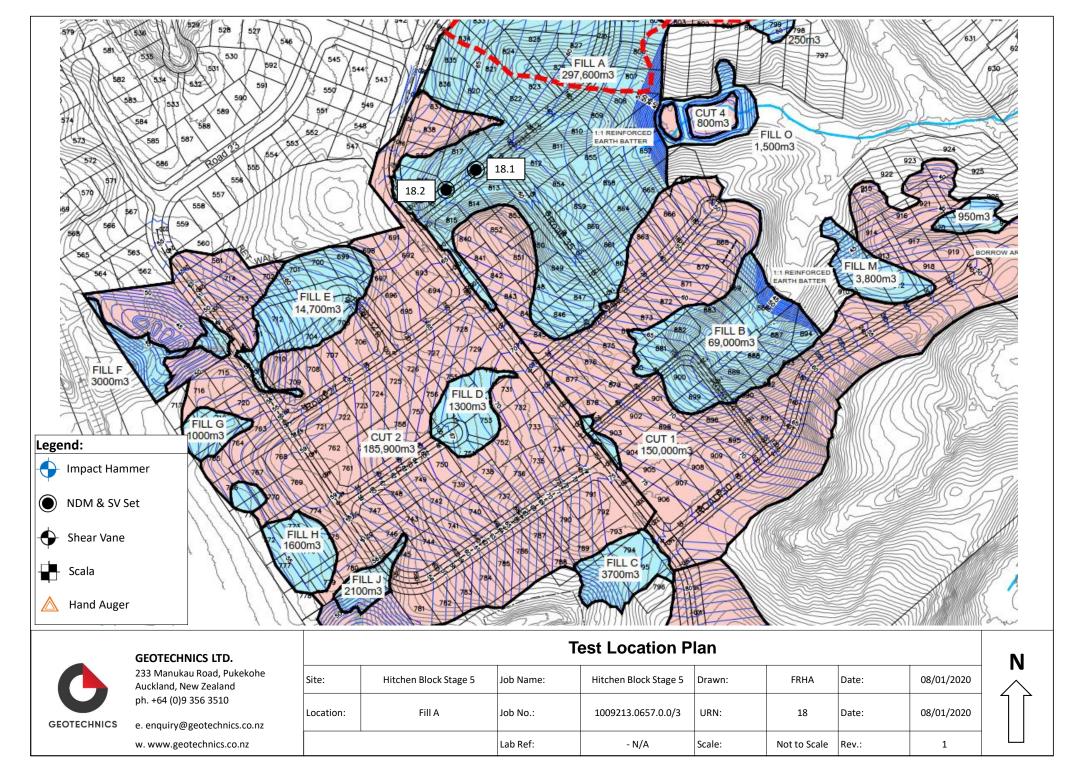


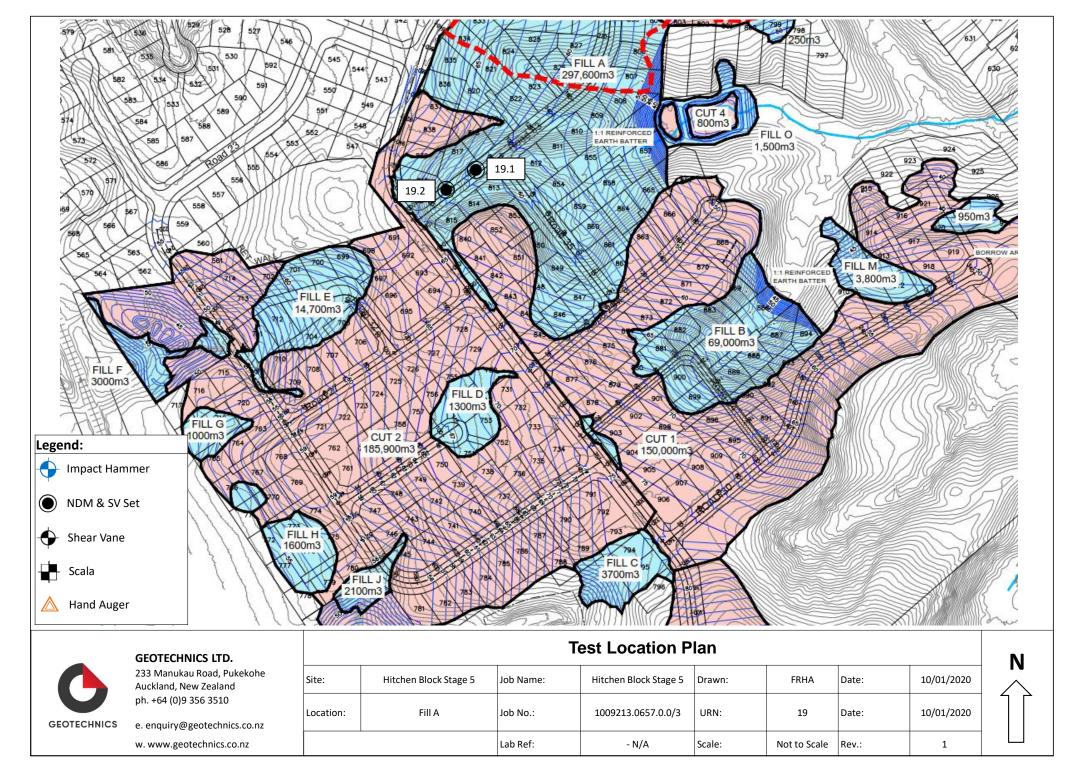


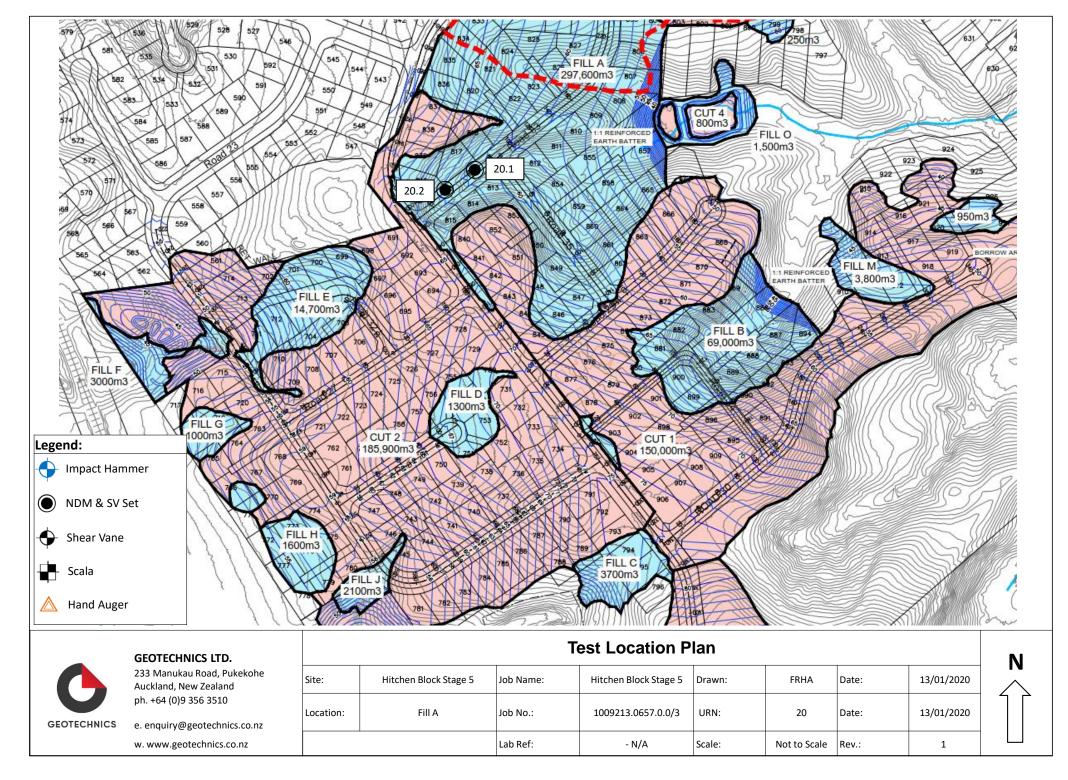


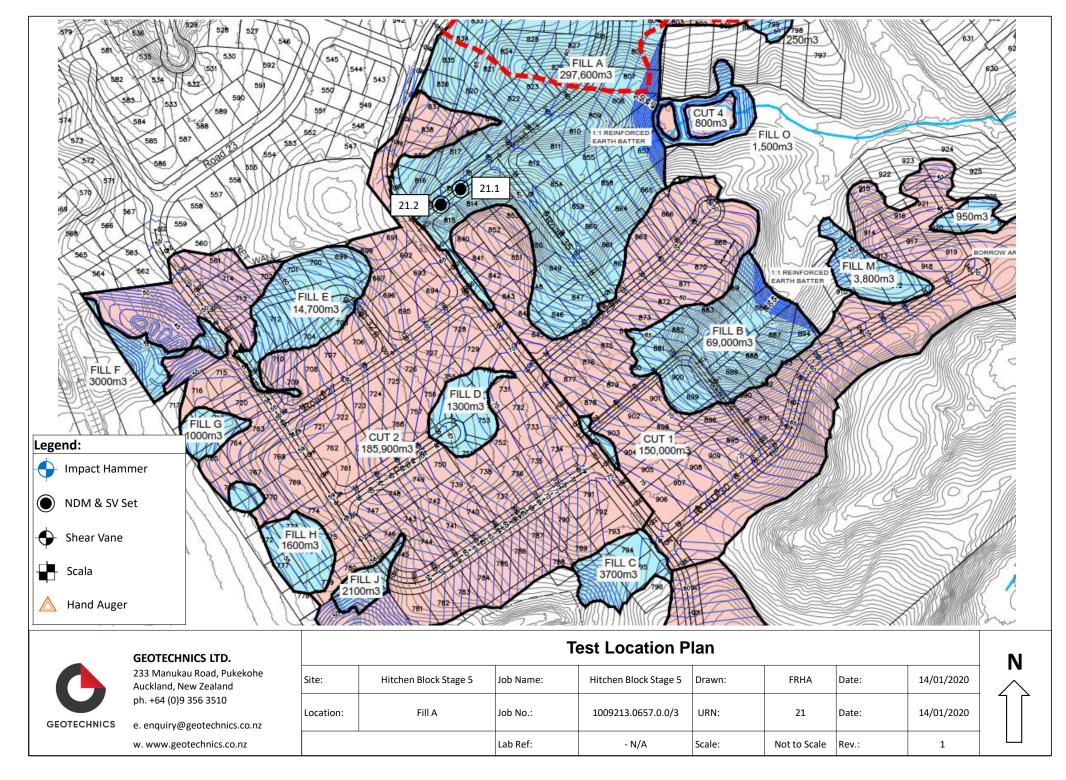


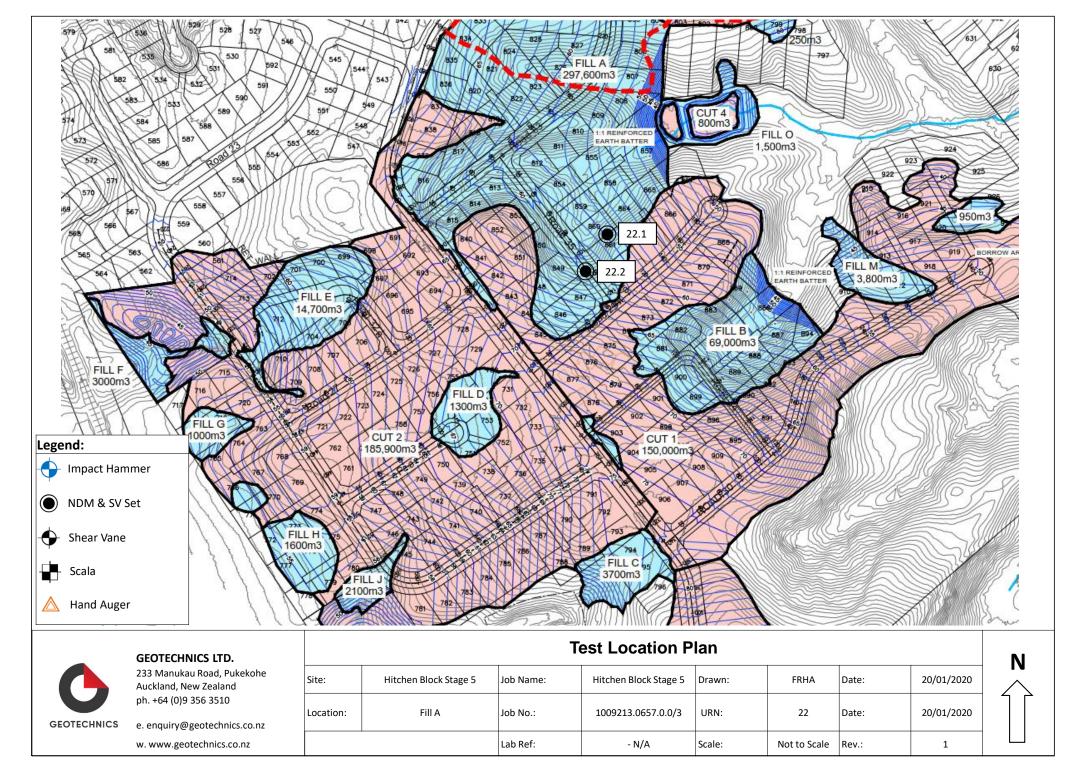


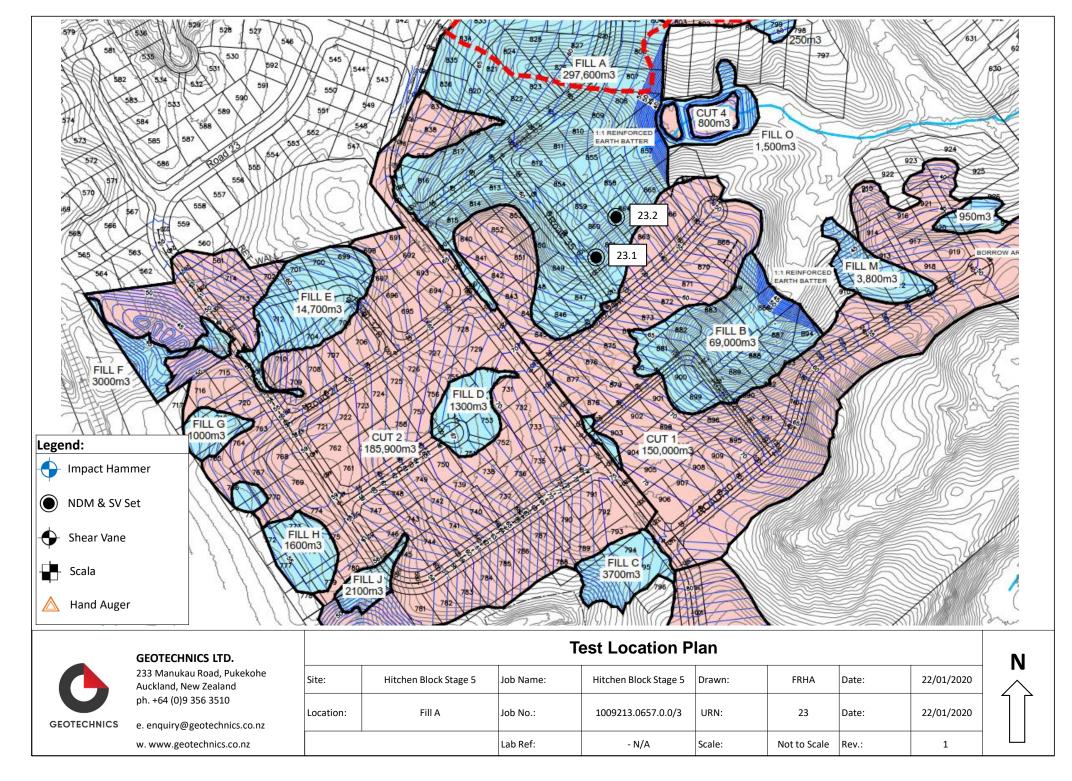


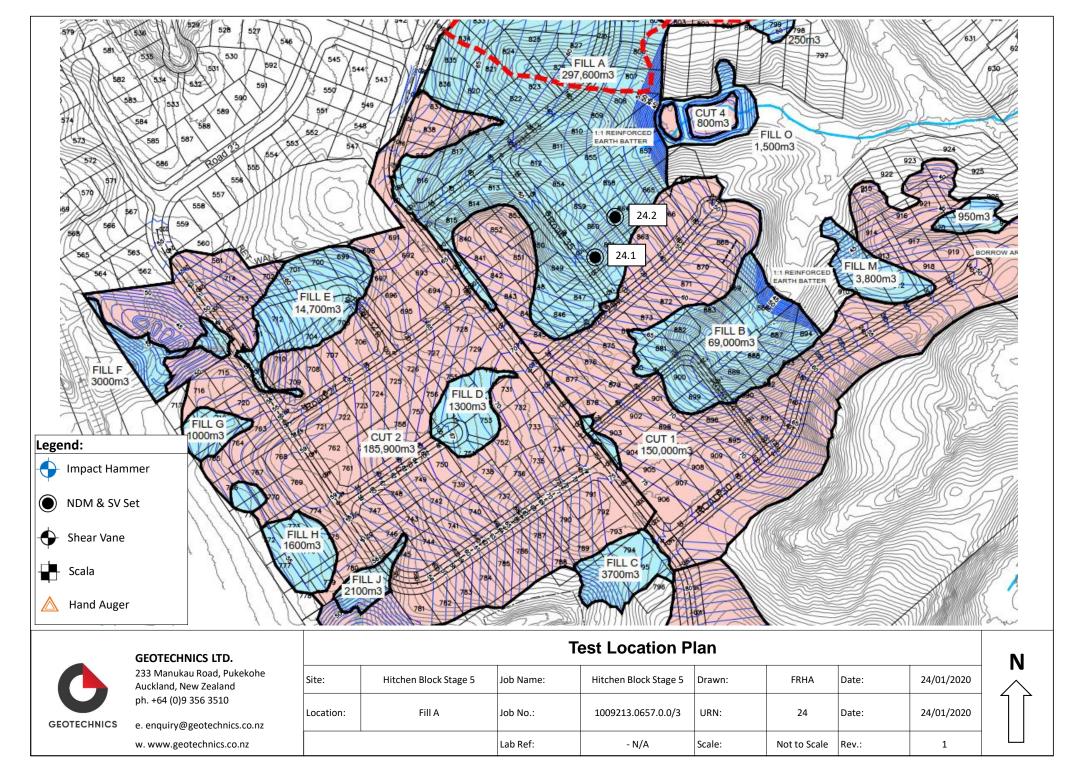


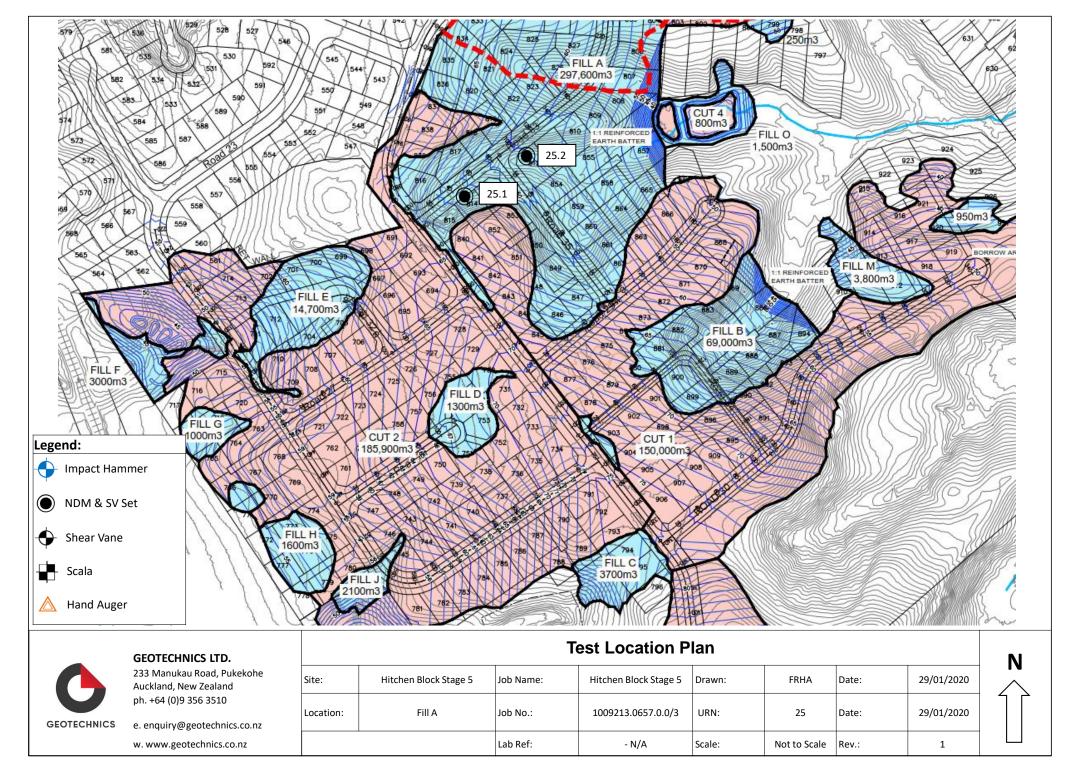


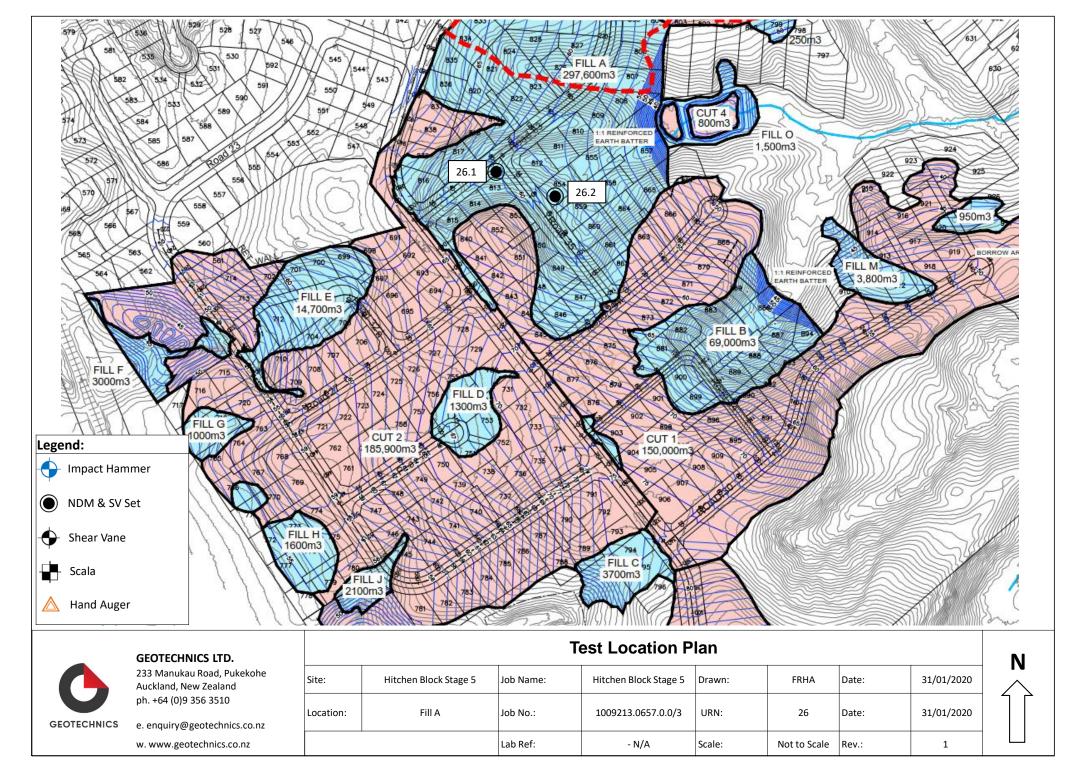


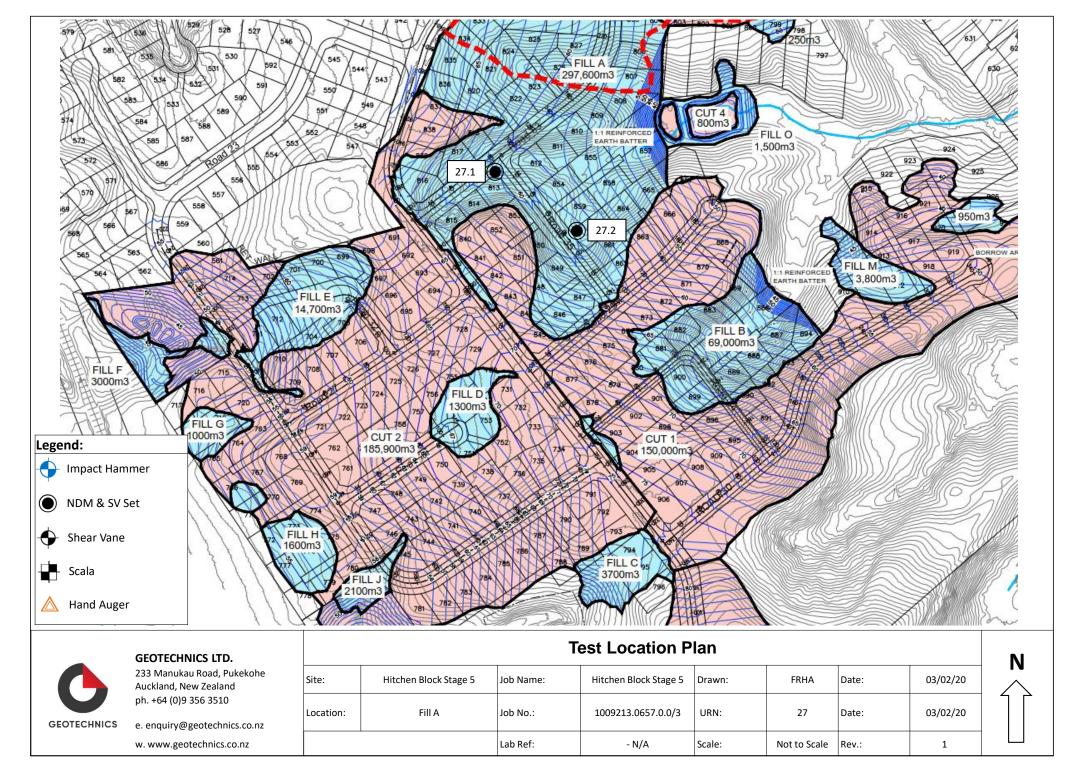


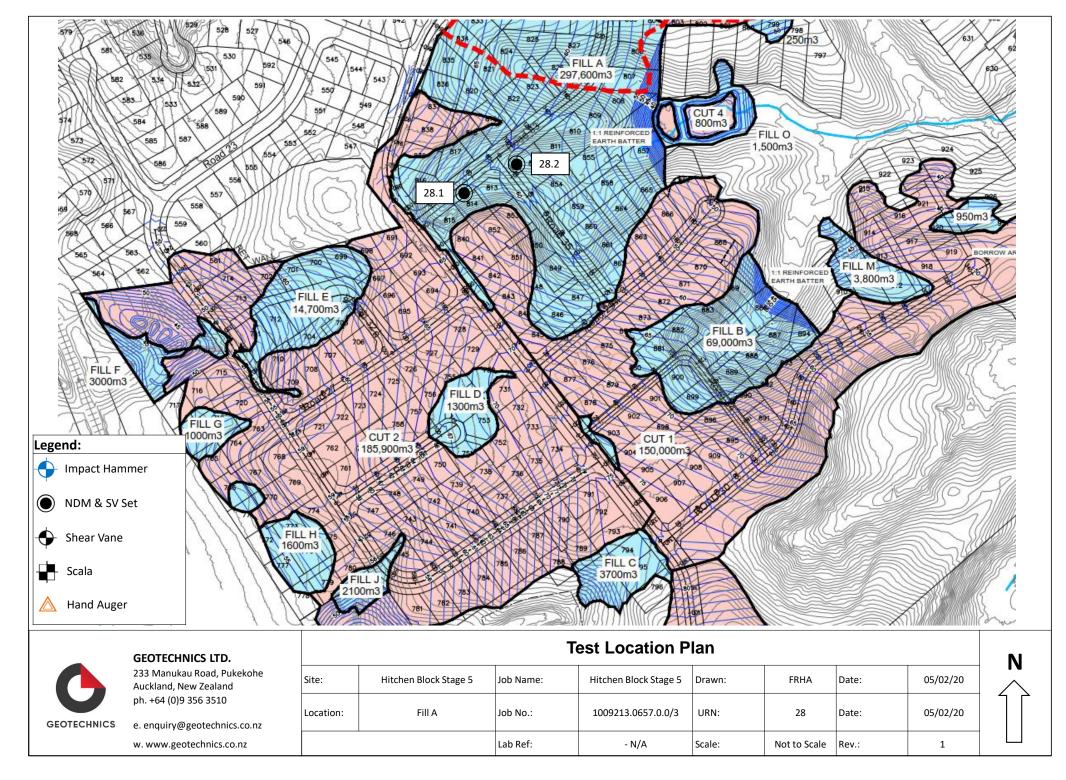


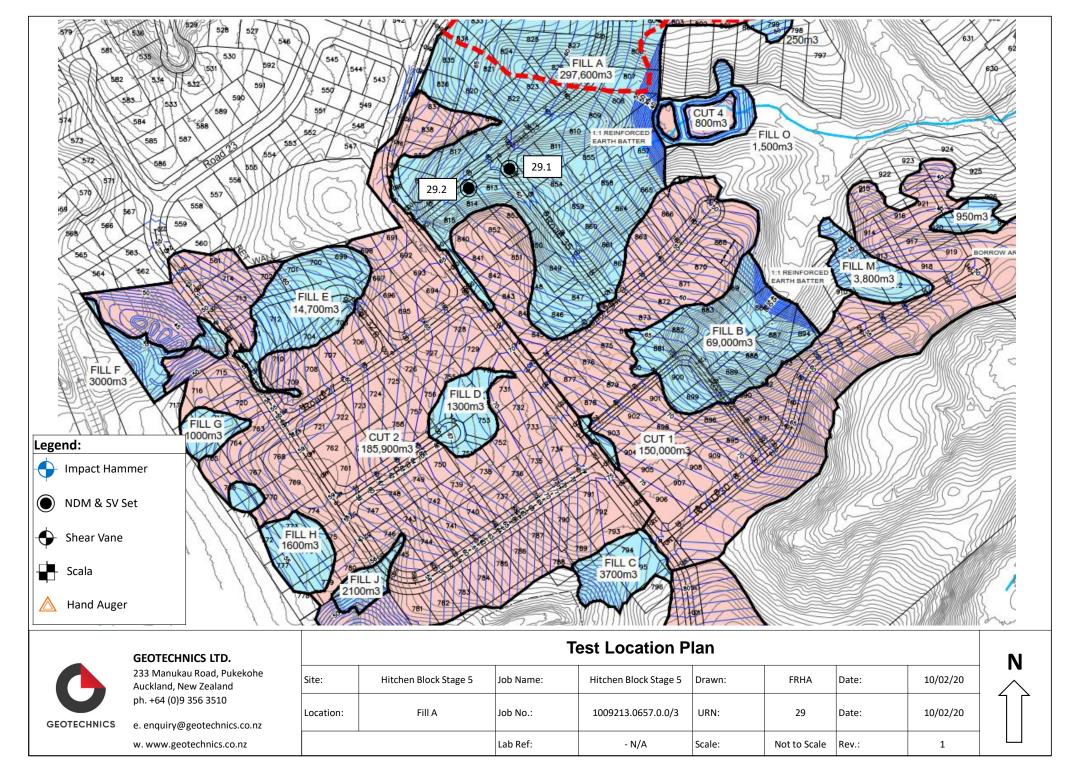


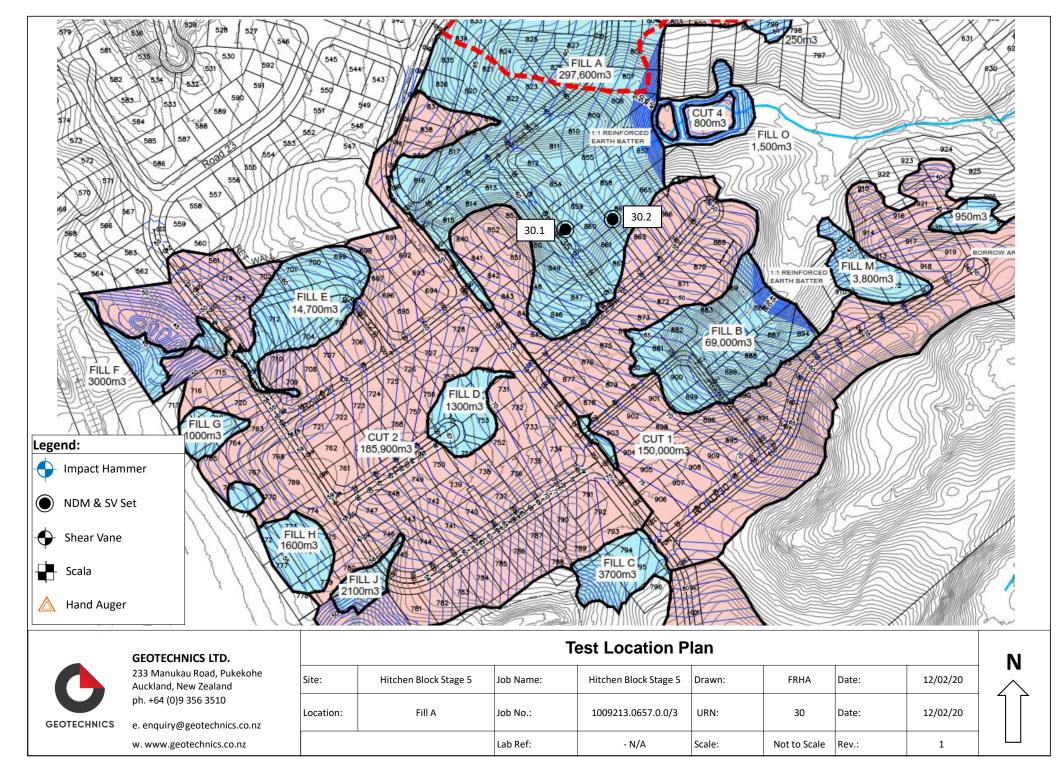


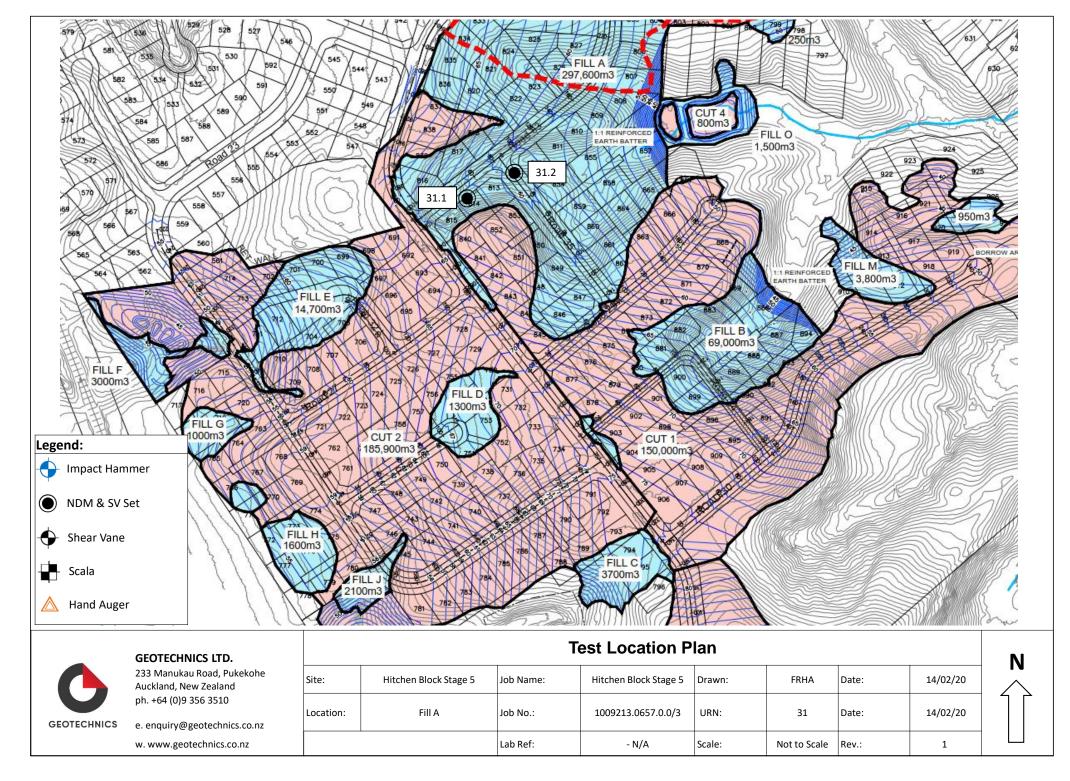


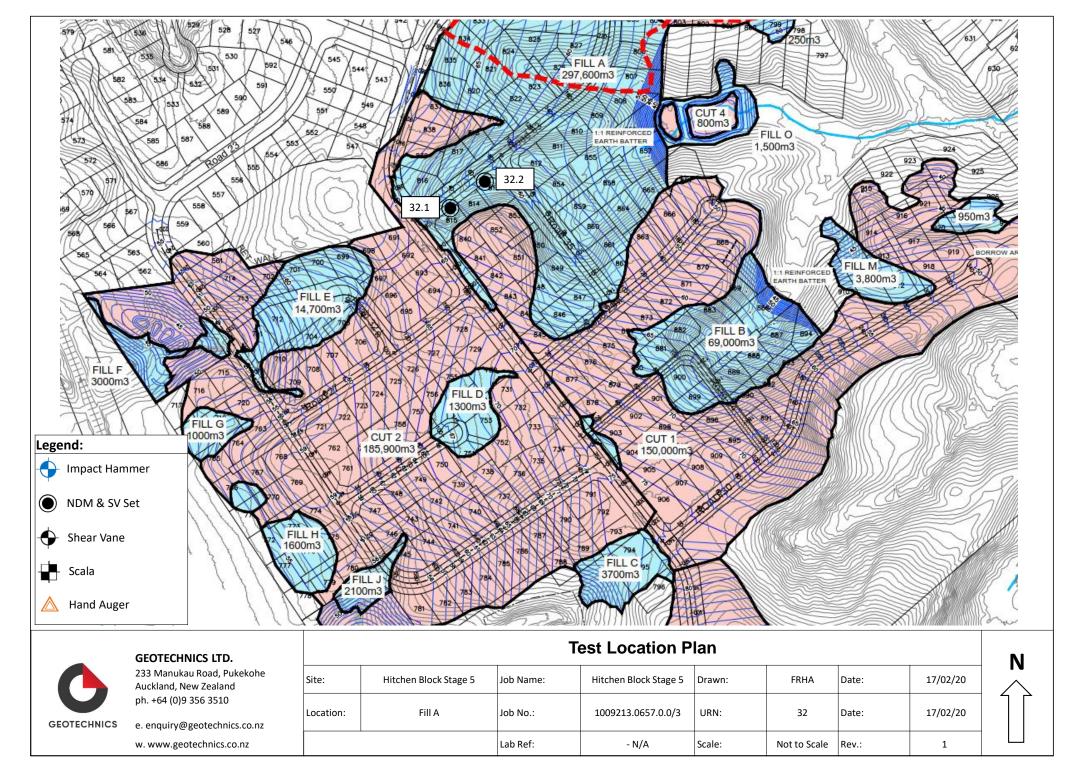


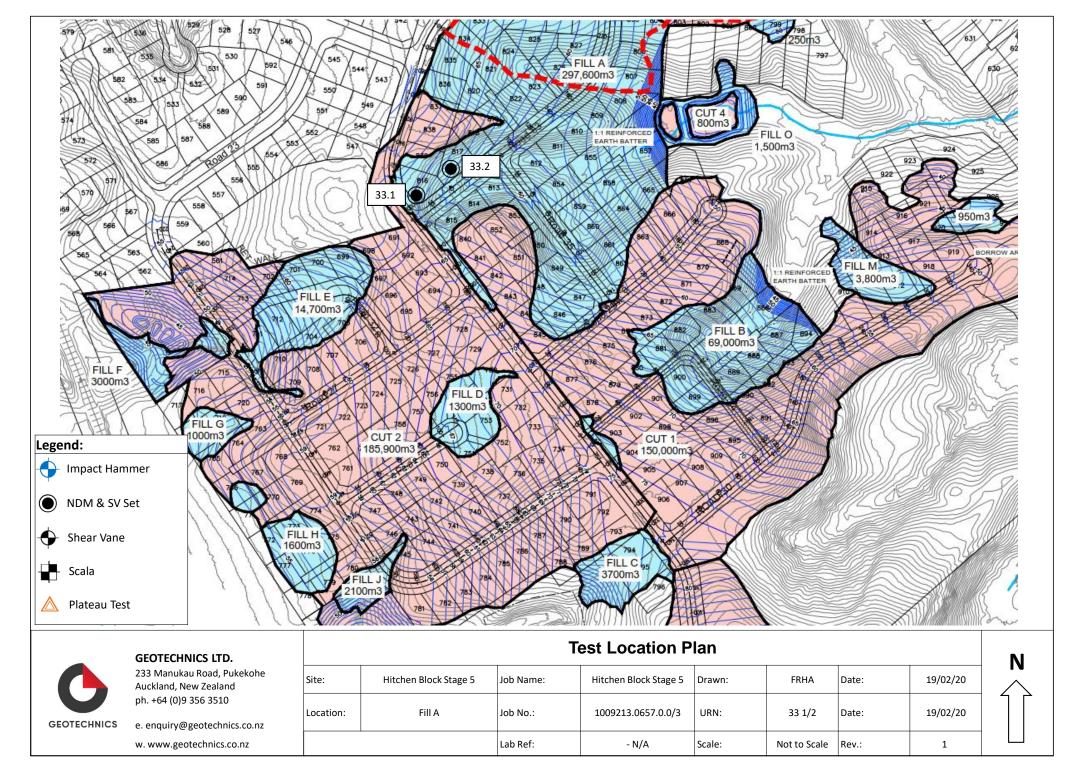


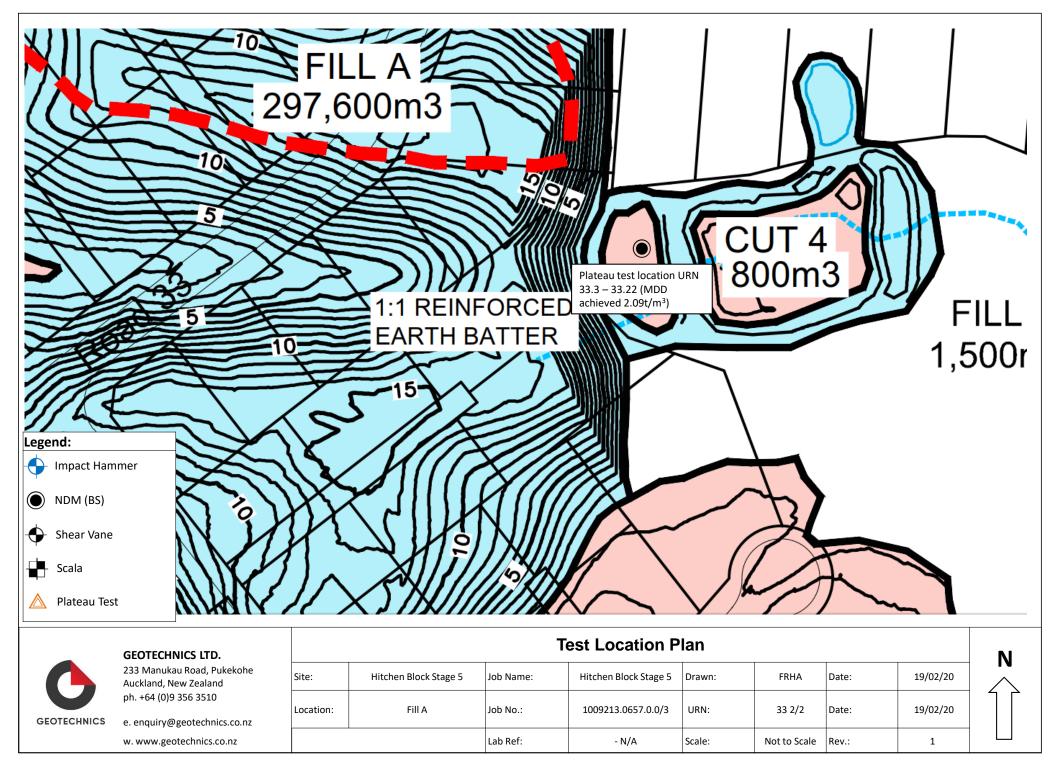


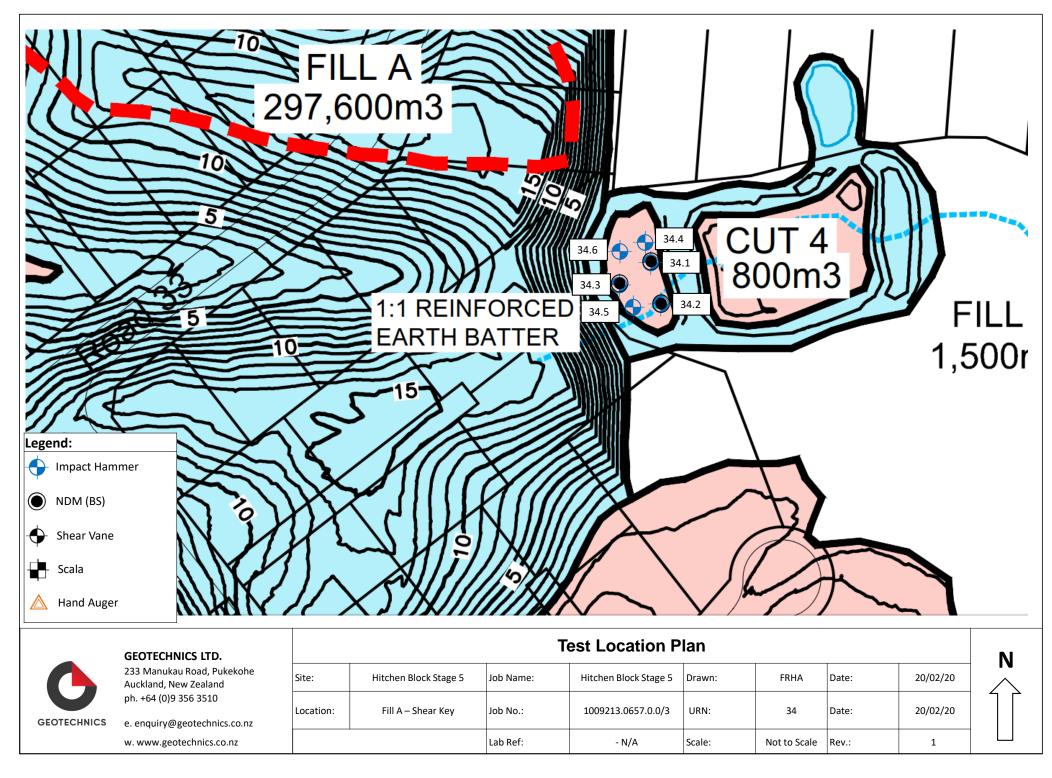


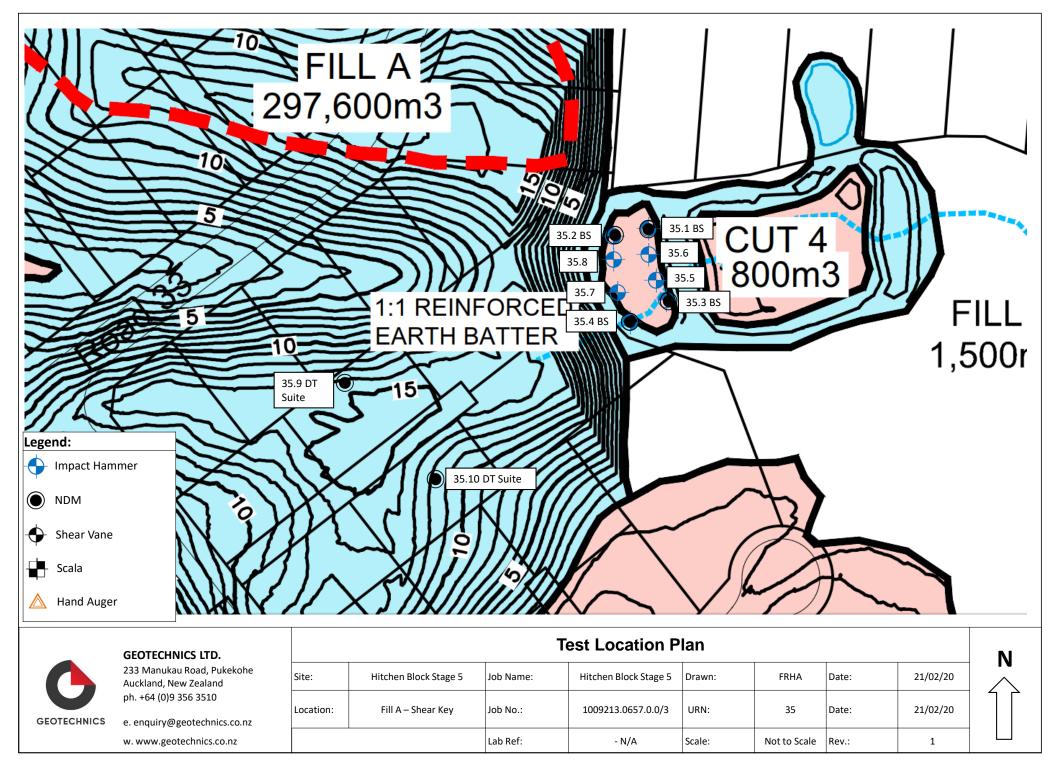


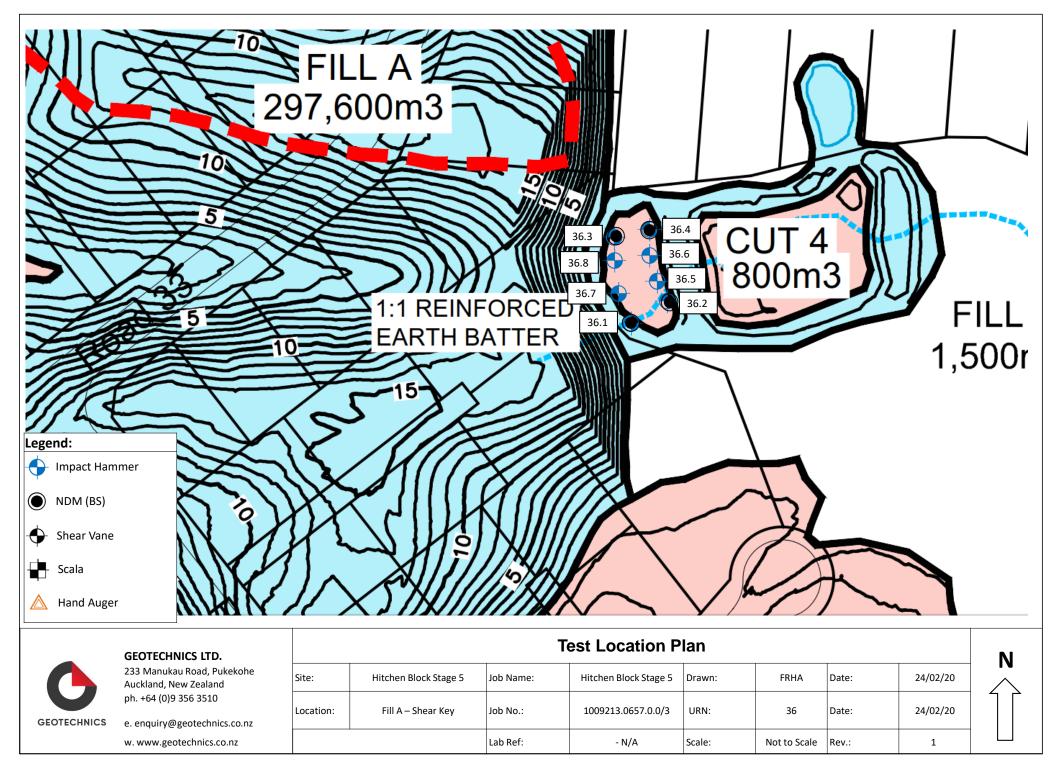


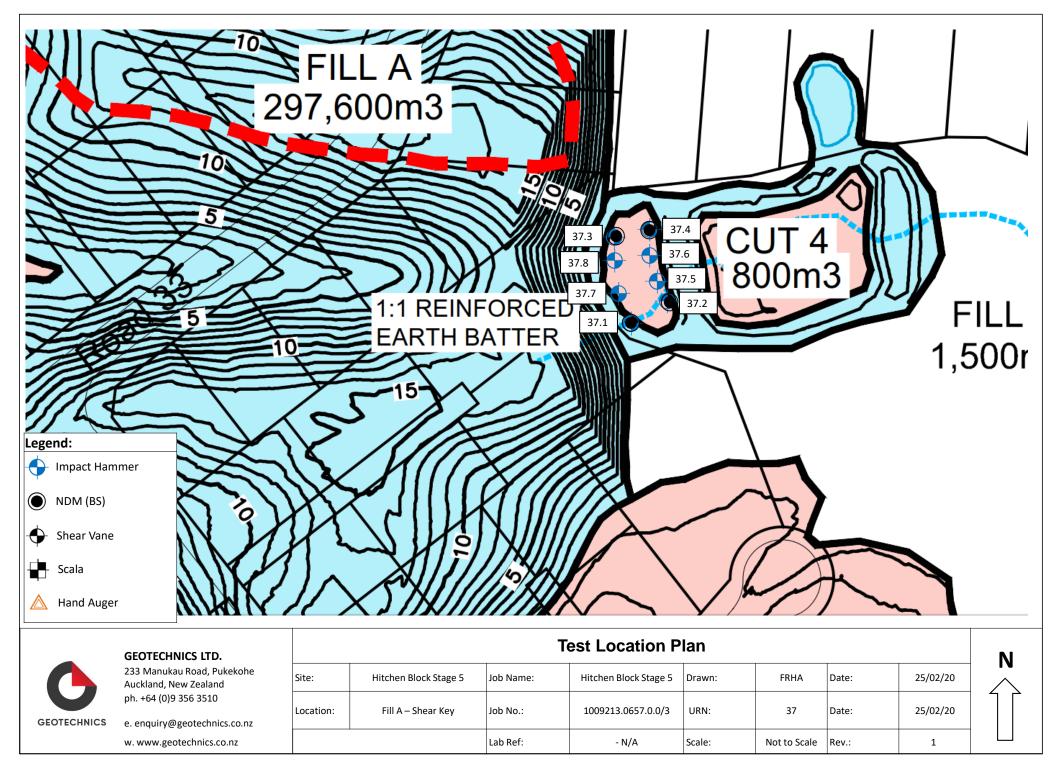


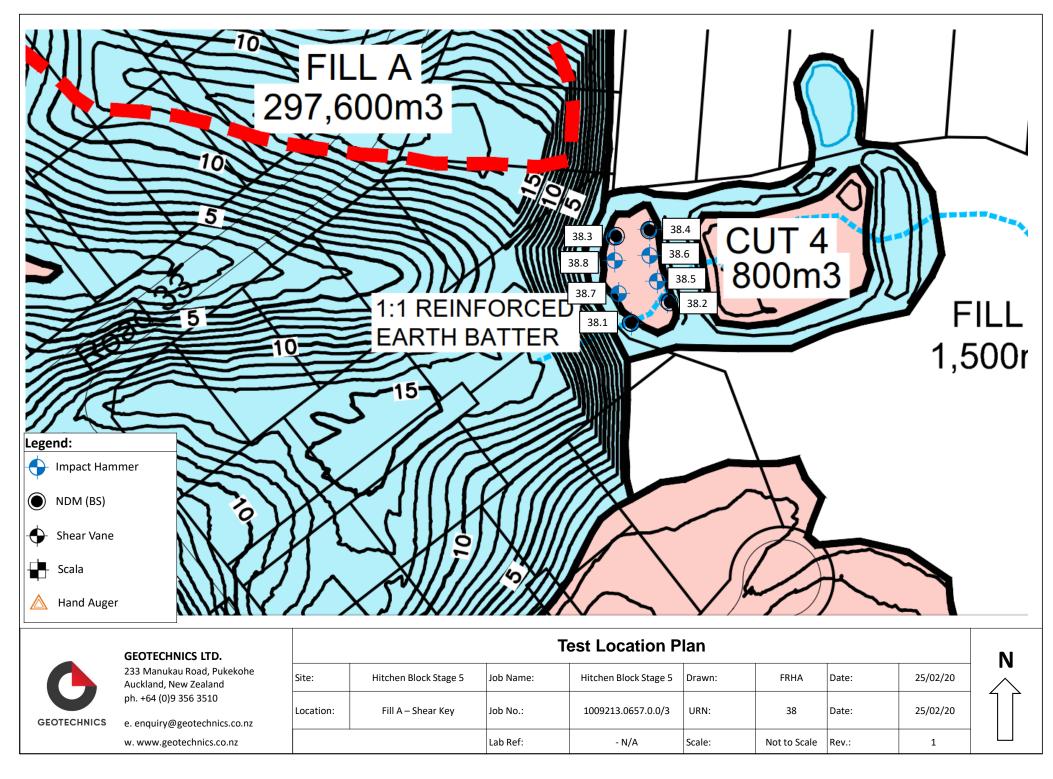


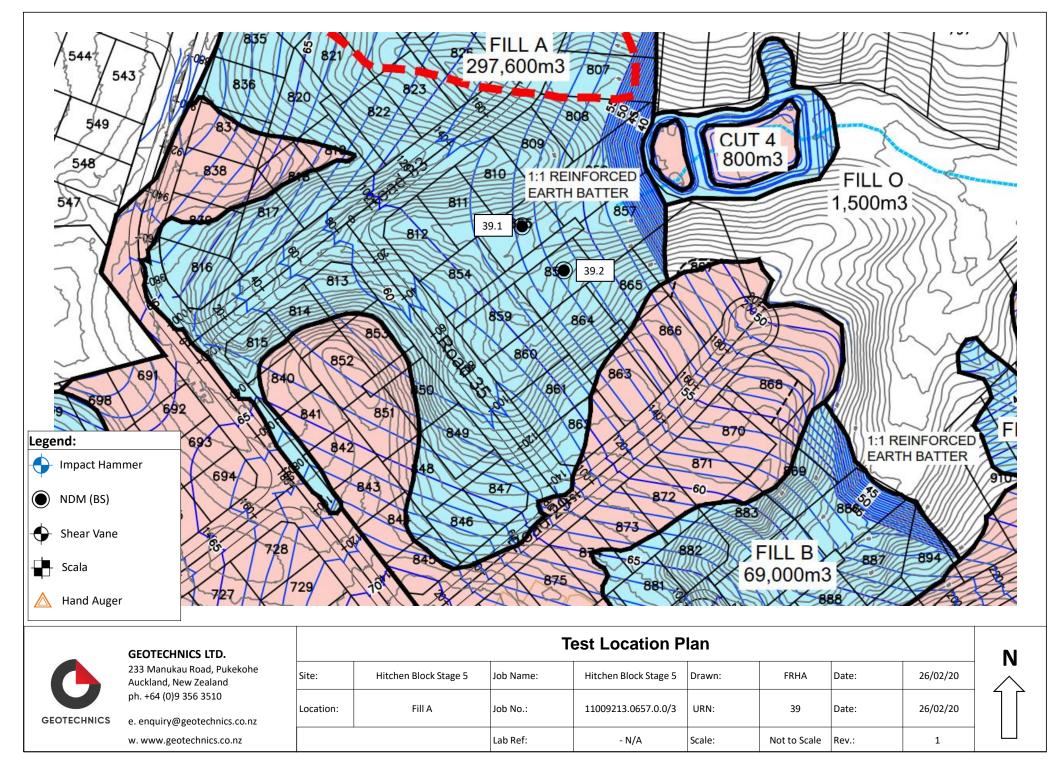


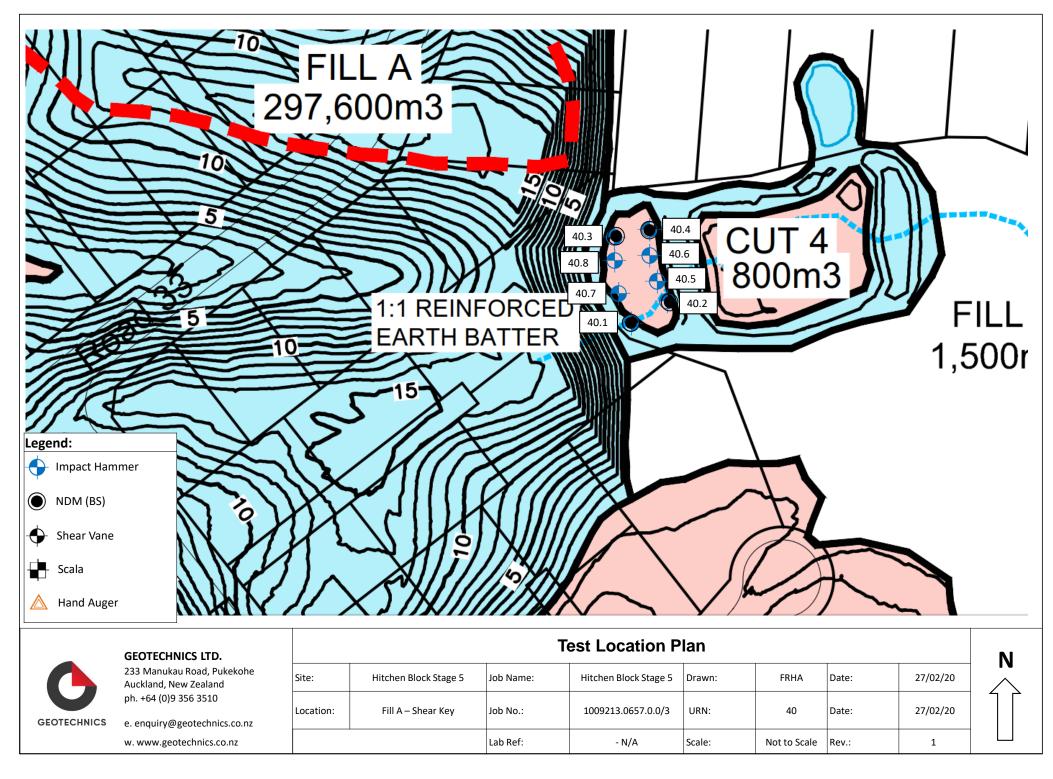


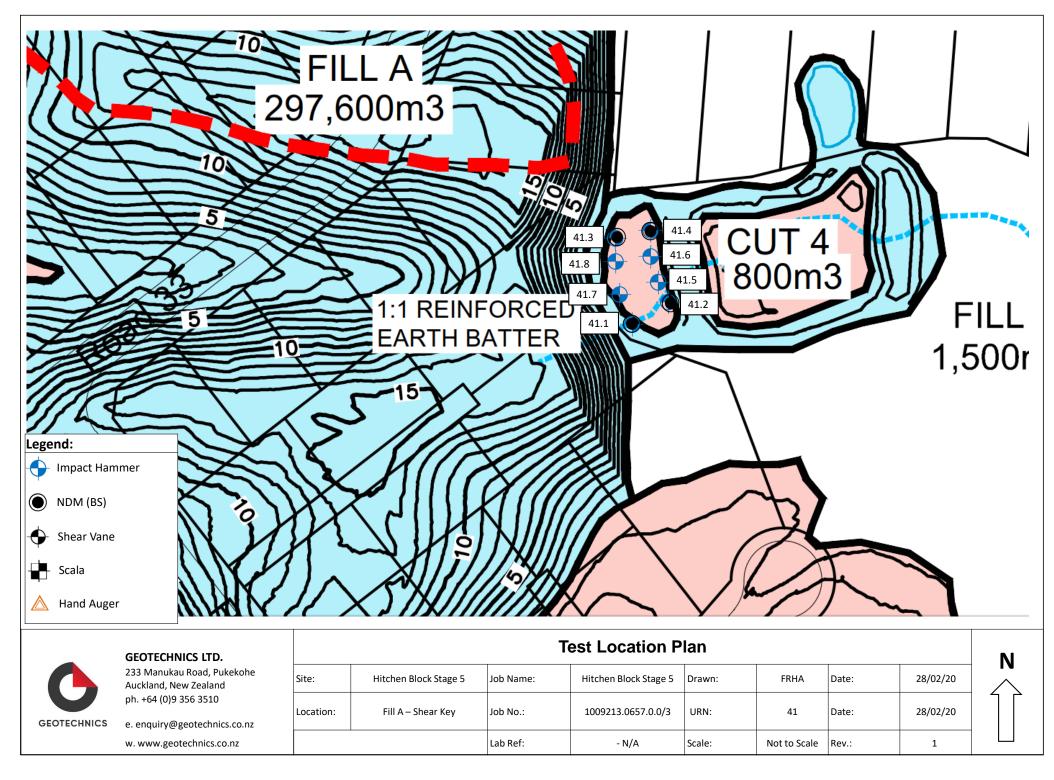


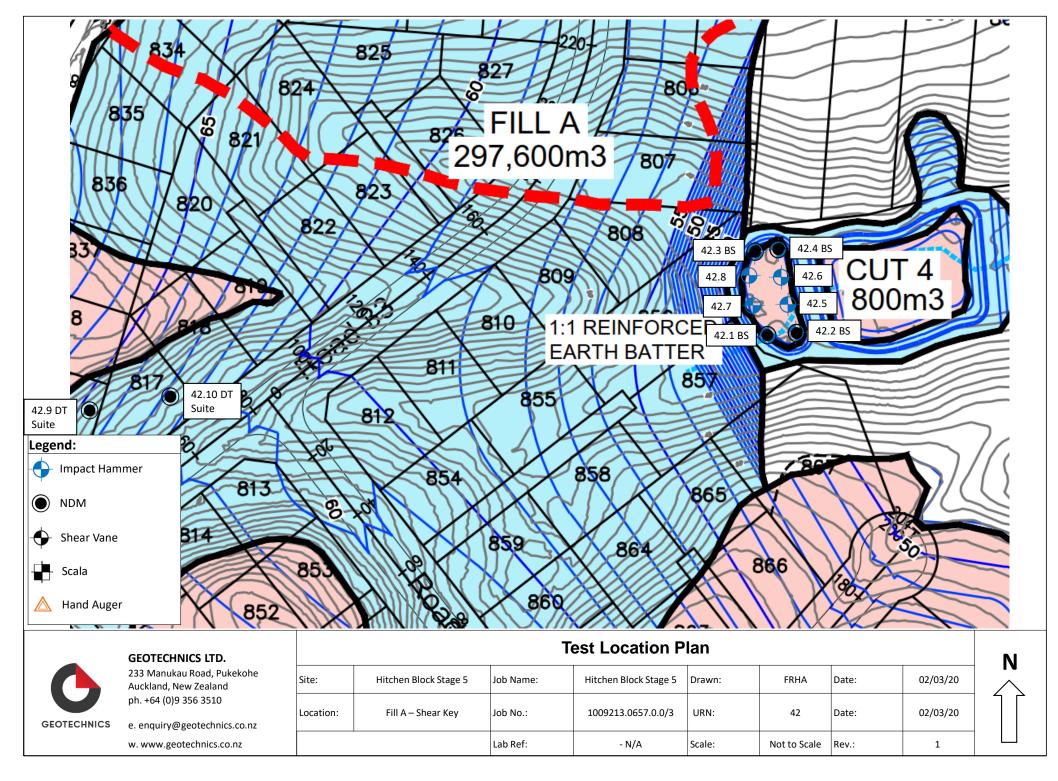


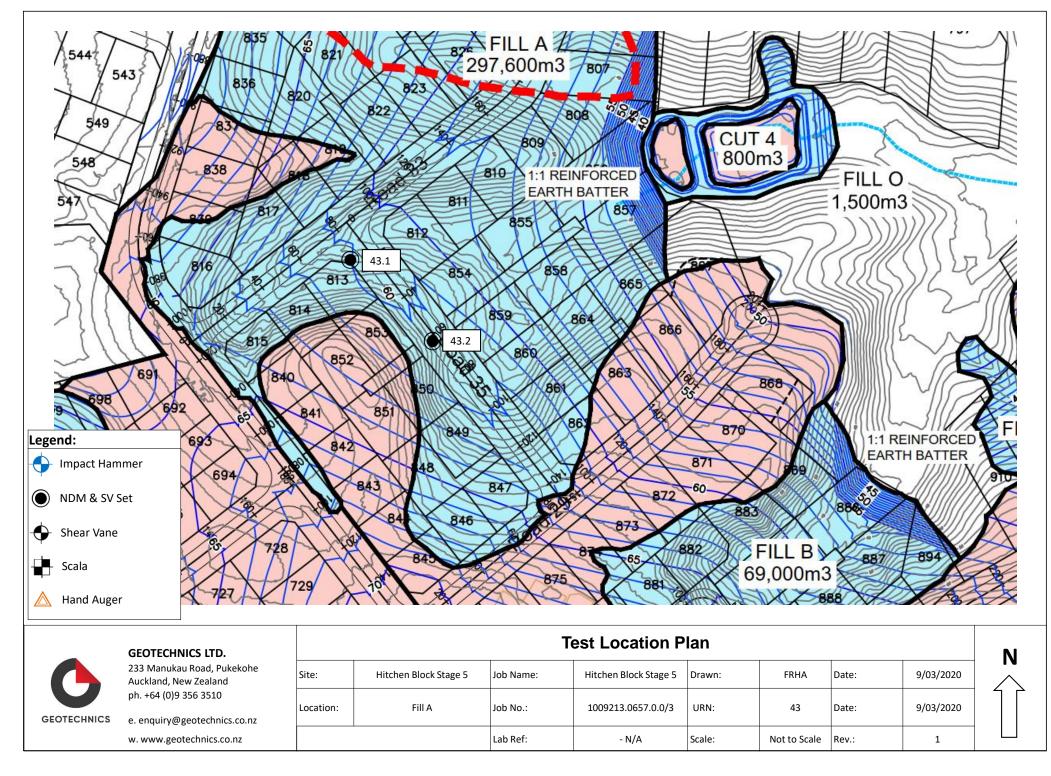


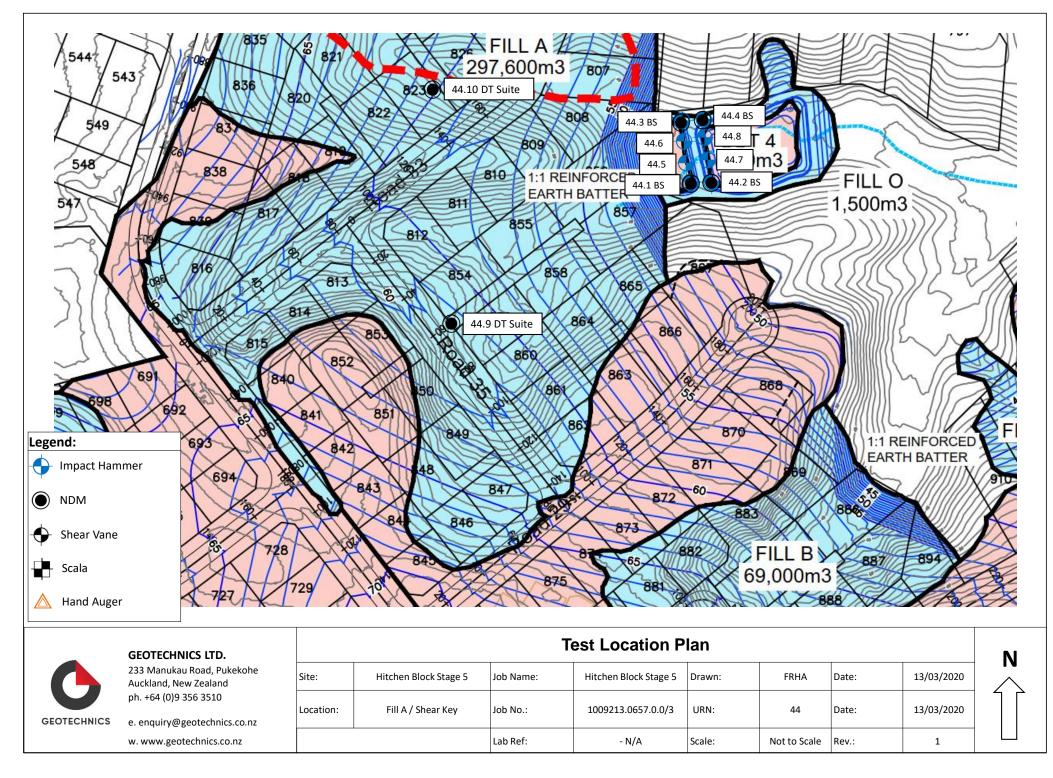


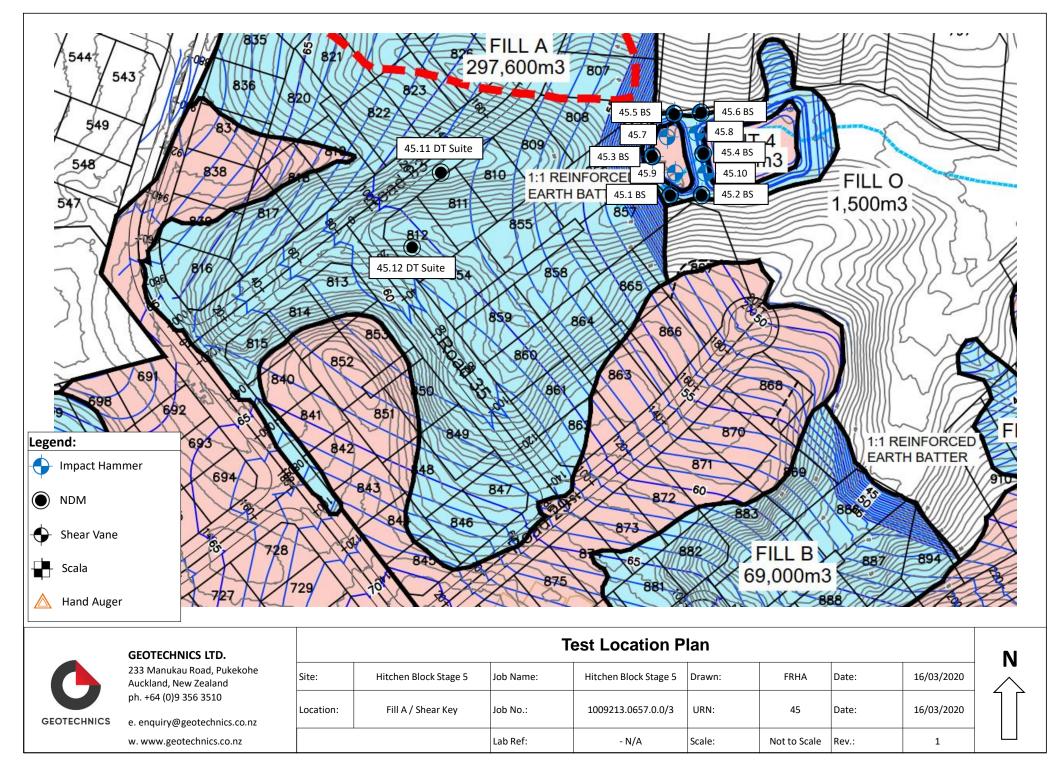


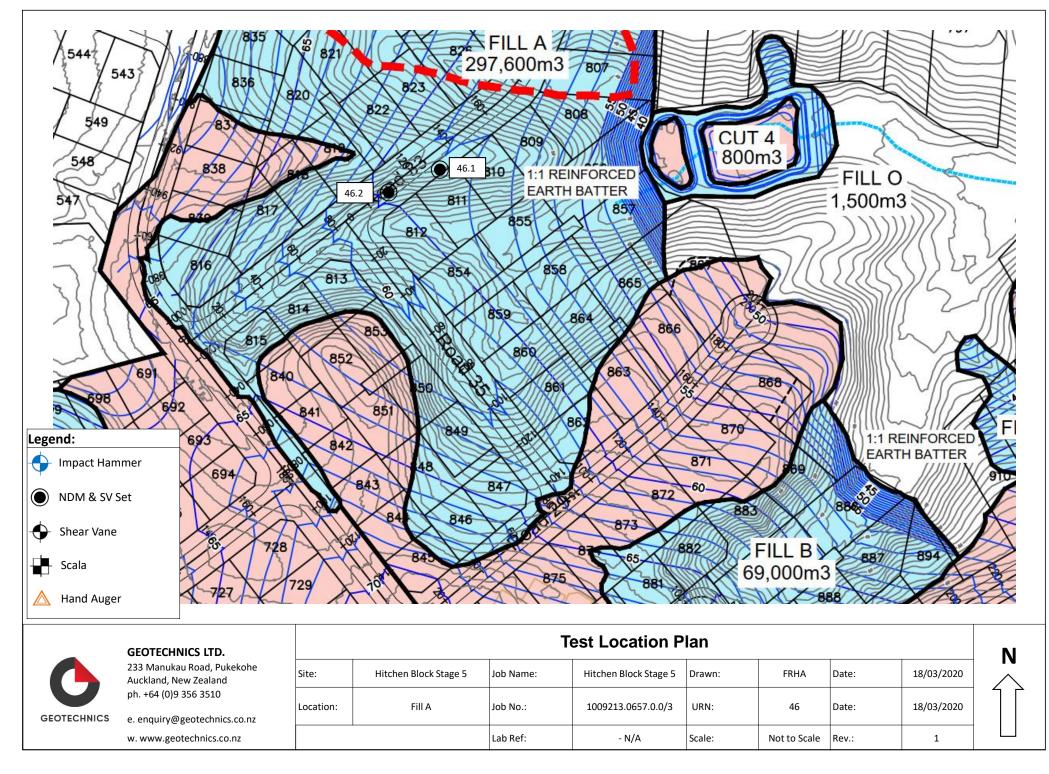


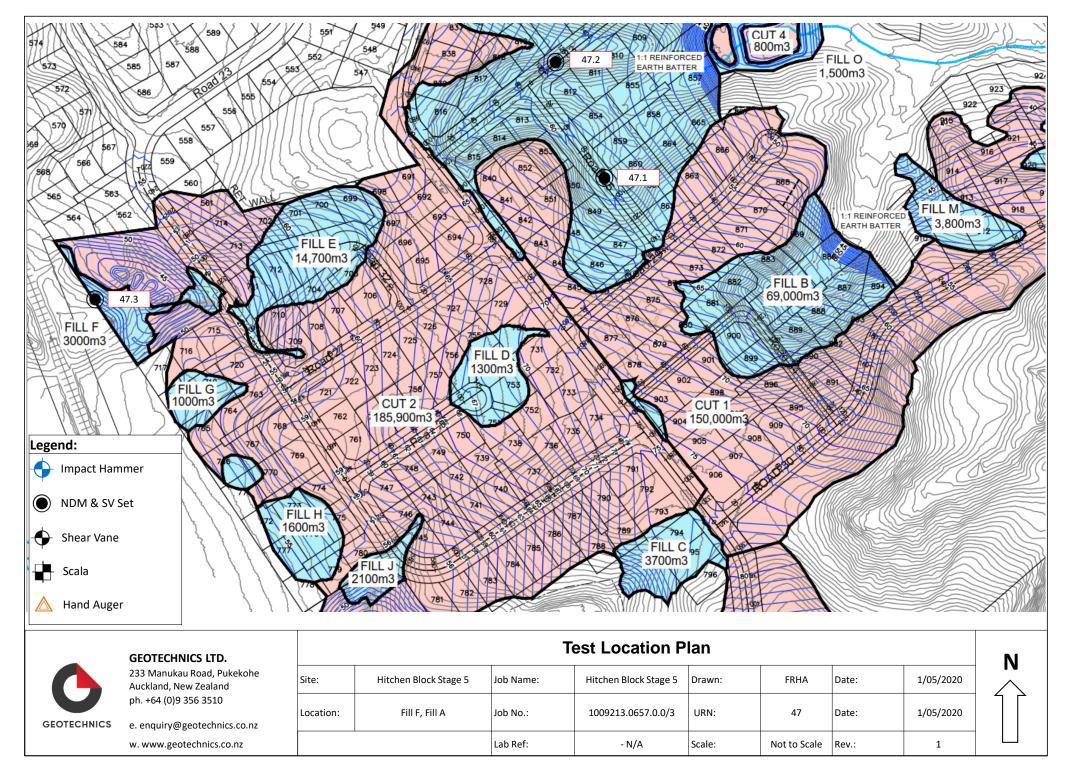


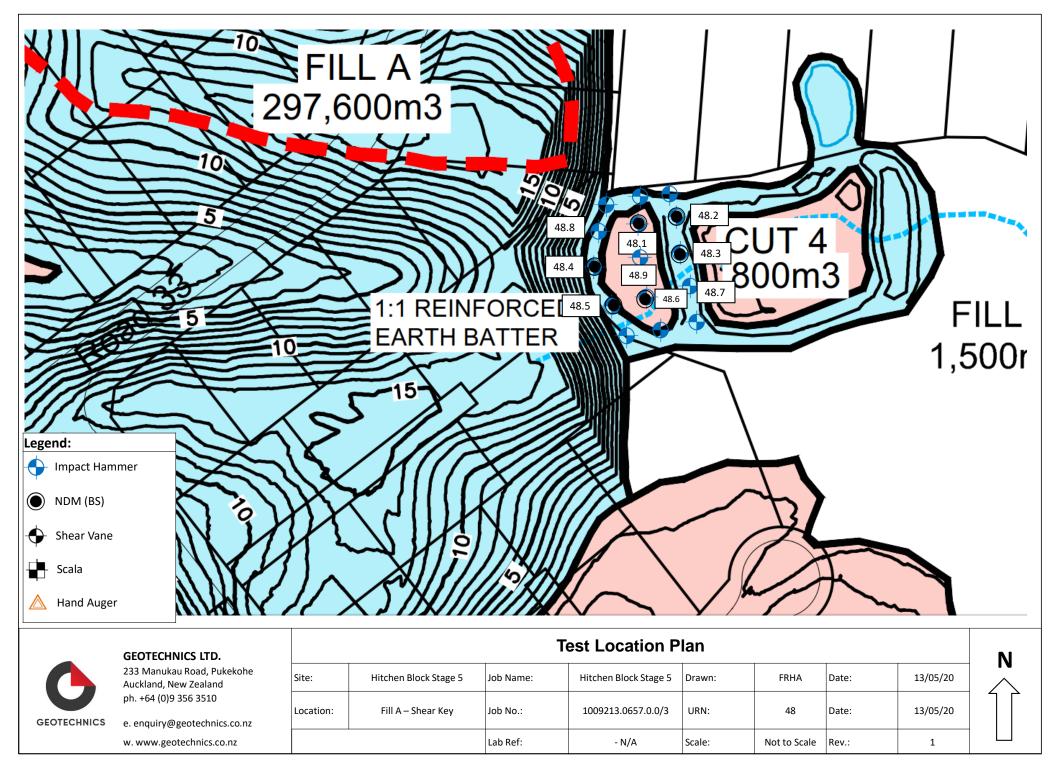


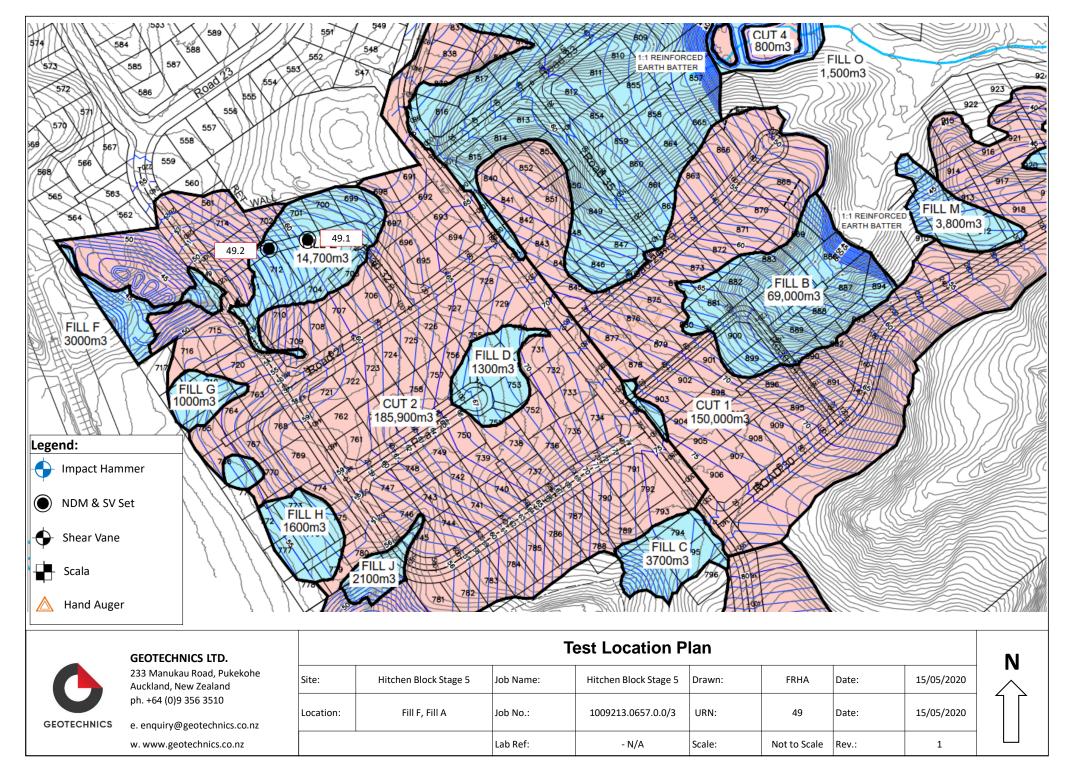






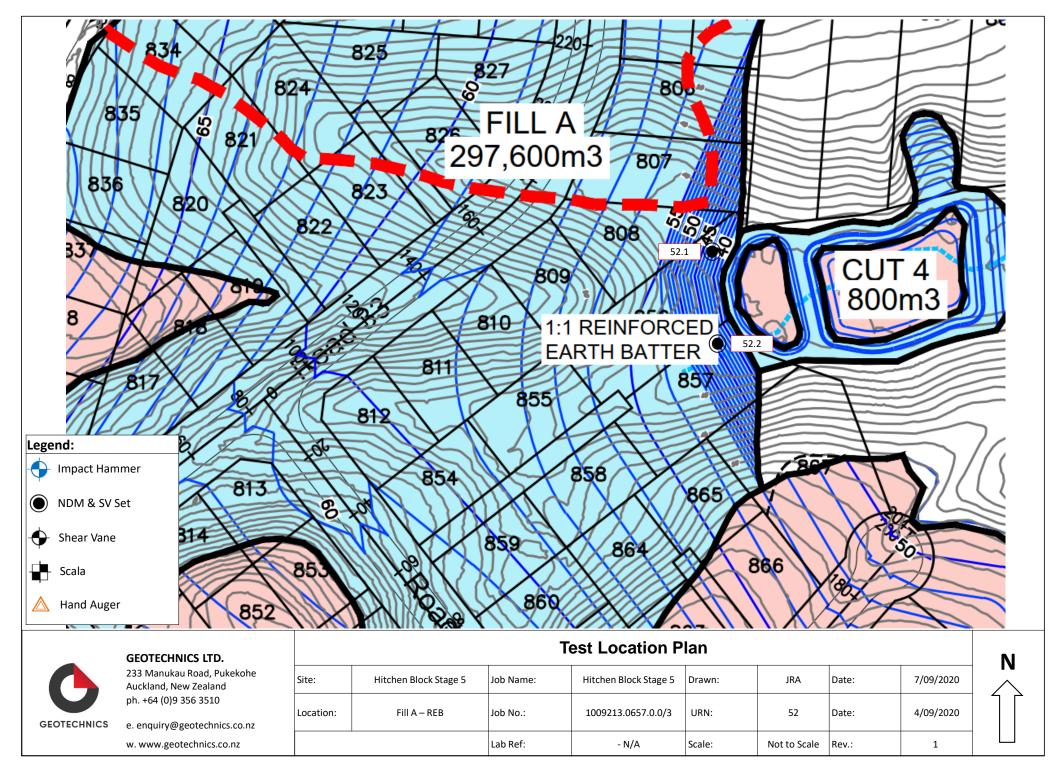


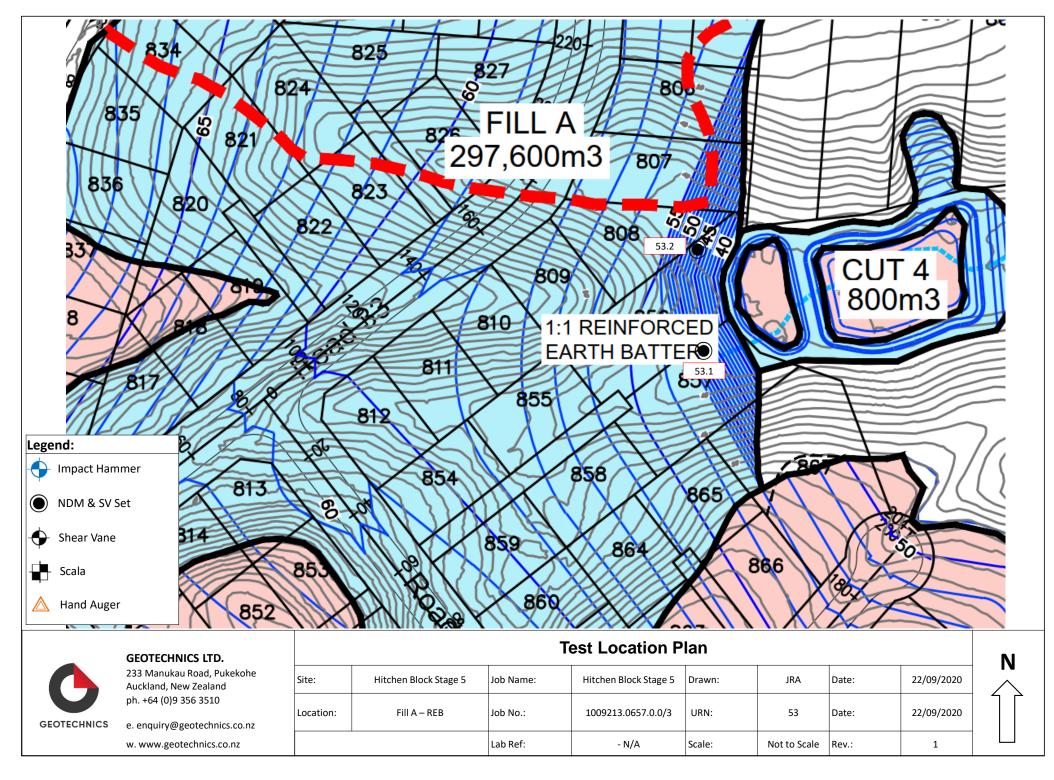


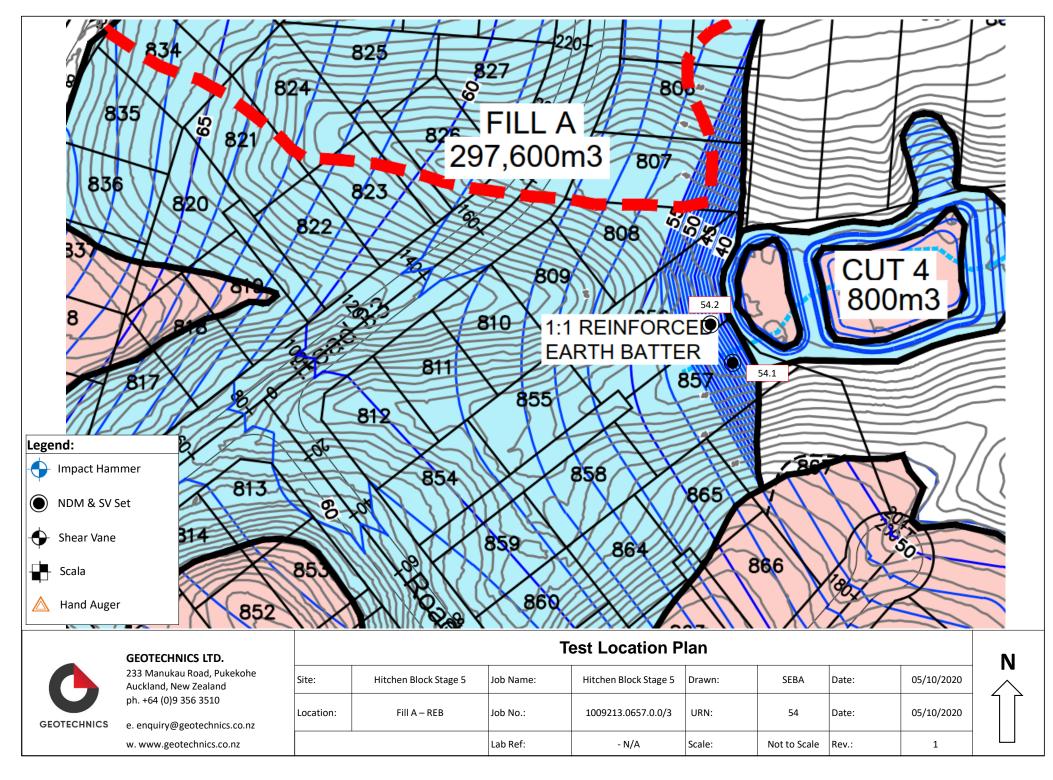


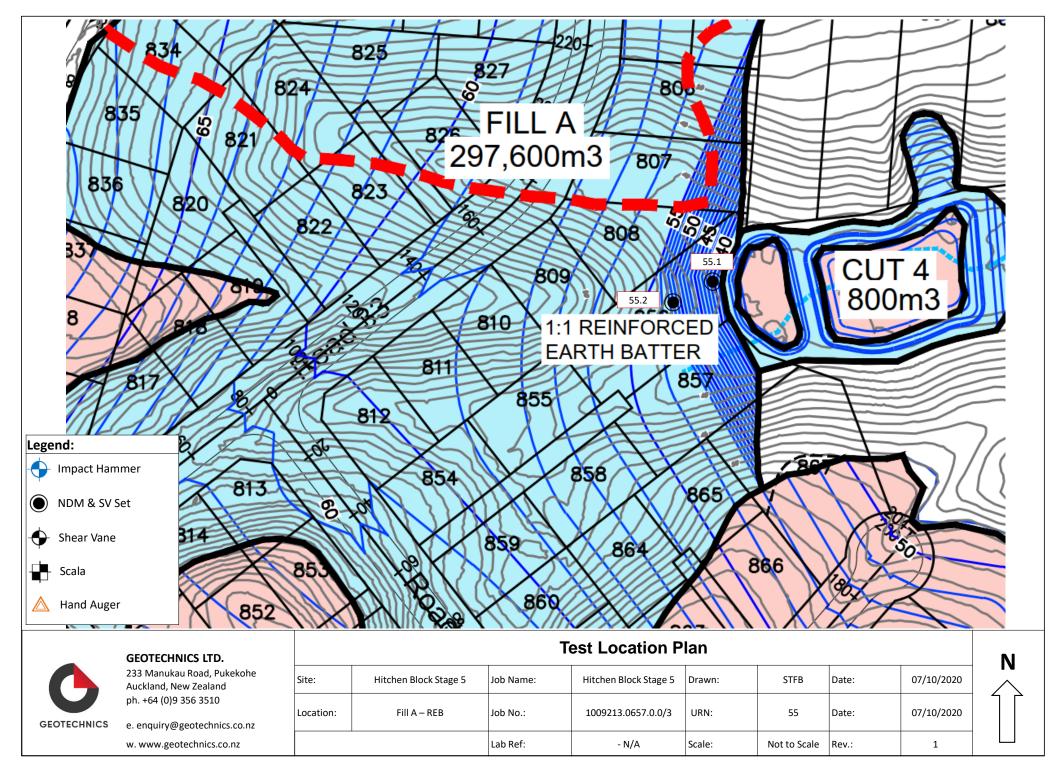
| Impact Hammer Impact Hammer | | | C 8 | | 4 n. | 3 | | | |
|---|--------------------|----------------------|----------------------|-----------------------|----------------|------|-------|------------|--|
| GEOTECHNICS LTD. | Test Location Plan | | | | | | | N | |
| 233 Manukau Road, Pukekohe Auckland, New Zealand | Site: H | itchen Block Stage 5 | Job Name: | Hitchen Block Stage 5 | Drawn: | FRHA | Date: | 18/05/2020 | |
| ph. +64 (0)9 356 3510 GEOTECHNICS e. enquiry@geotechnics.co.nz | Location: | Fill A – Shear Key | Job No.: Lab Ref: | 1009213.0657.0.0/3 | URN: Scale: | | Date: | 18/05/2020 | |

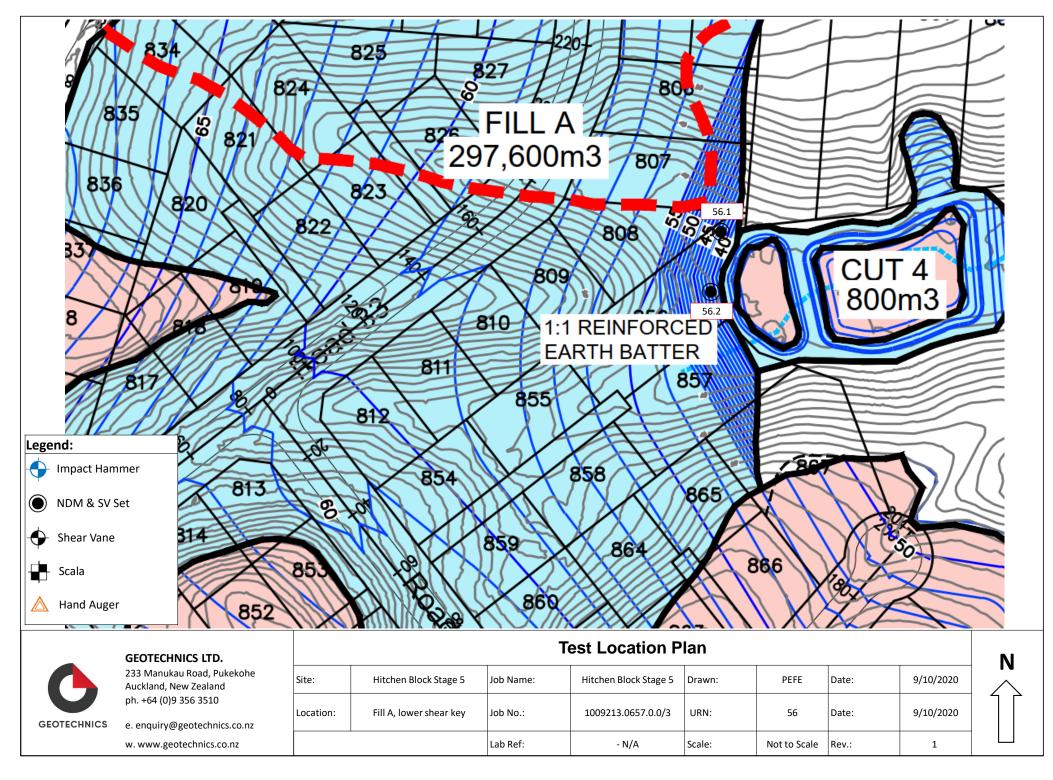
| Legend: | Set | 51.1 | | | | 4 m(| 3 | | | |
|-------------|---|-----------|-----------------------|-----------|-----------------------|---------|--------------|-------|------------|--------|
| Hand Auger | | | | | Test Location F | Plan | | | | |
| | GEOTECHNICS LTD. 233 Manukau Road, Pukekohe Auckland, New Zealand | Site: | Hitchen Block Stage 5 | Job Name: | Hitchen Block Stage 5 | Drawn: | FRHA | Date: | 17/06/2020 | N ^ |
| GEOTECHNICS | e. enquiry@geotechnics.co.nz | Location: | Fill A – Shear Key | Job No.: | 1009213.0657.0.0/3 | URN: | 51 | Date: | 17/06/2020 | |
| | w. www.geotechnics.co.nz | | | Lab Ref: | - N/A | Scale: | Not to Scale | Rev.: | 1 | |

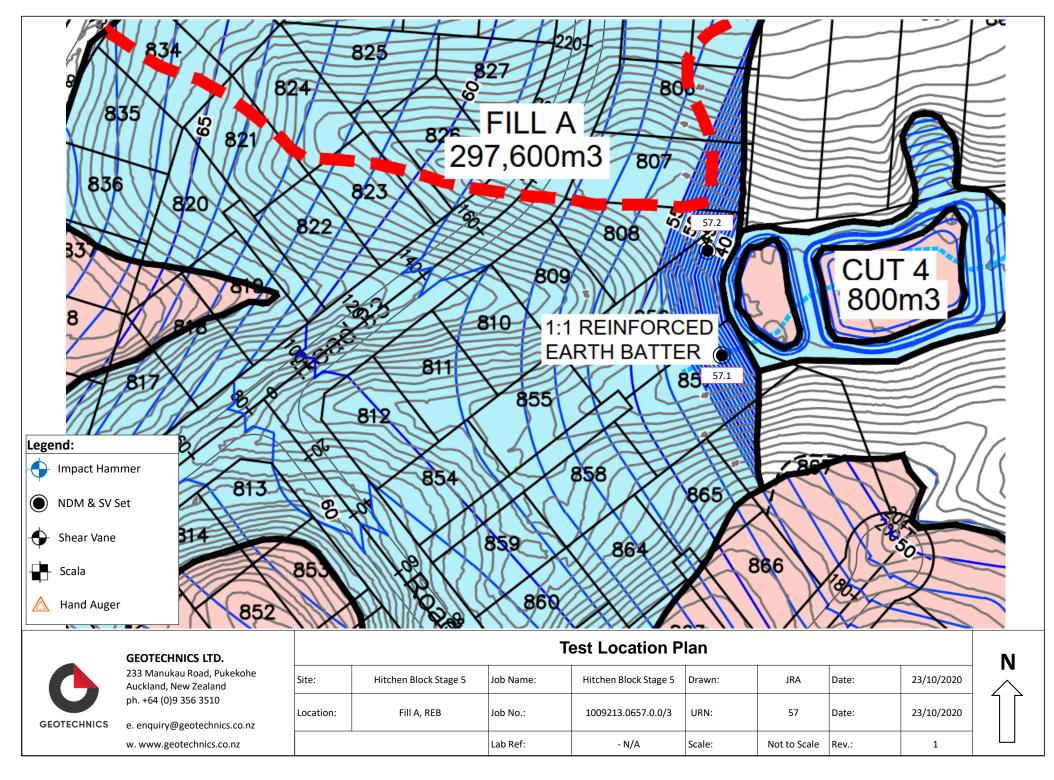


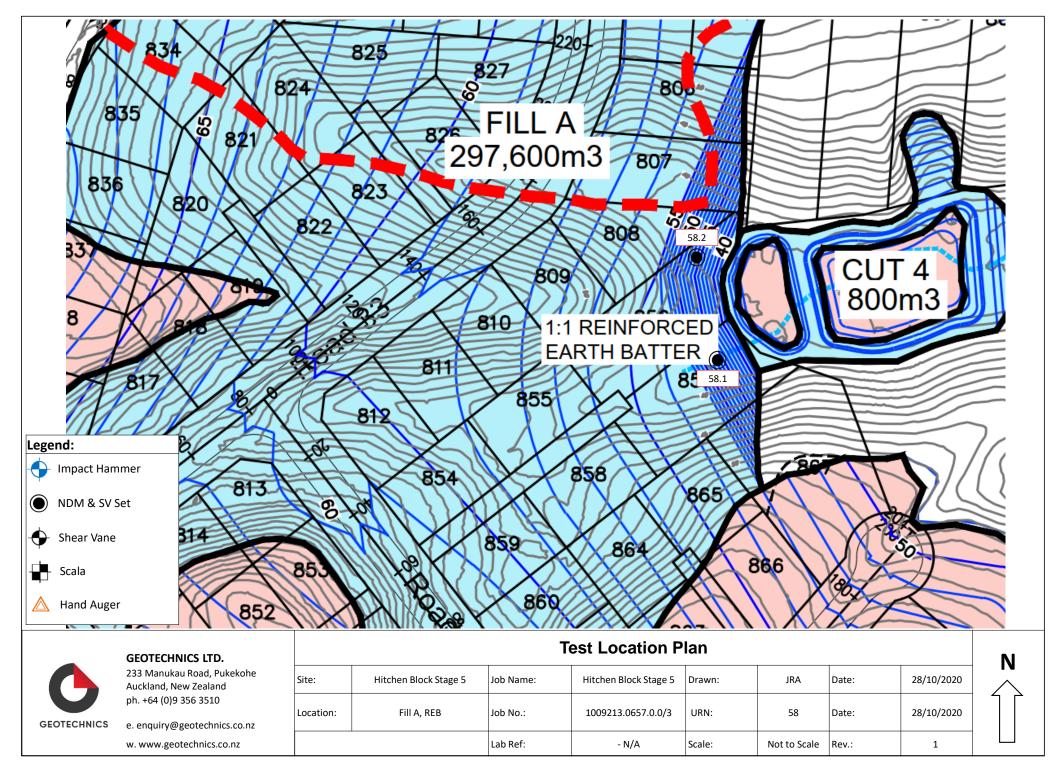


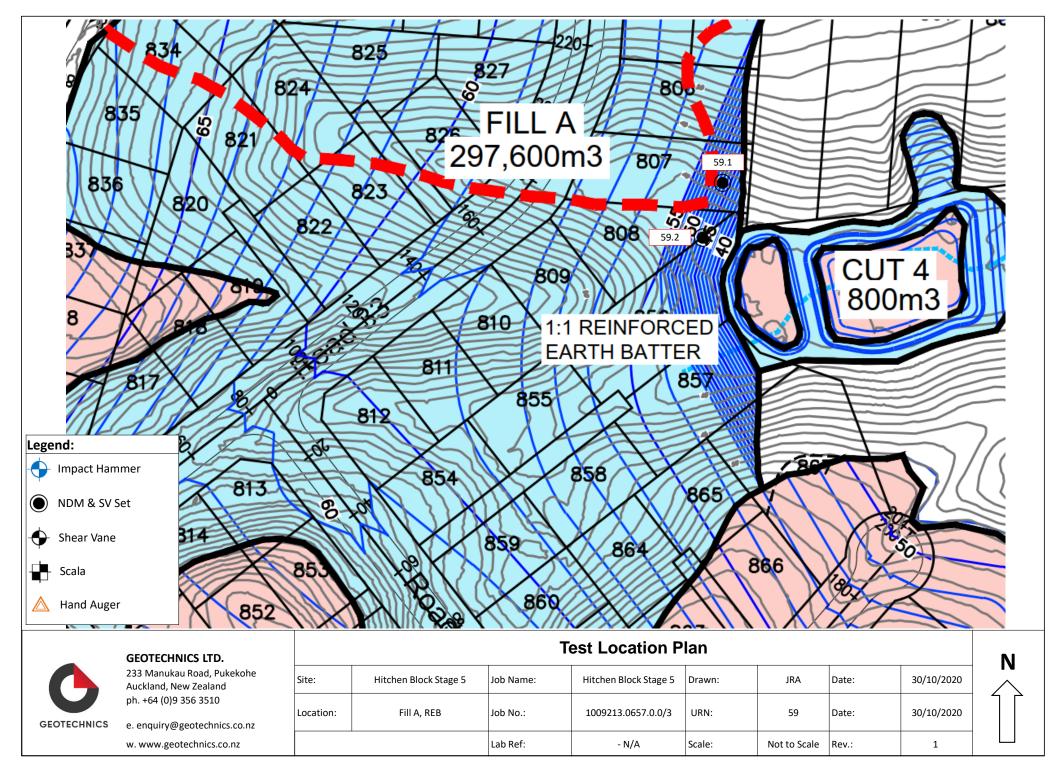


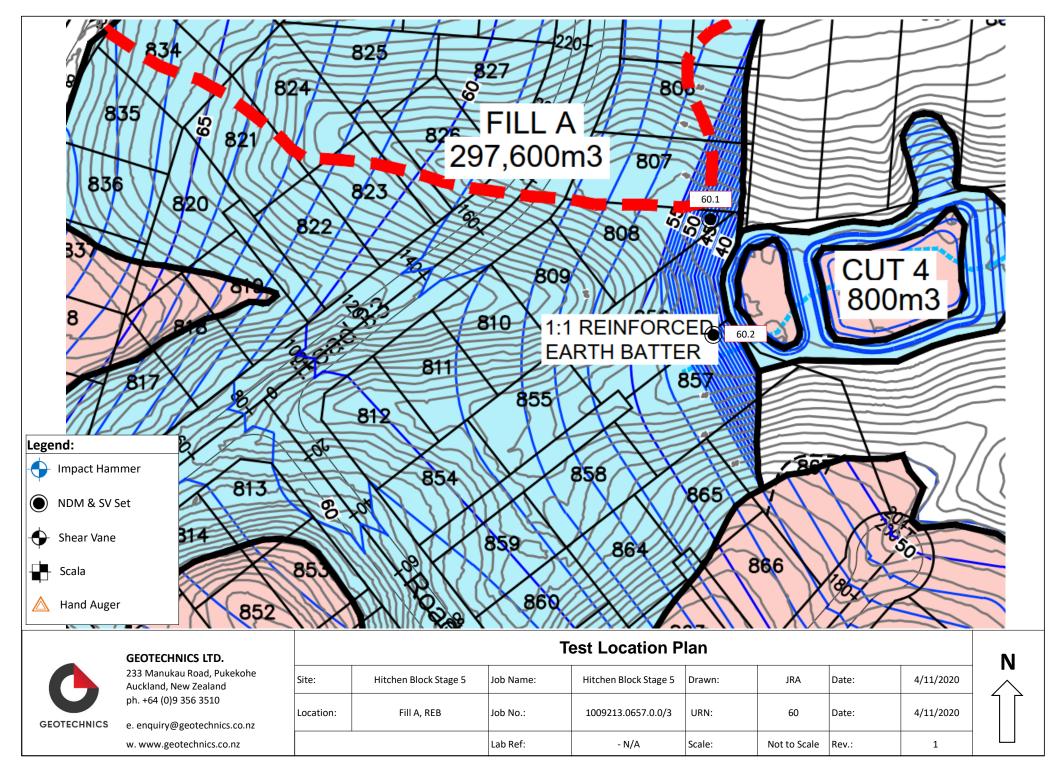


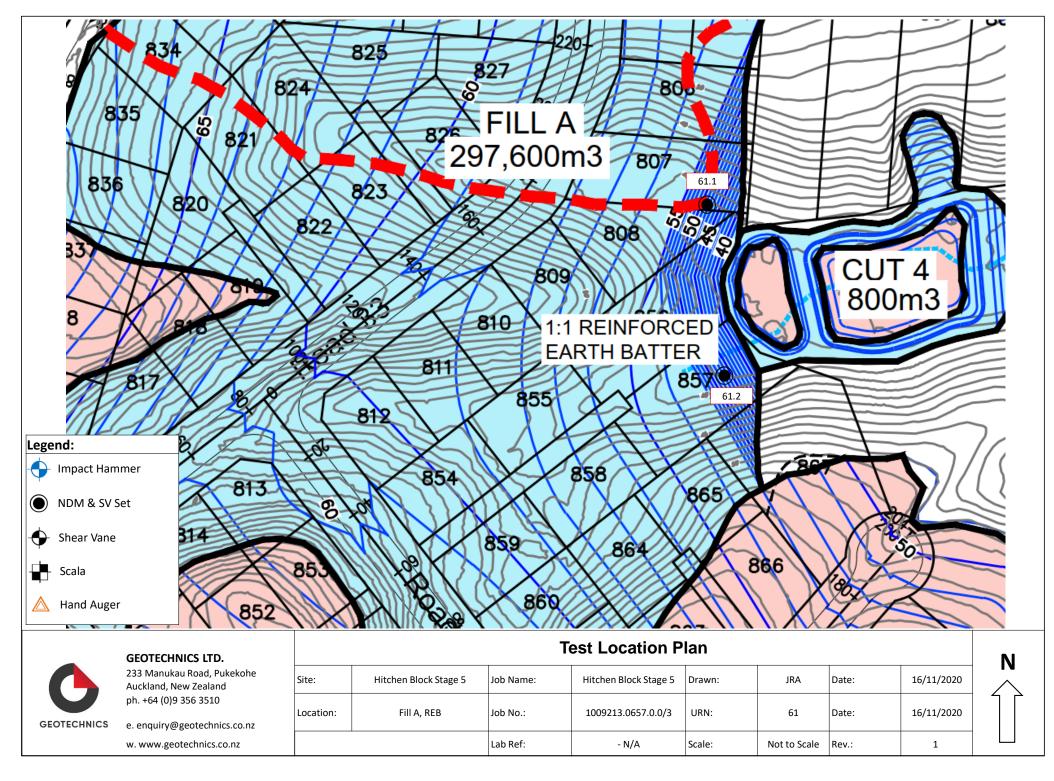


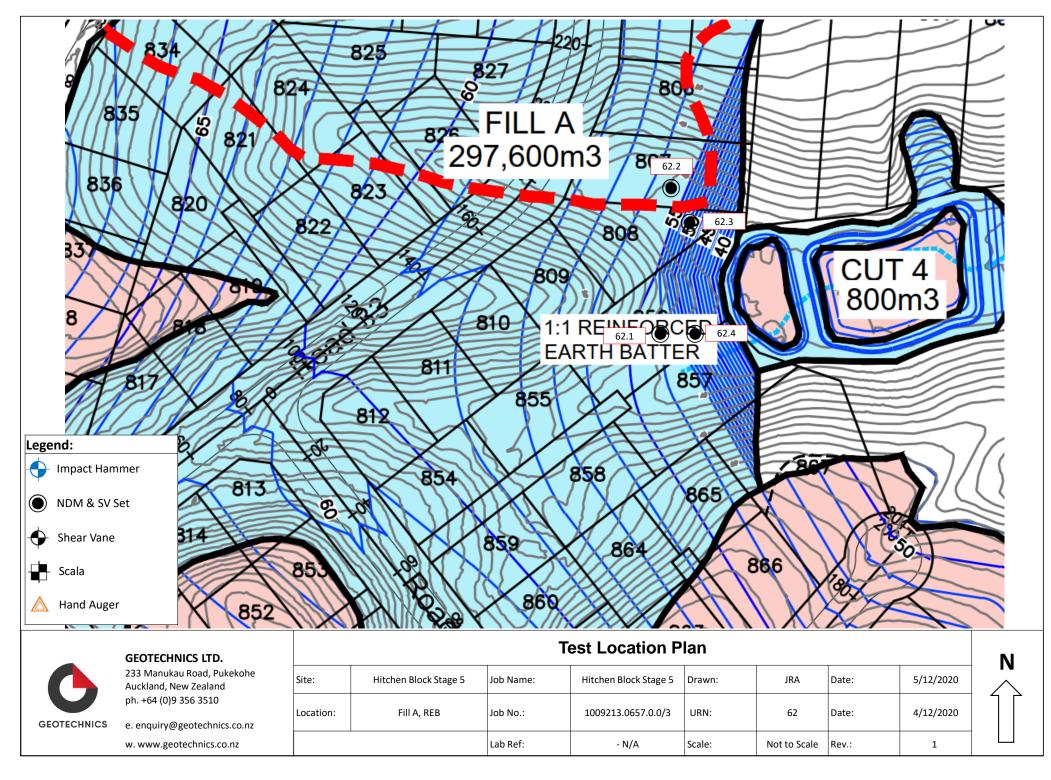


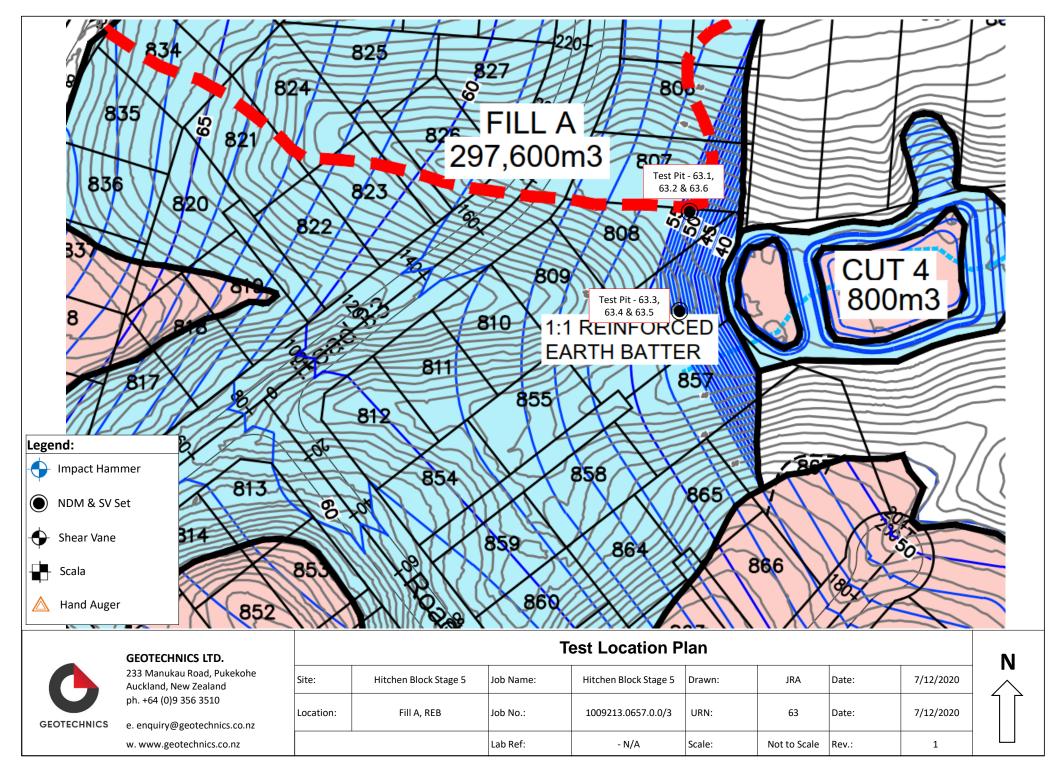


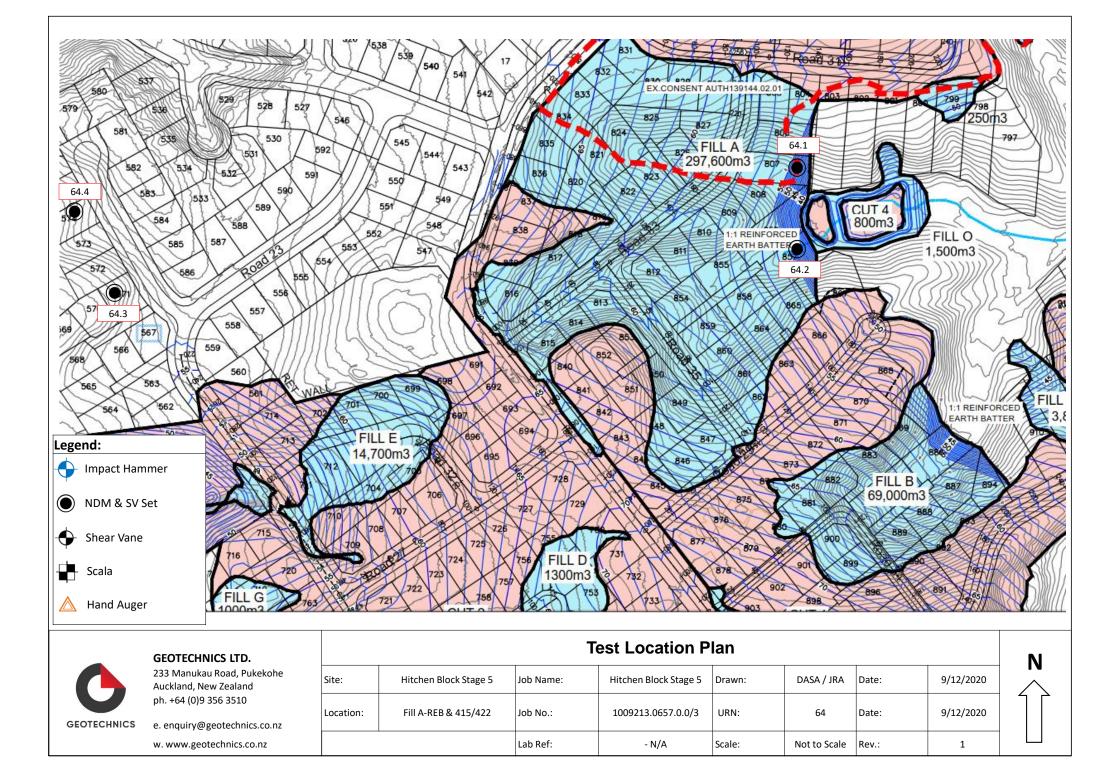


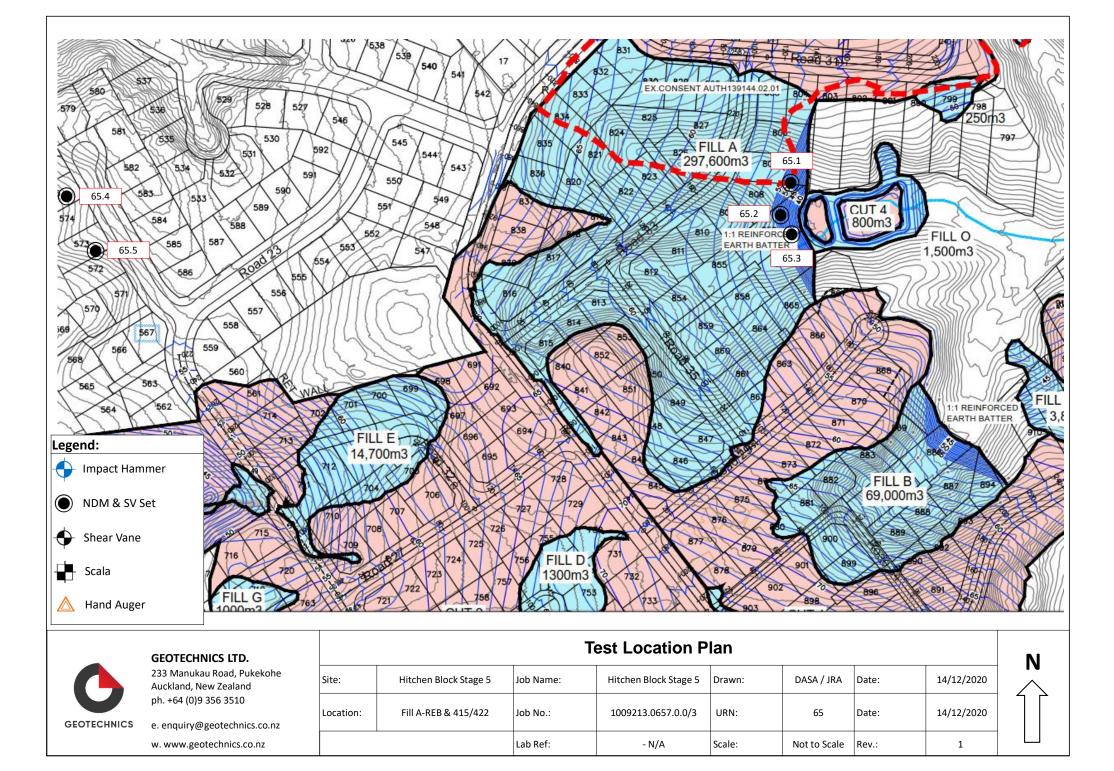


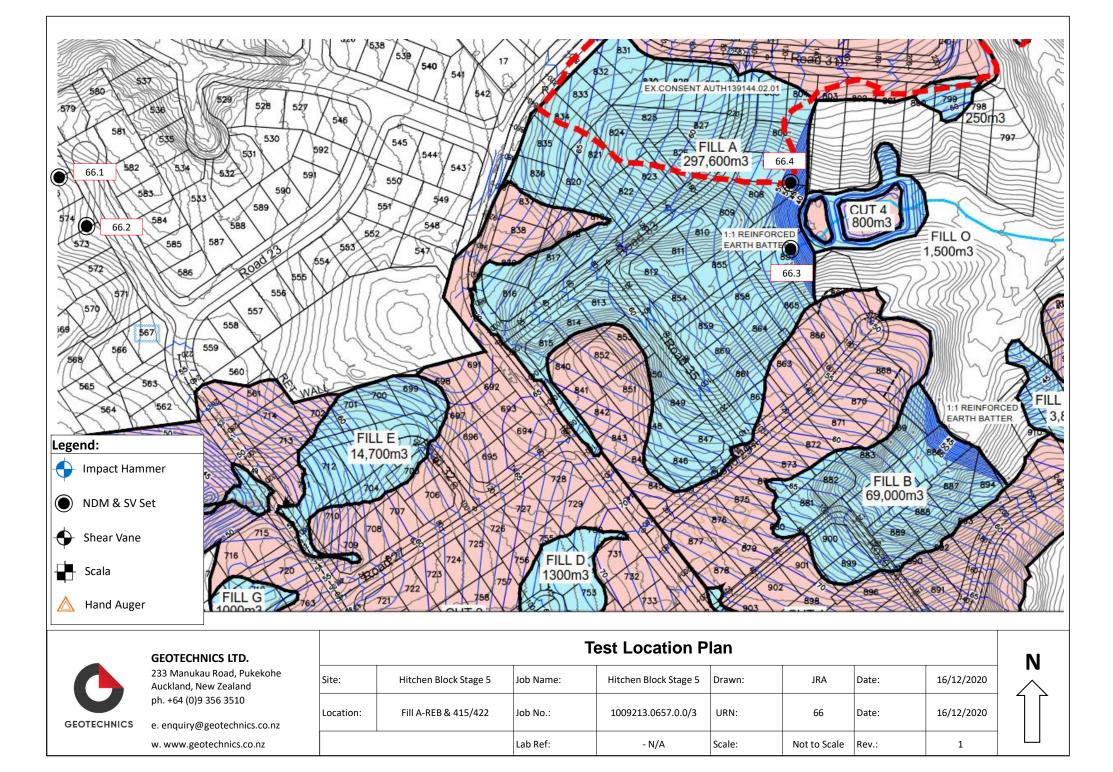


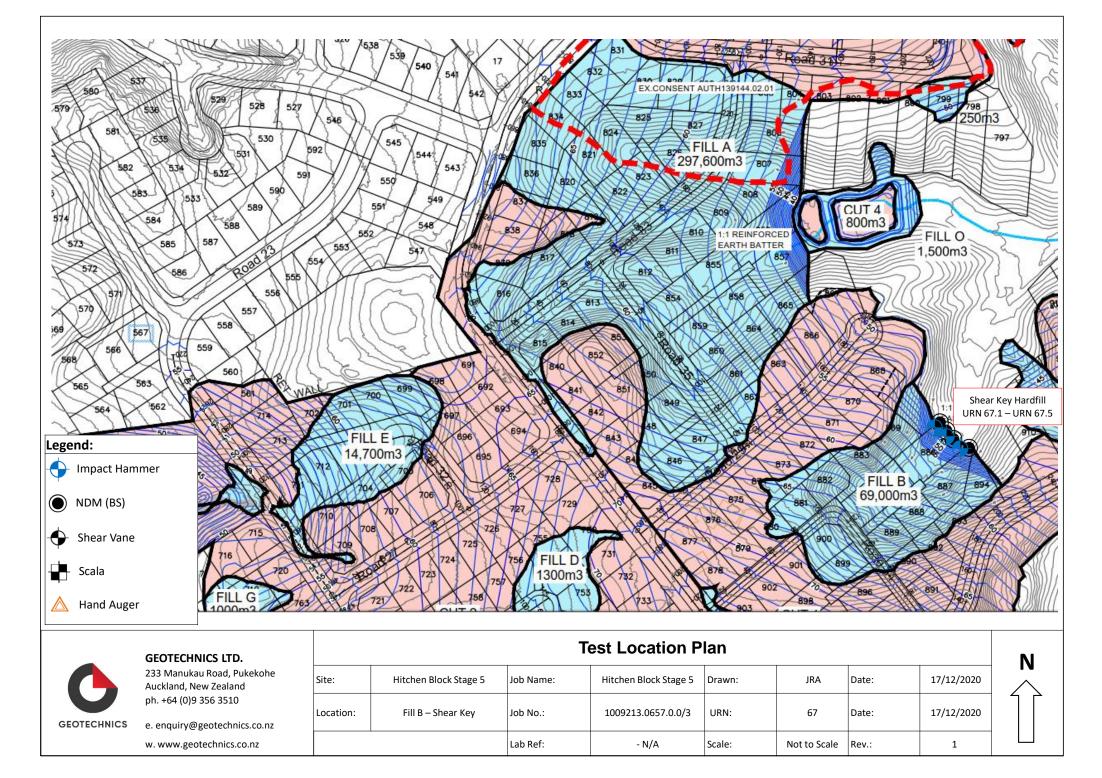


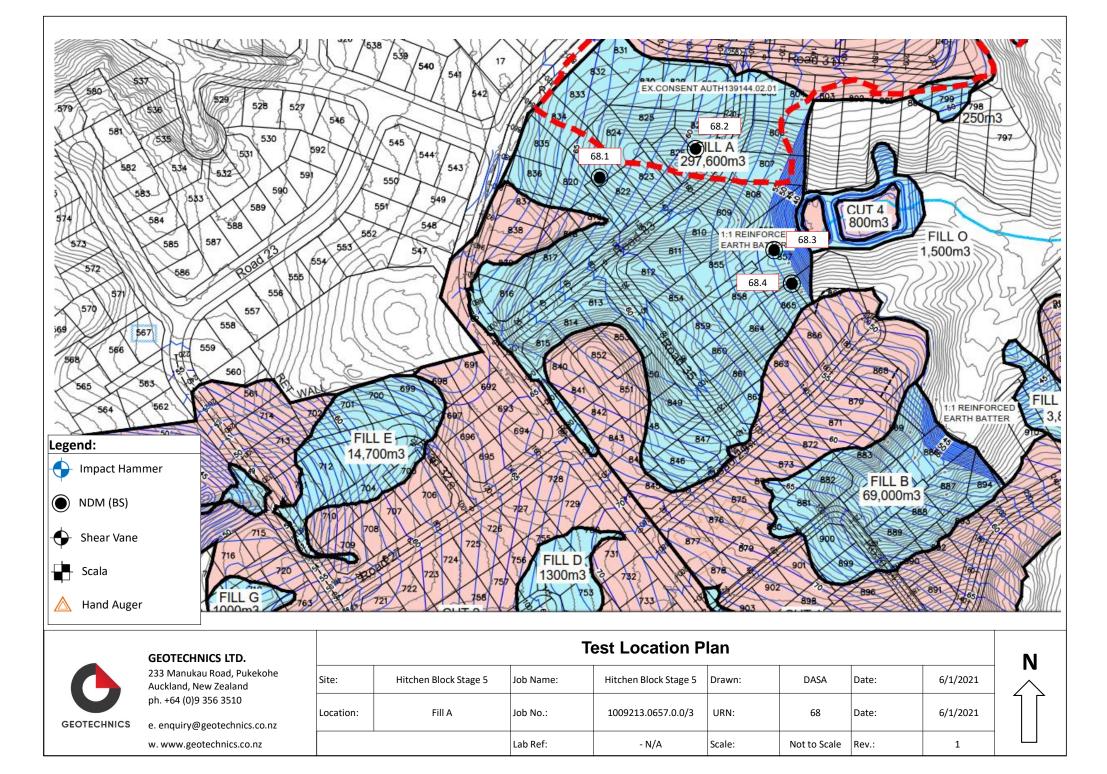


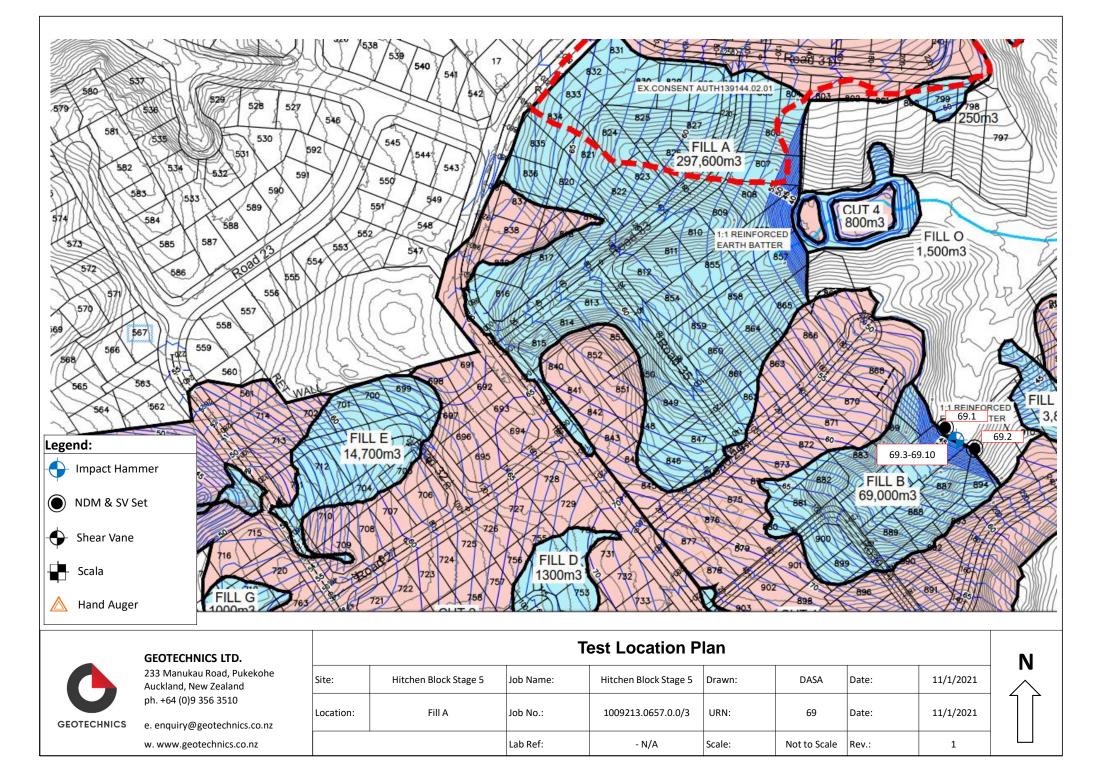


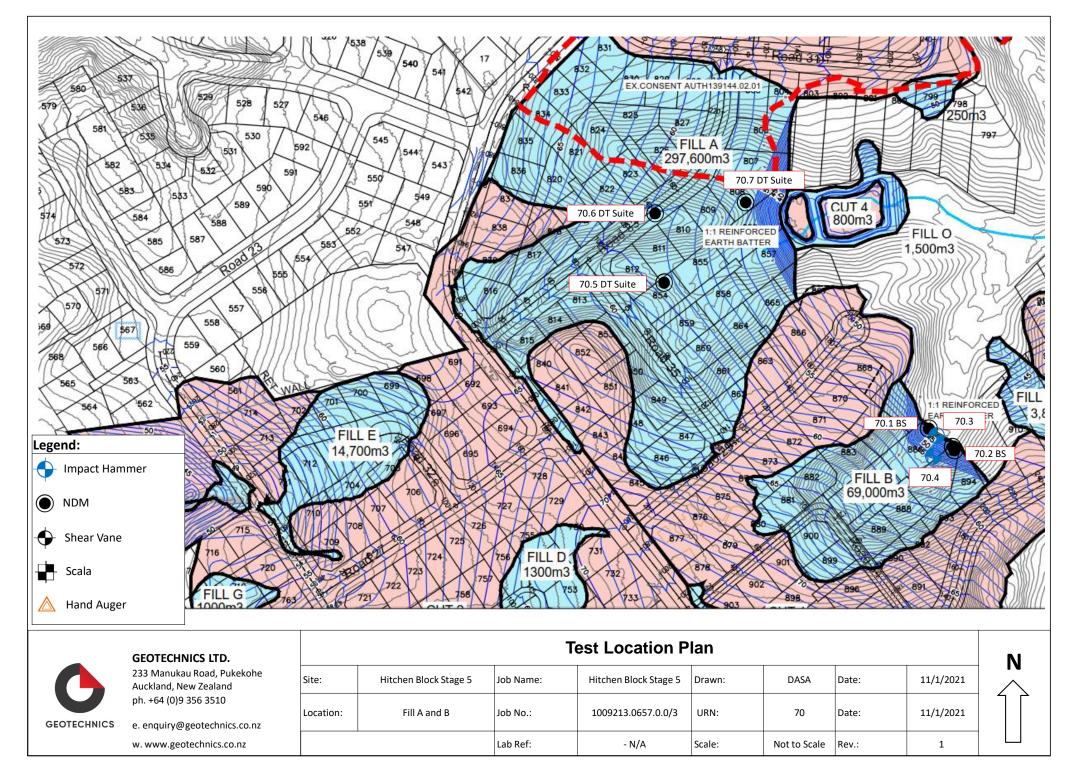


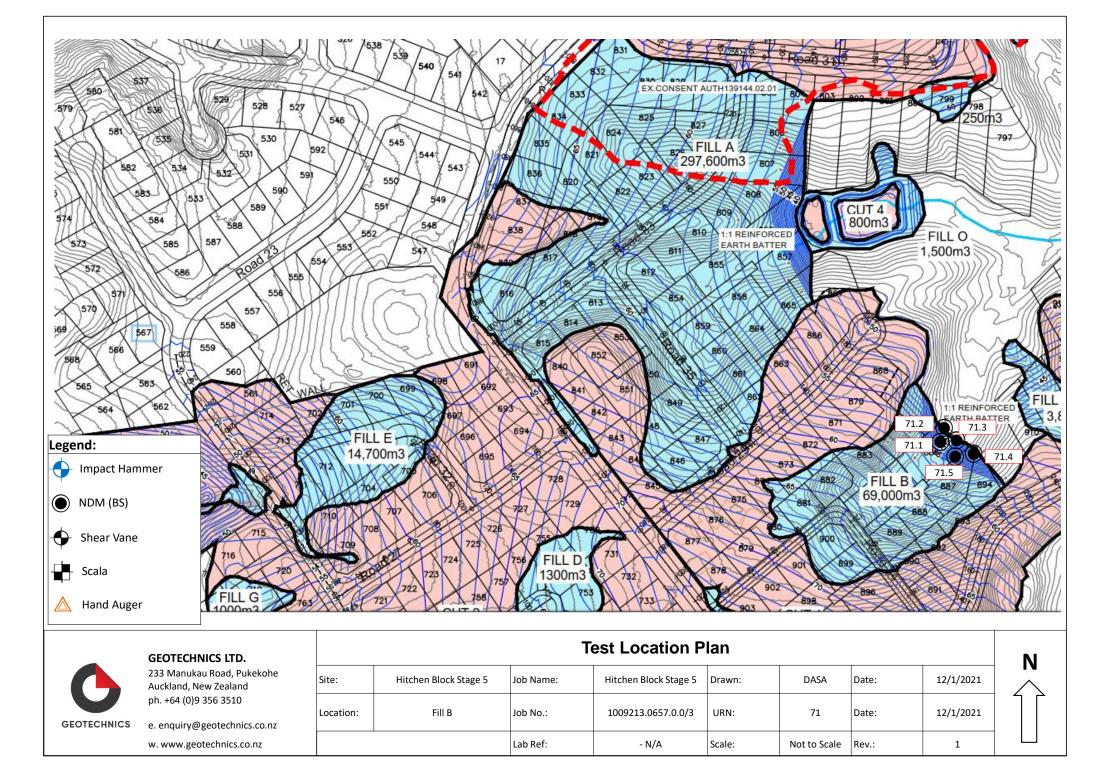


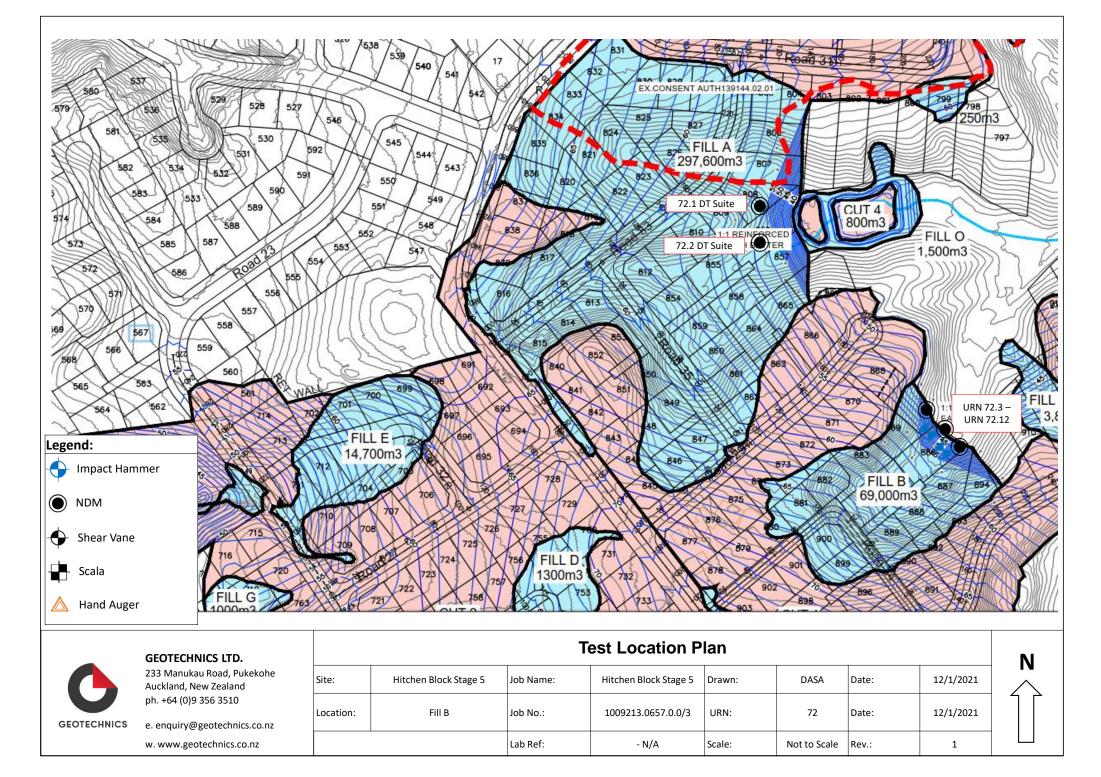


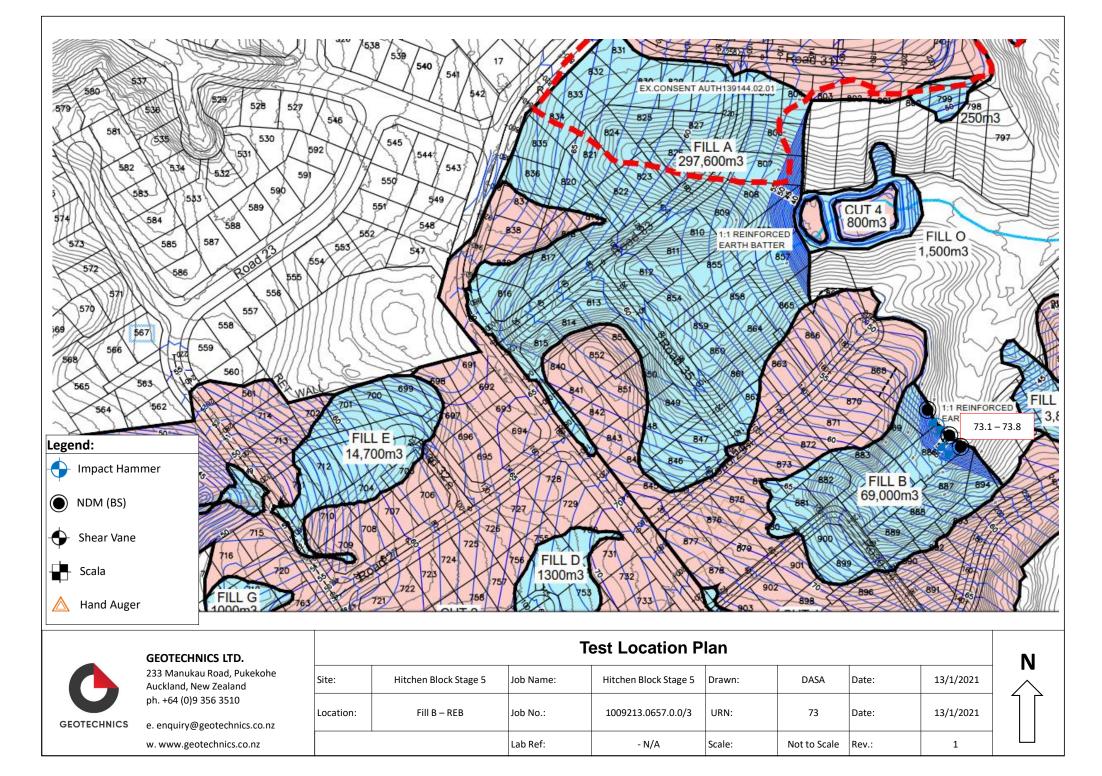


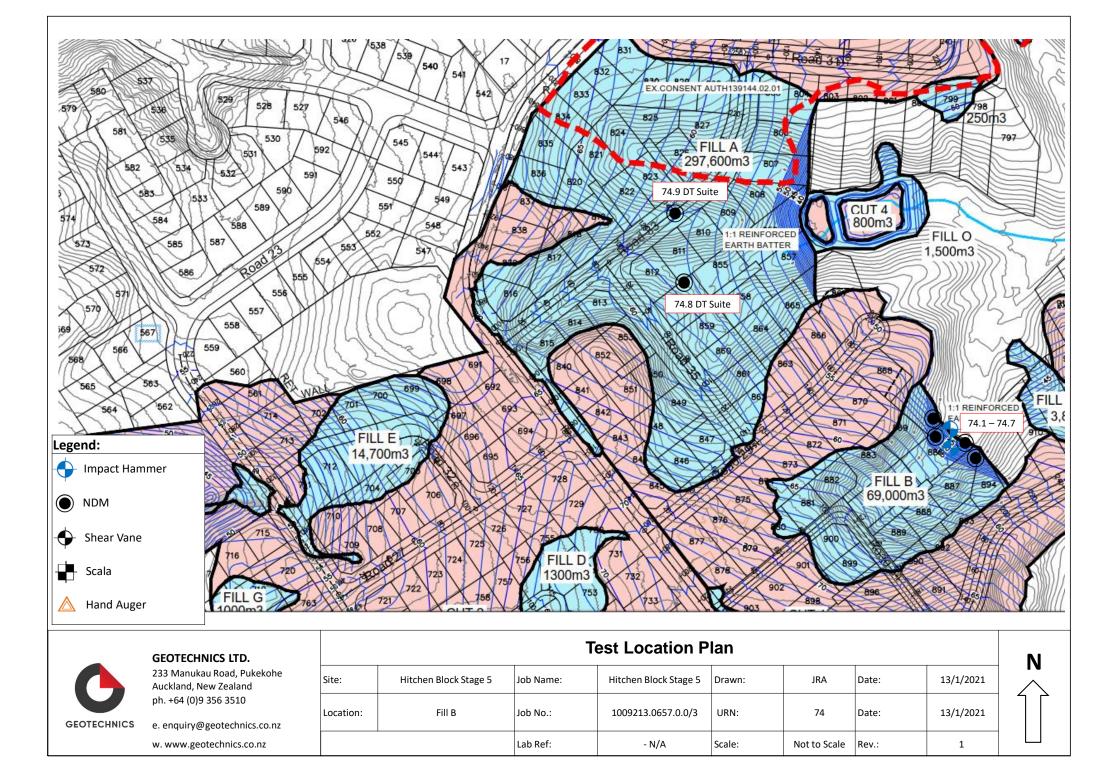


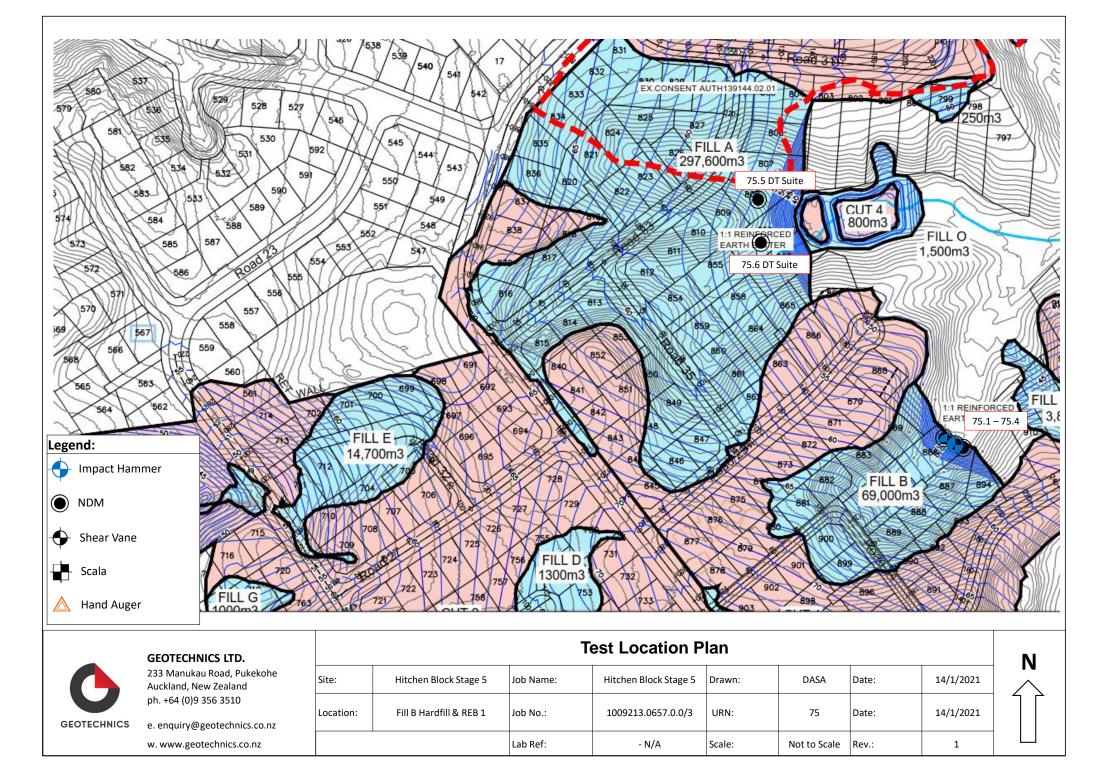


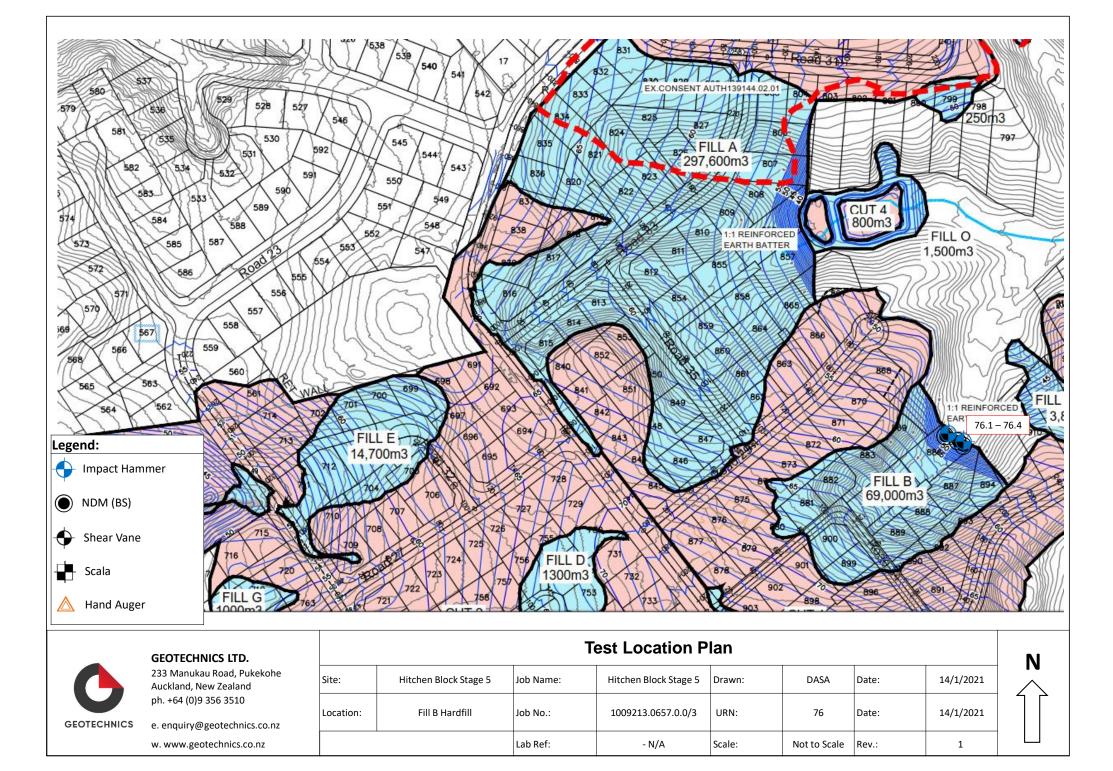


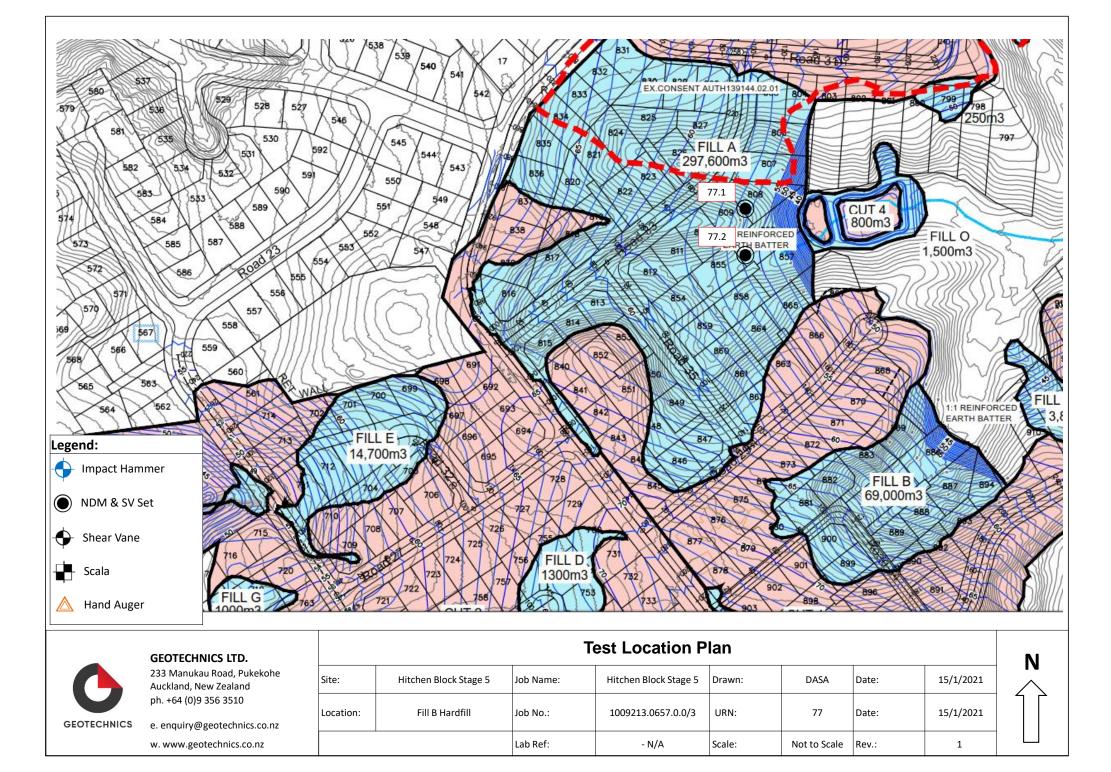


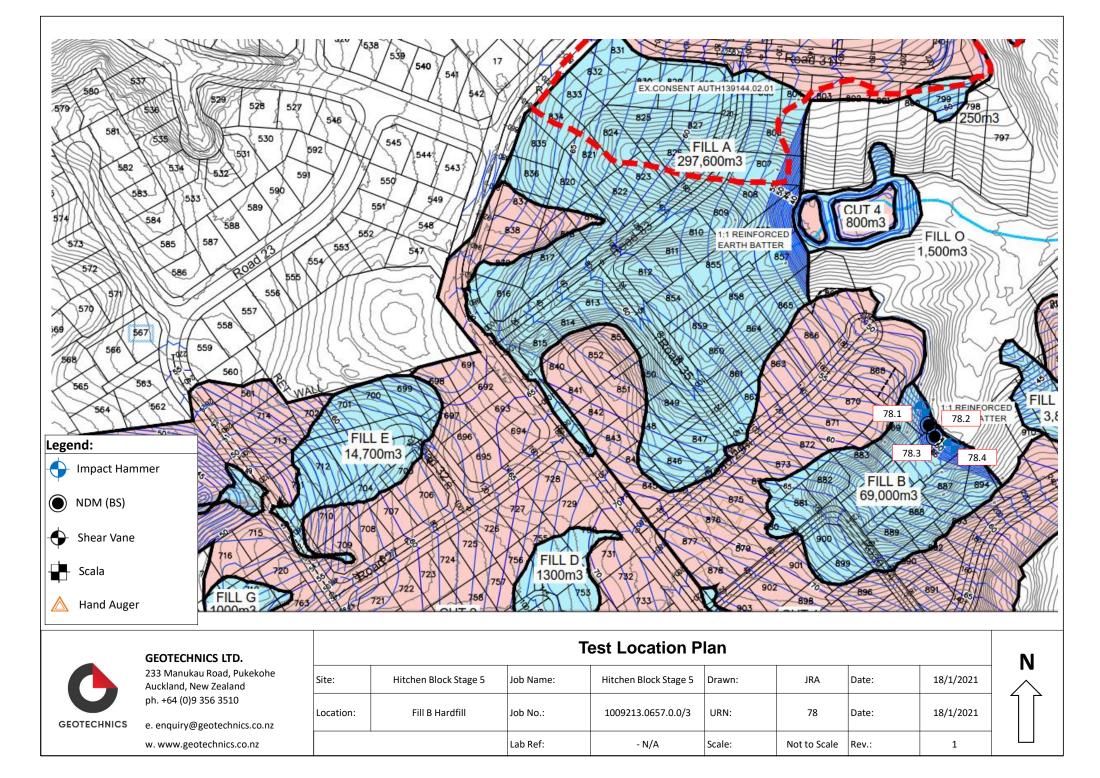


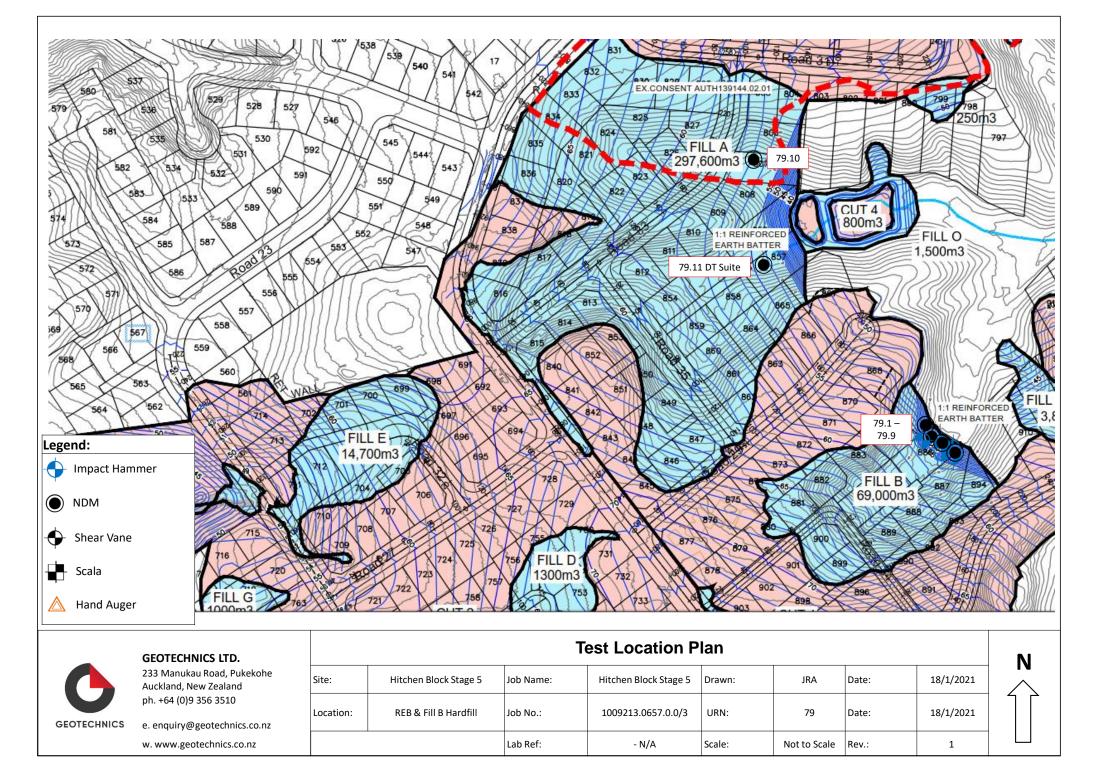


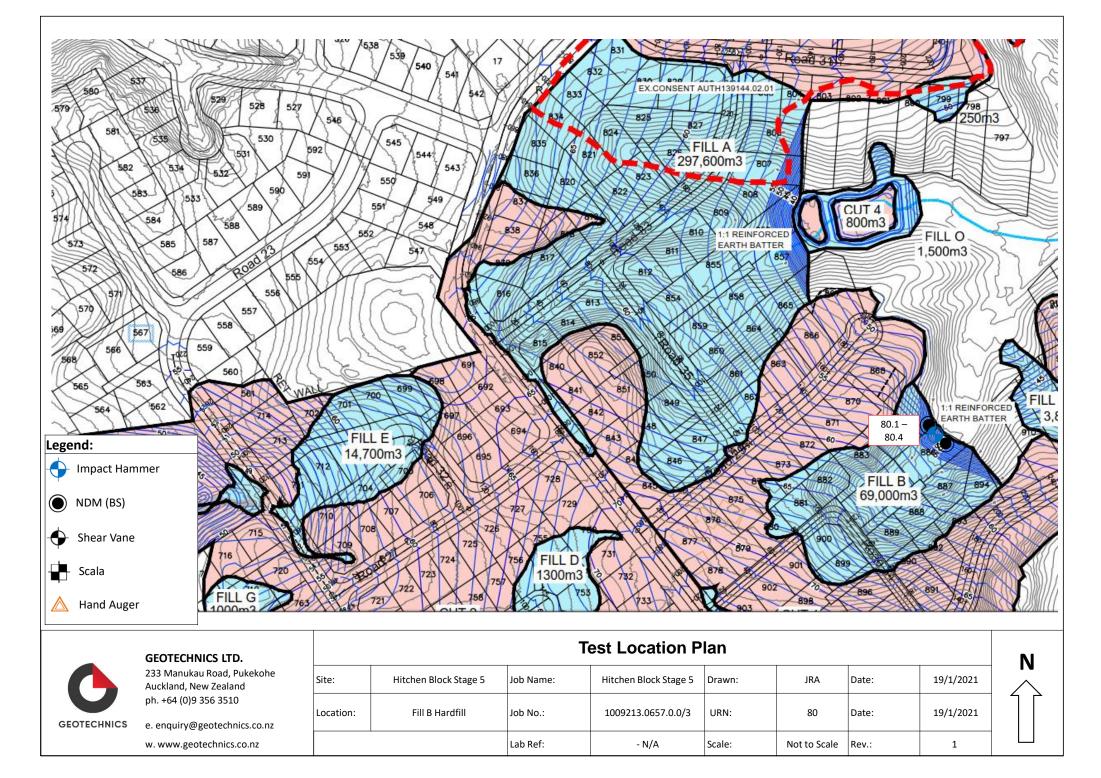


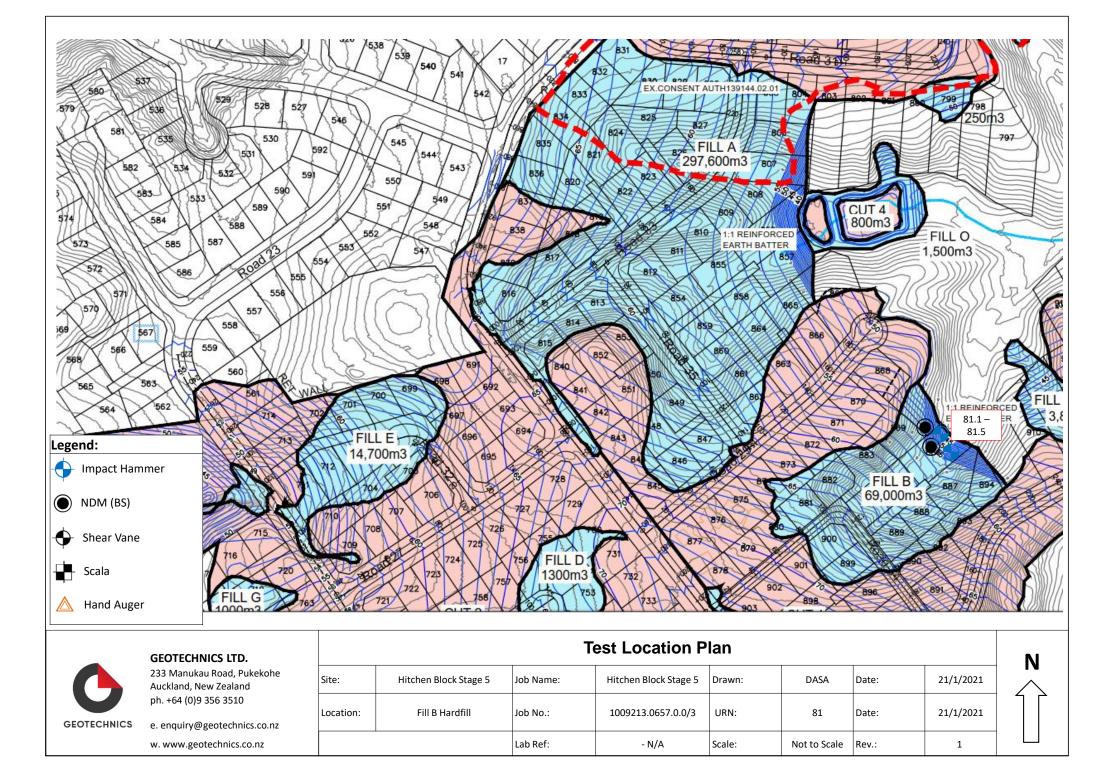


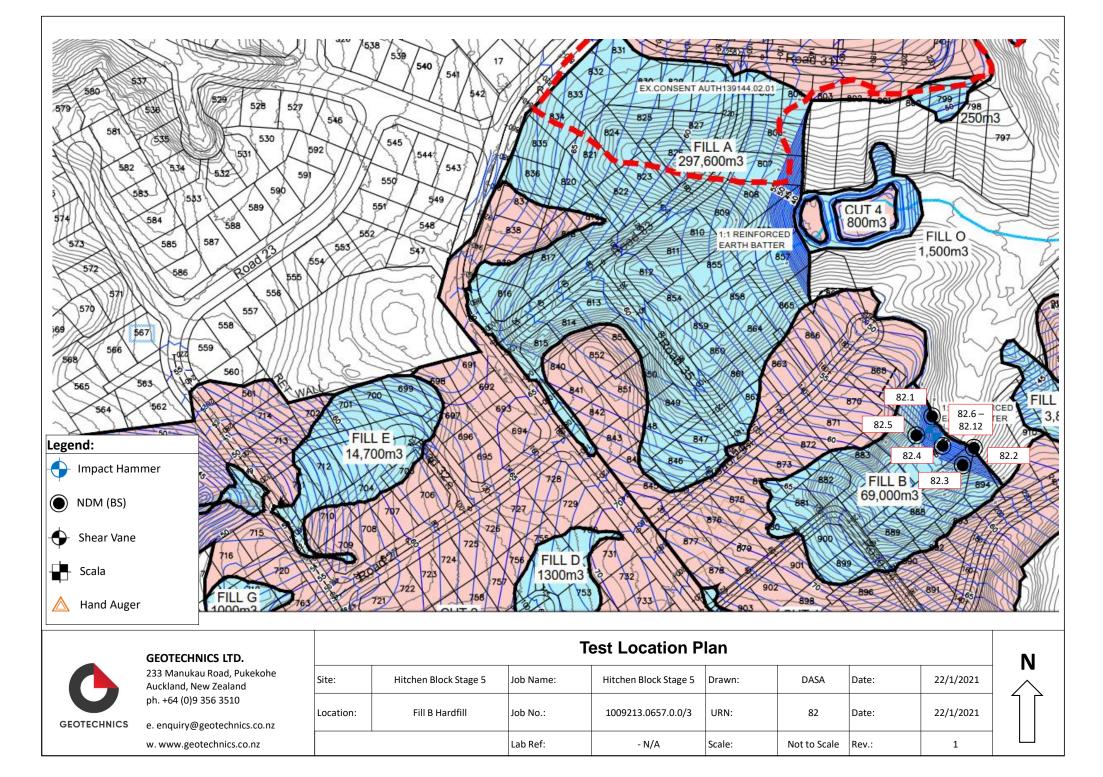


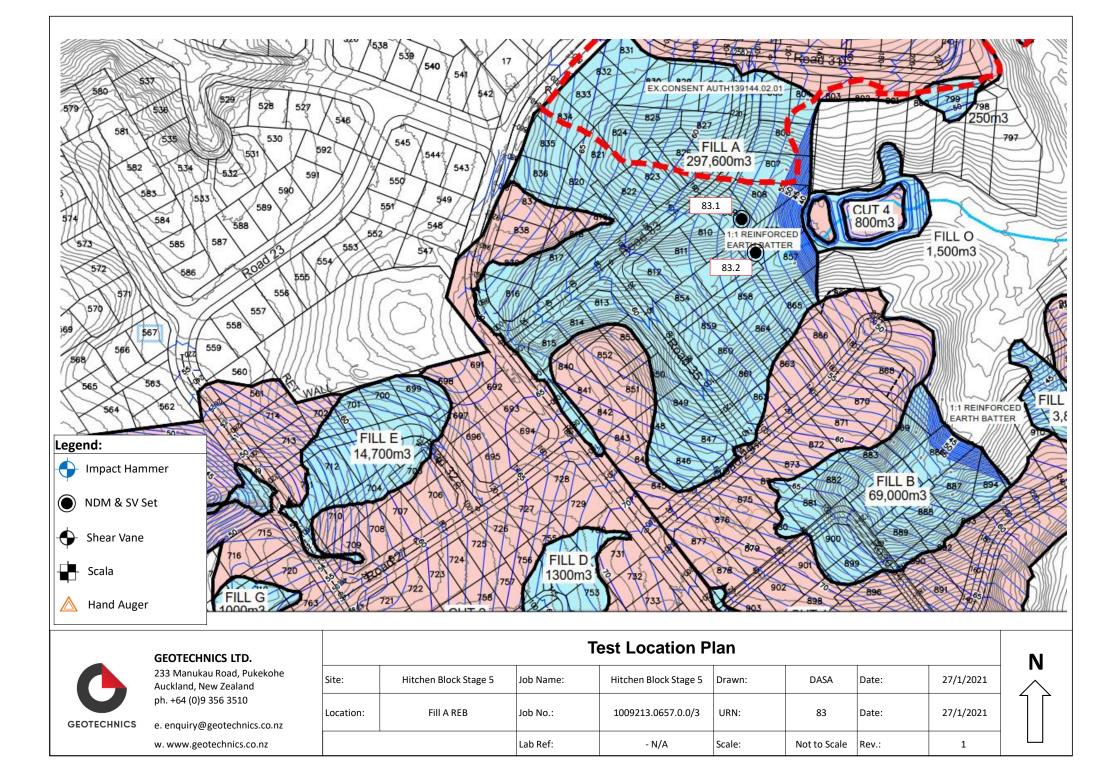


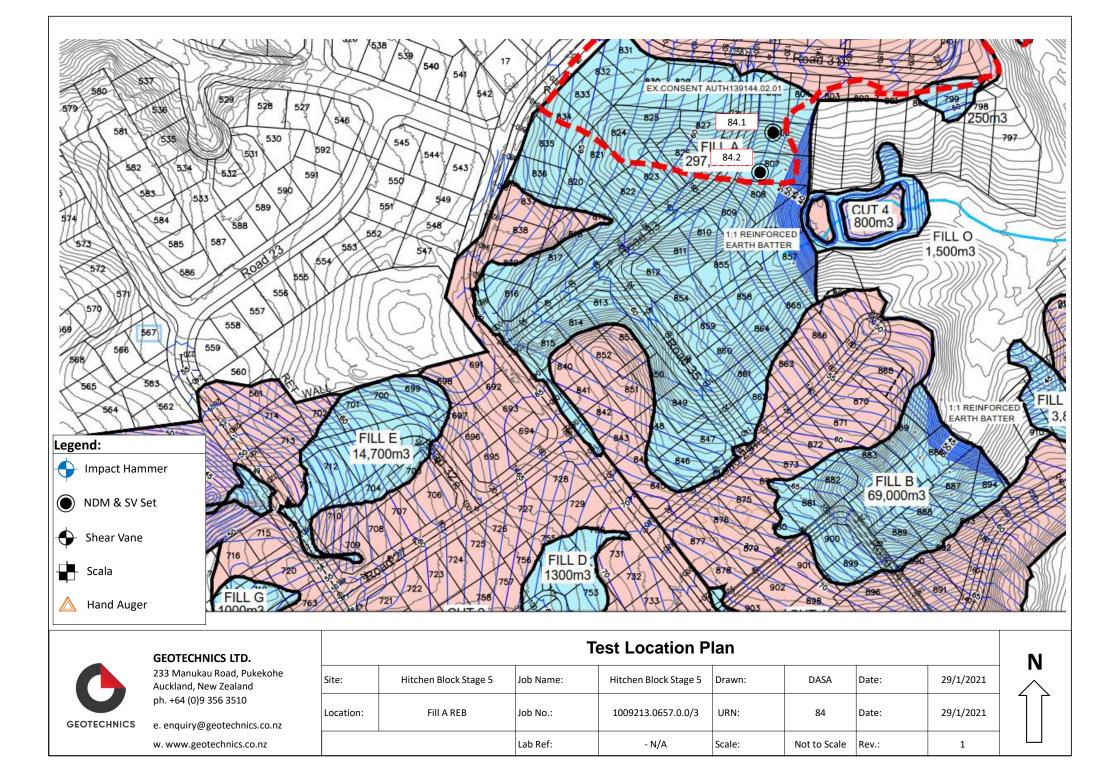


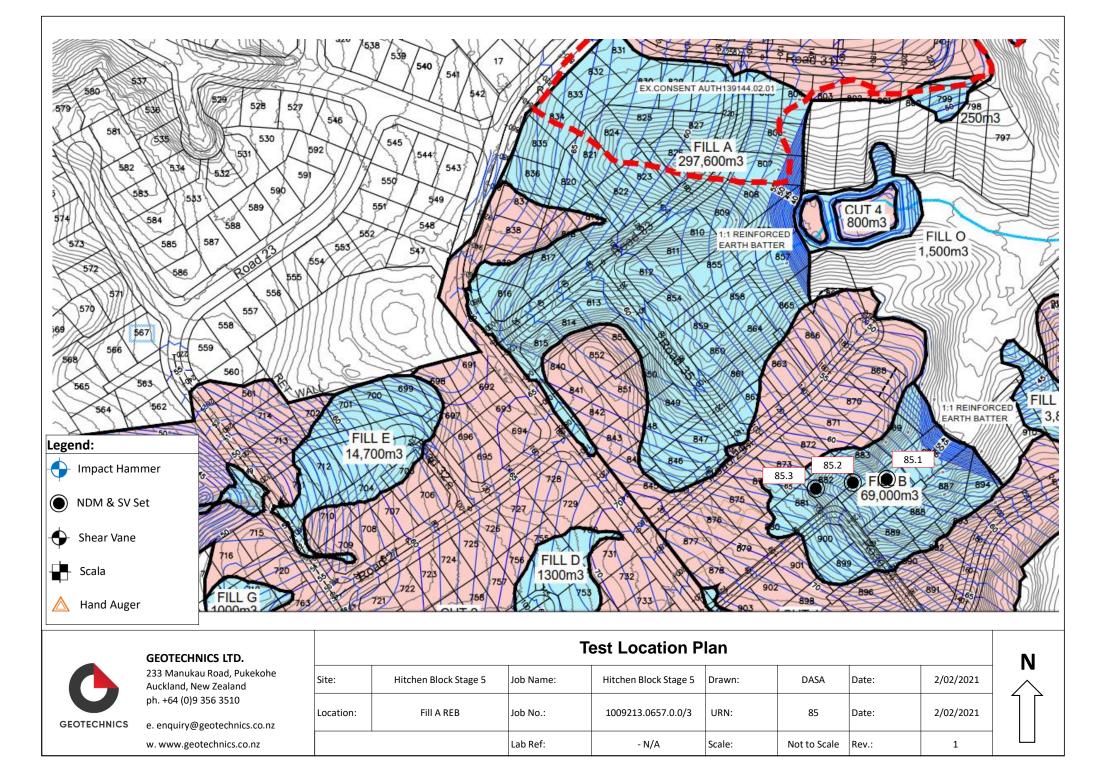


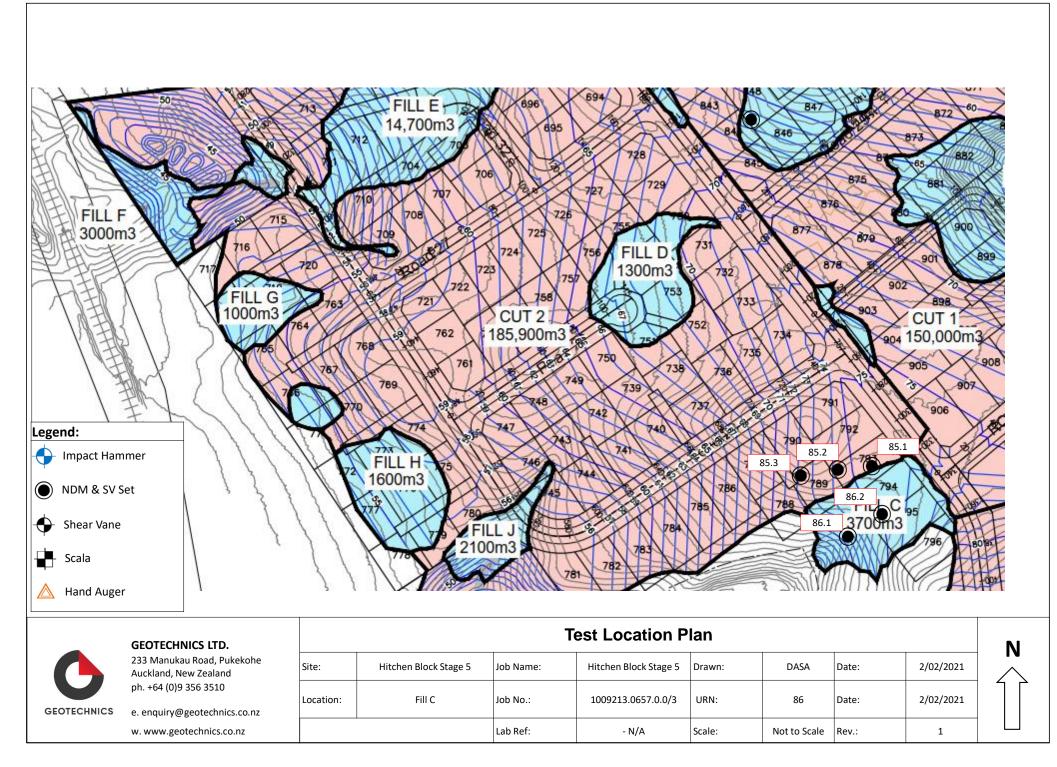


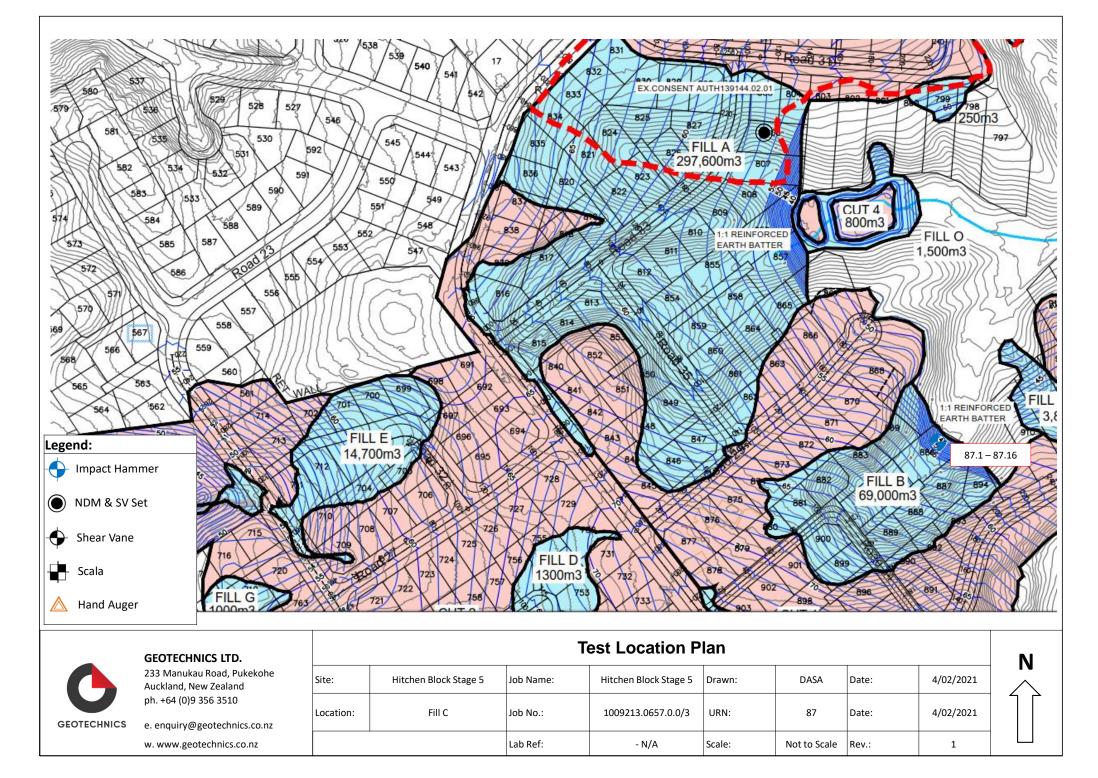


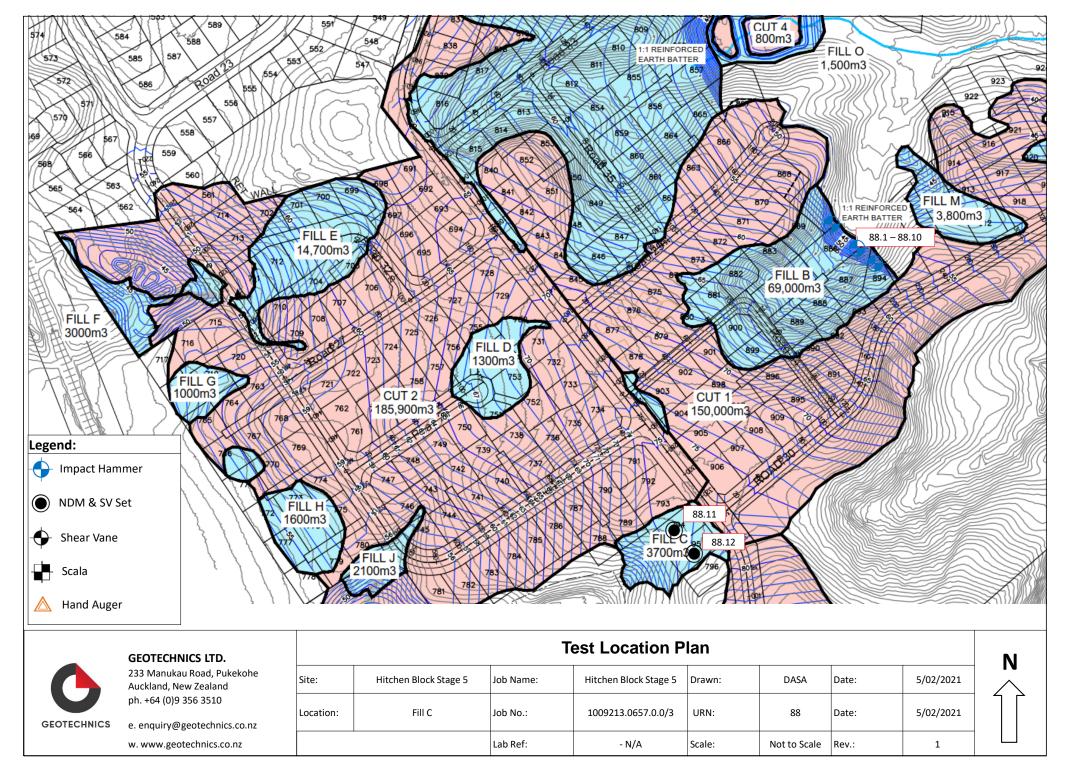


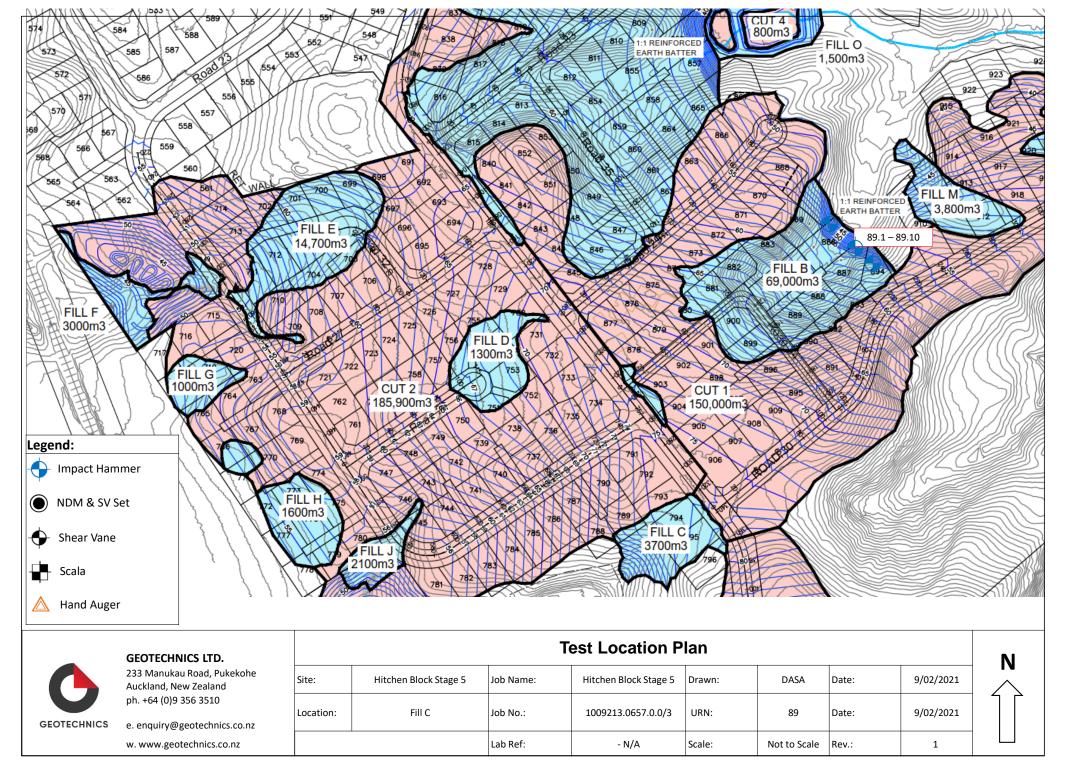


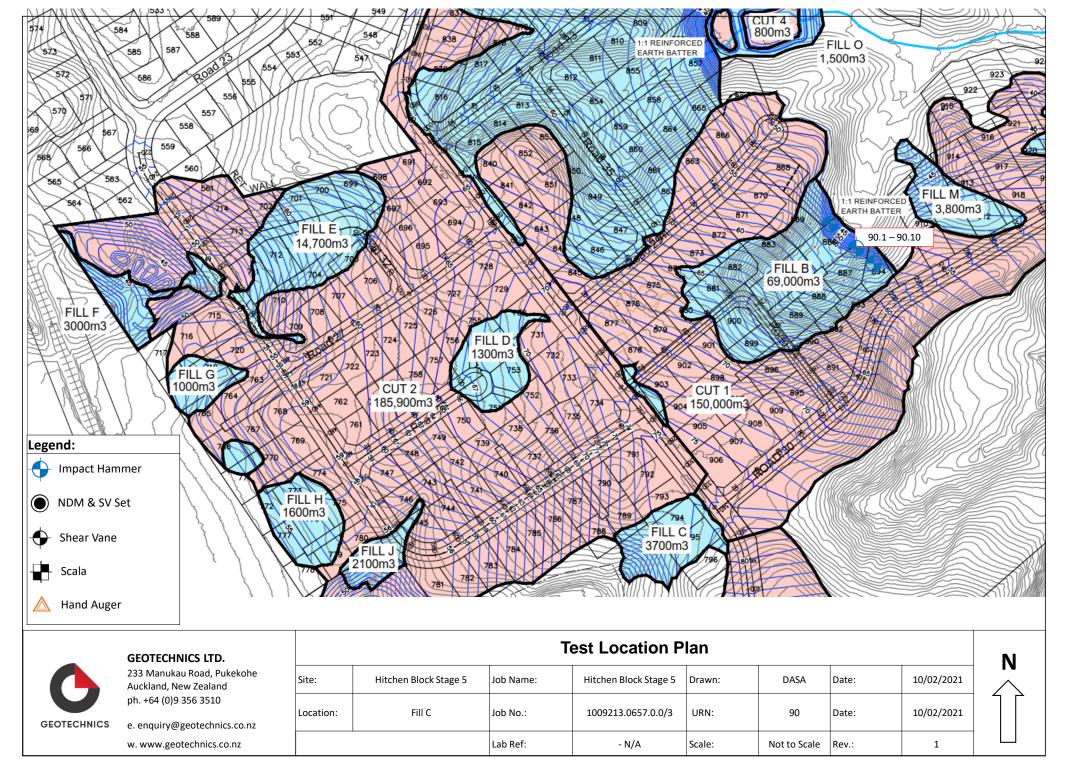


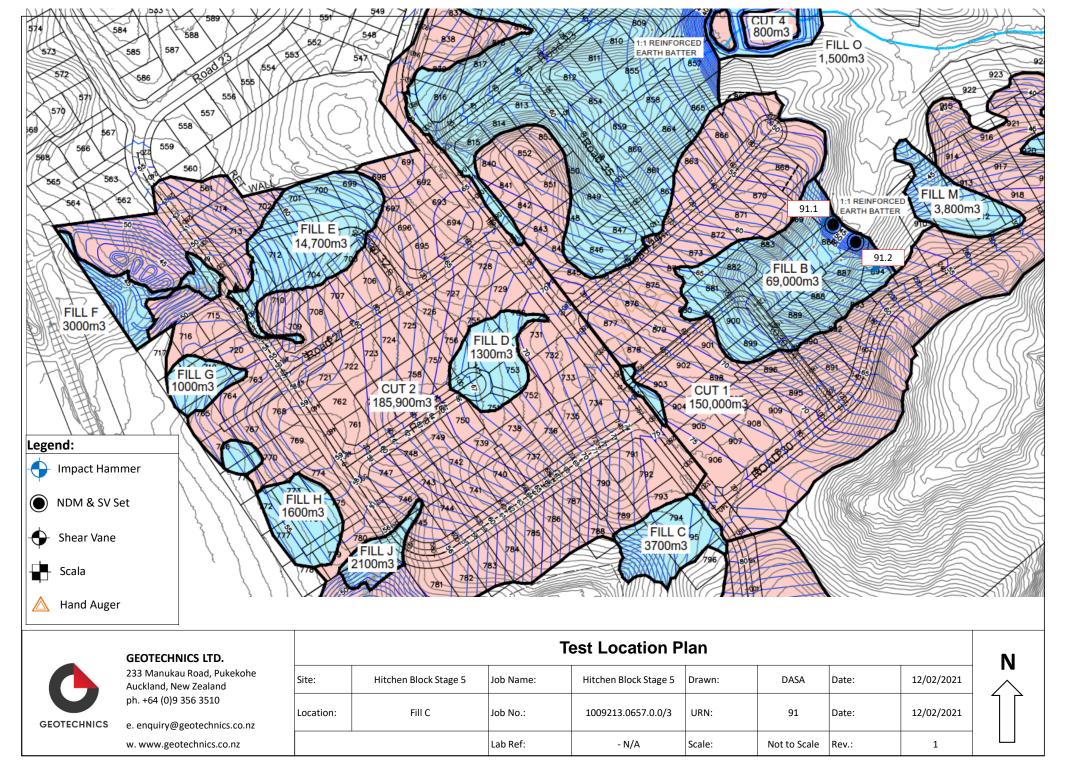


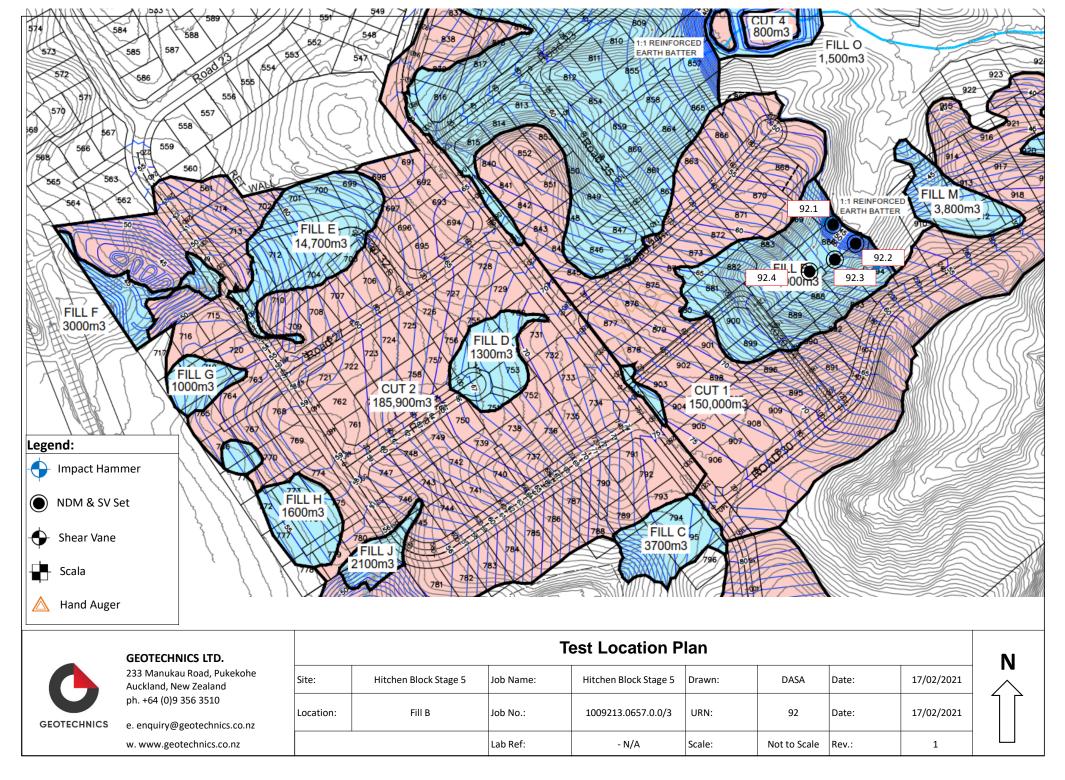


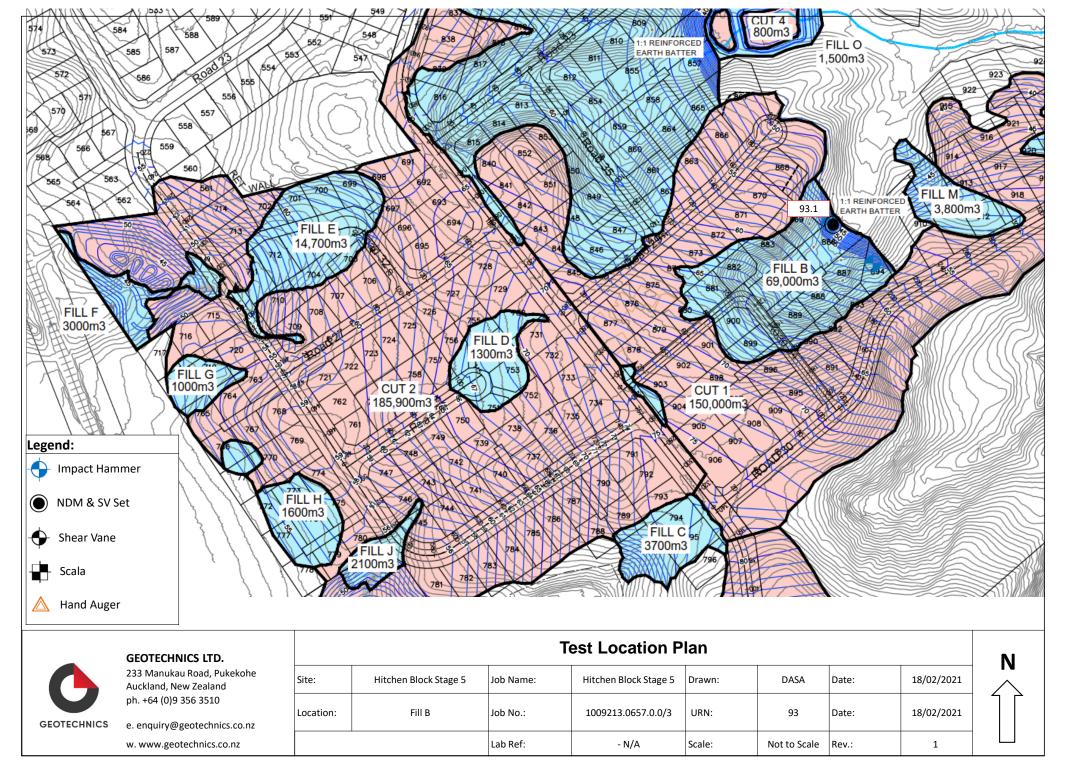


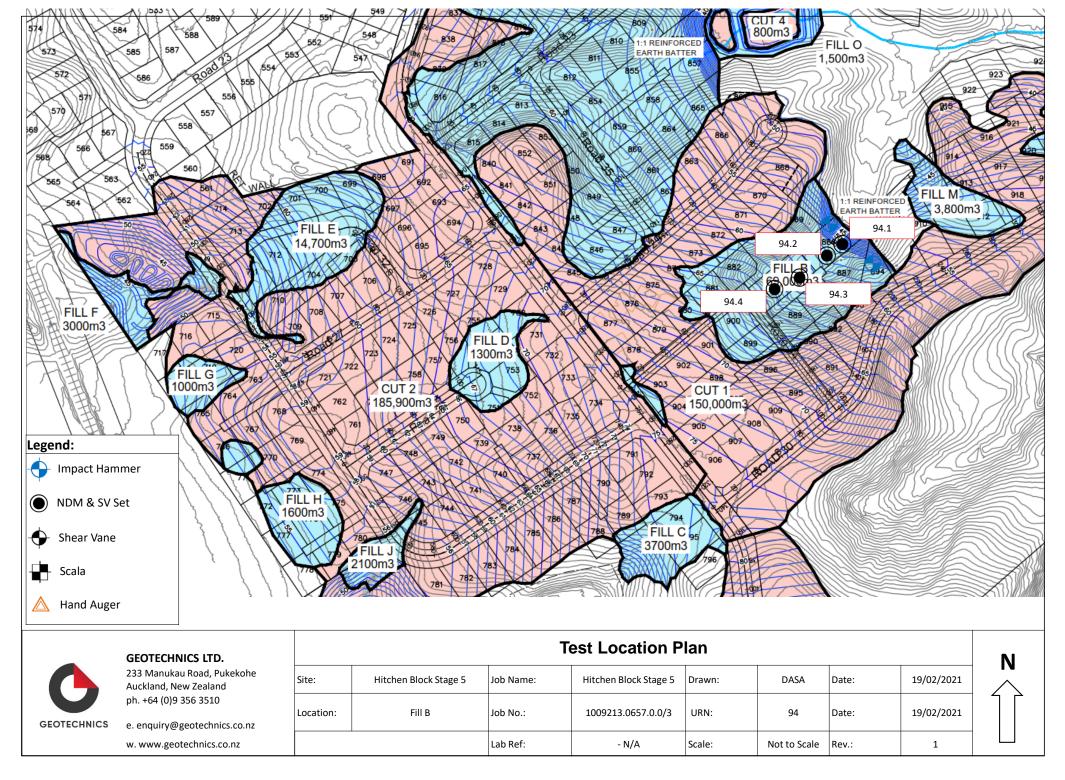


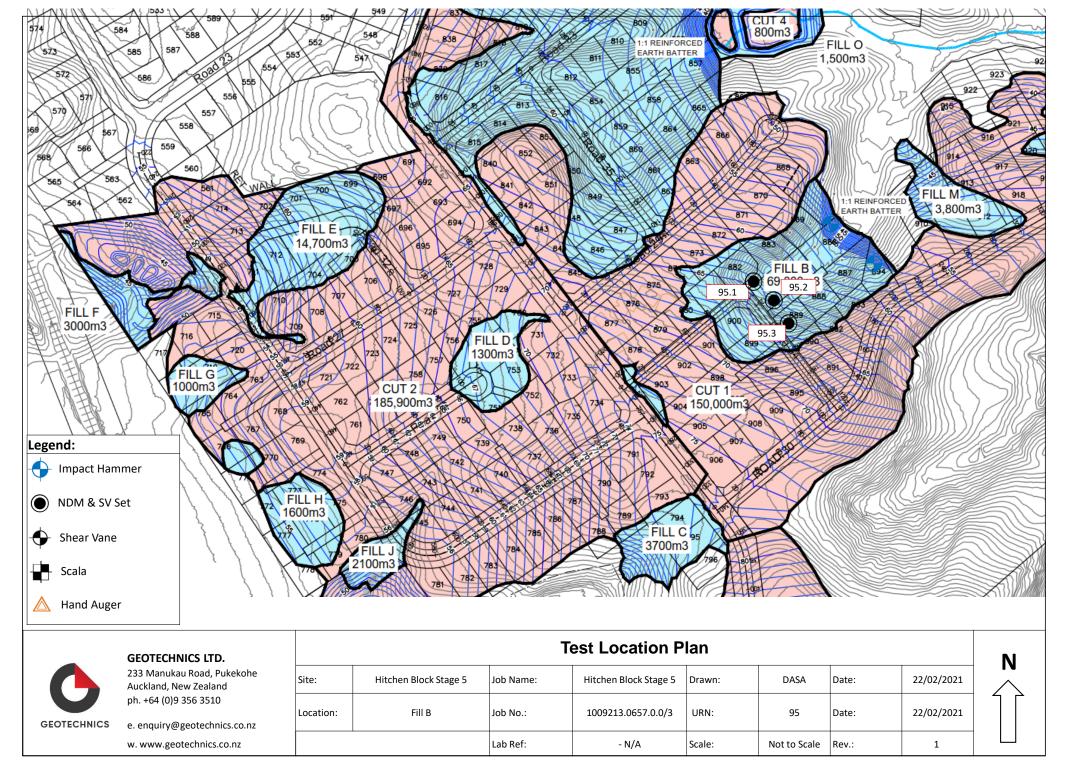


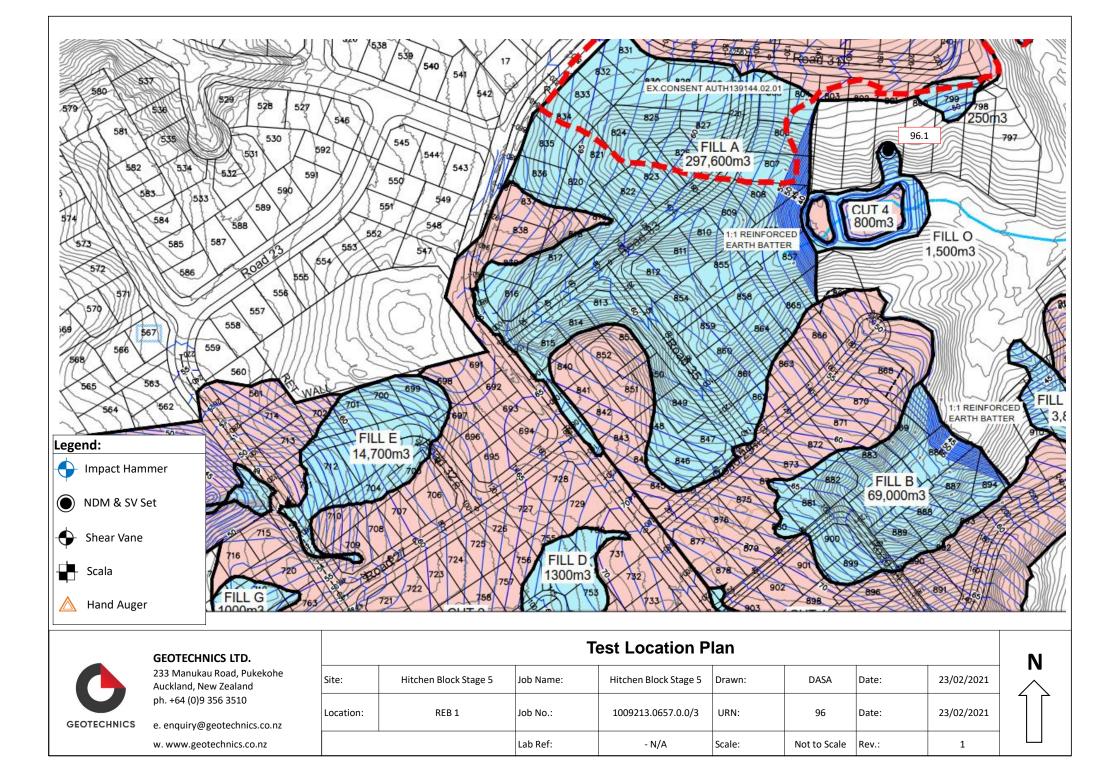


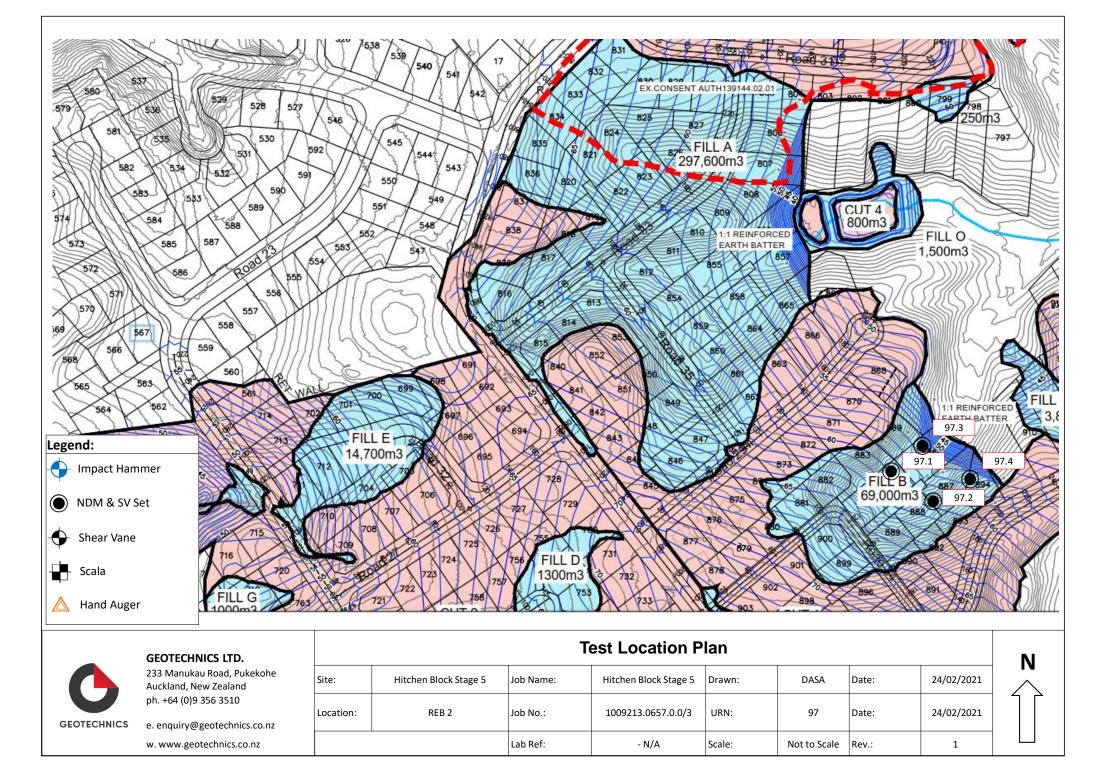


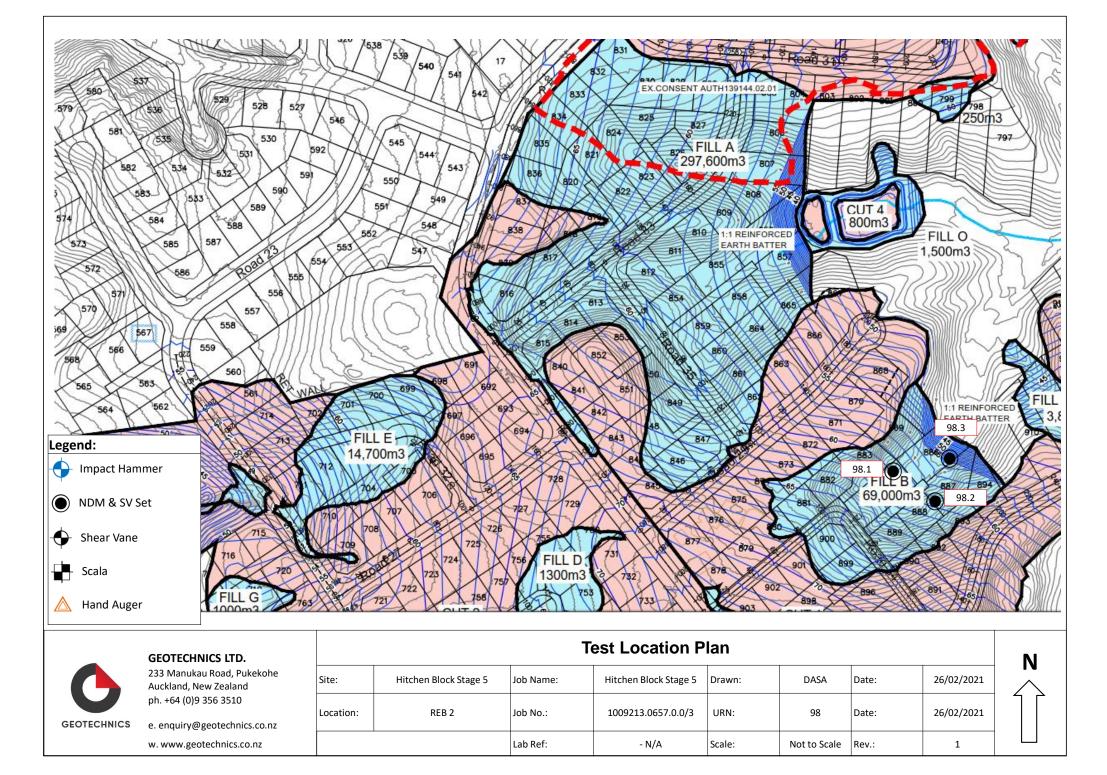


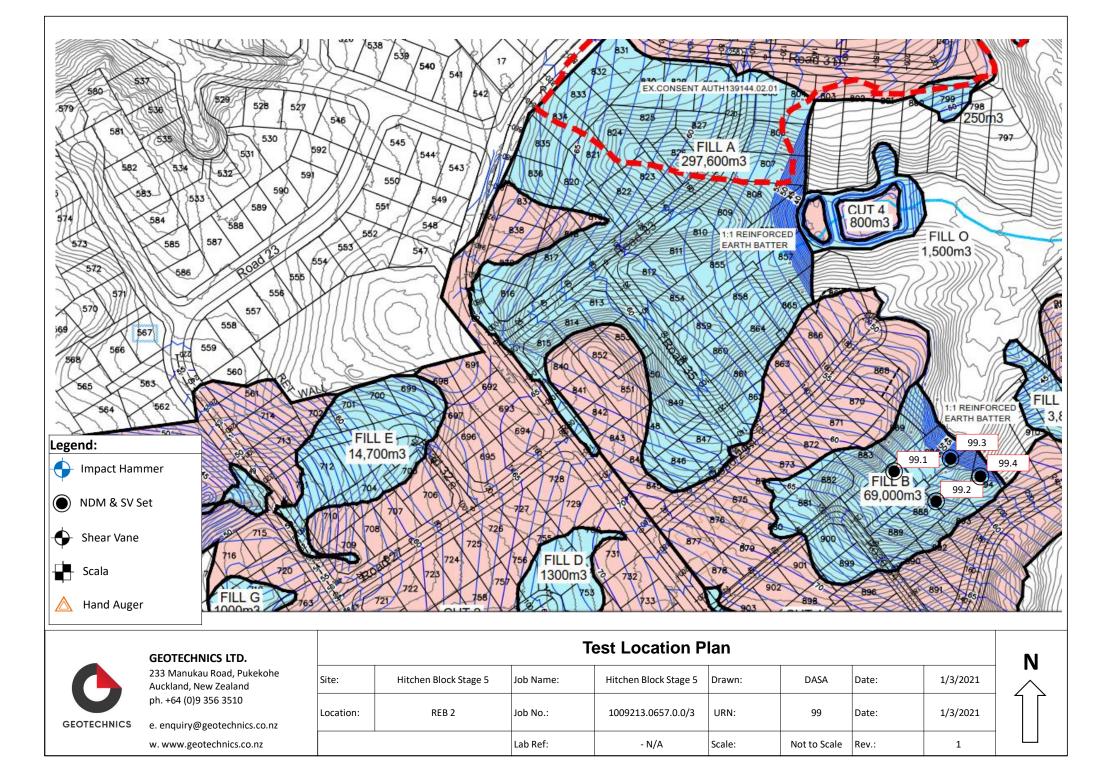


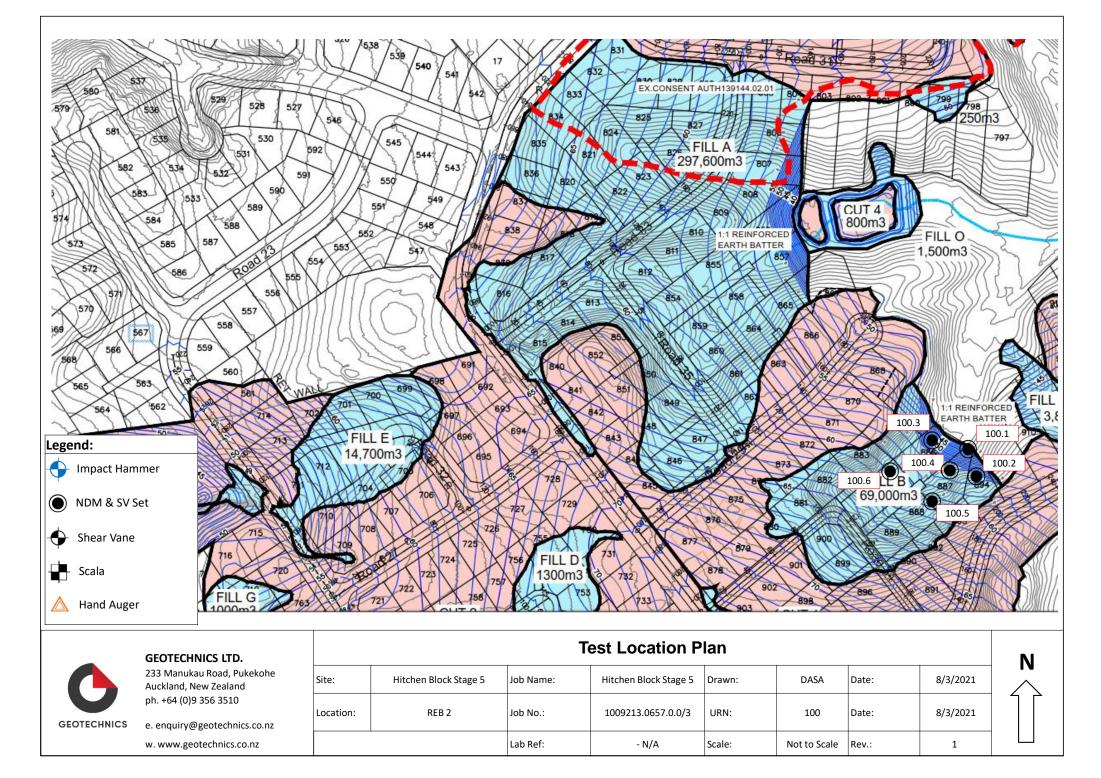


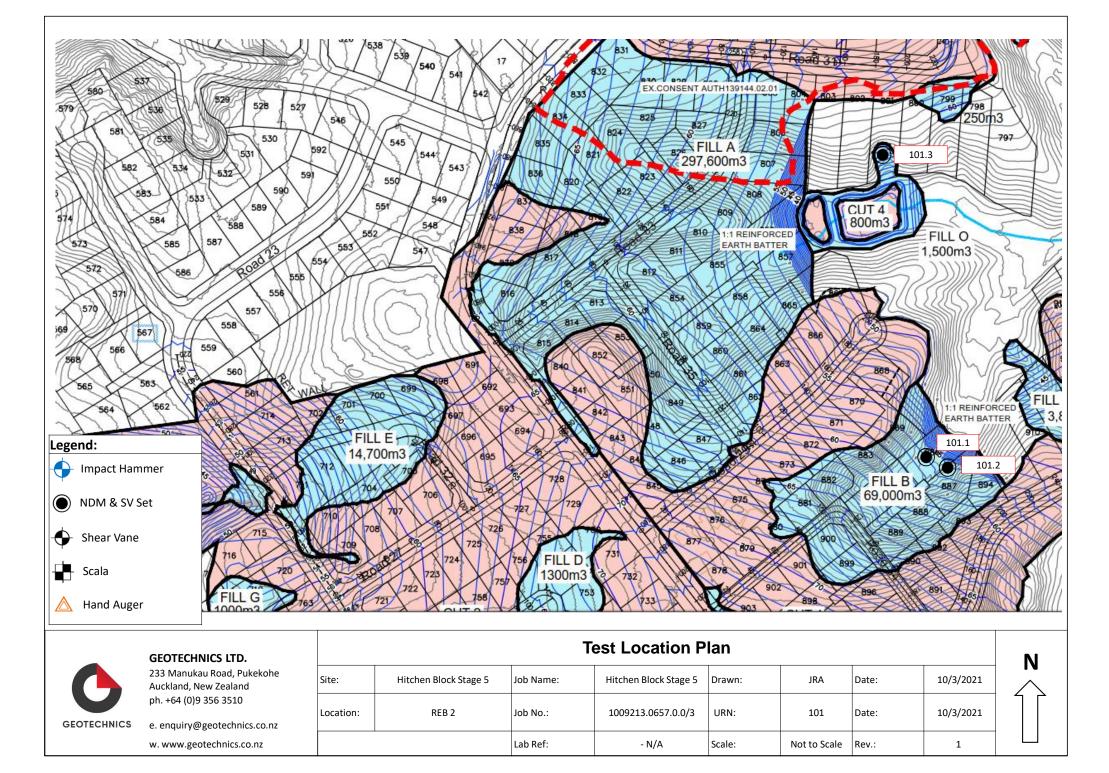


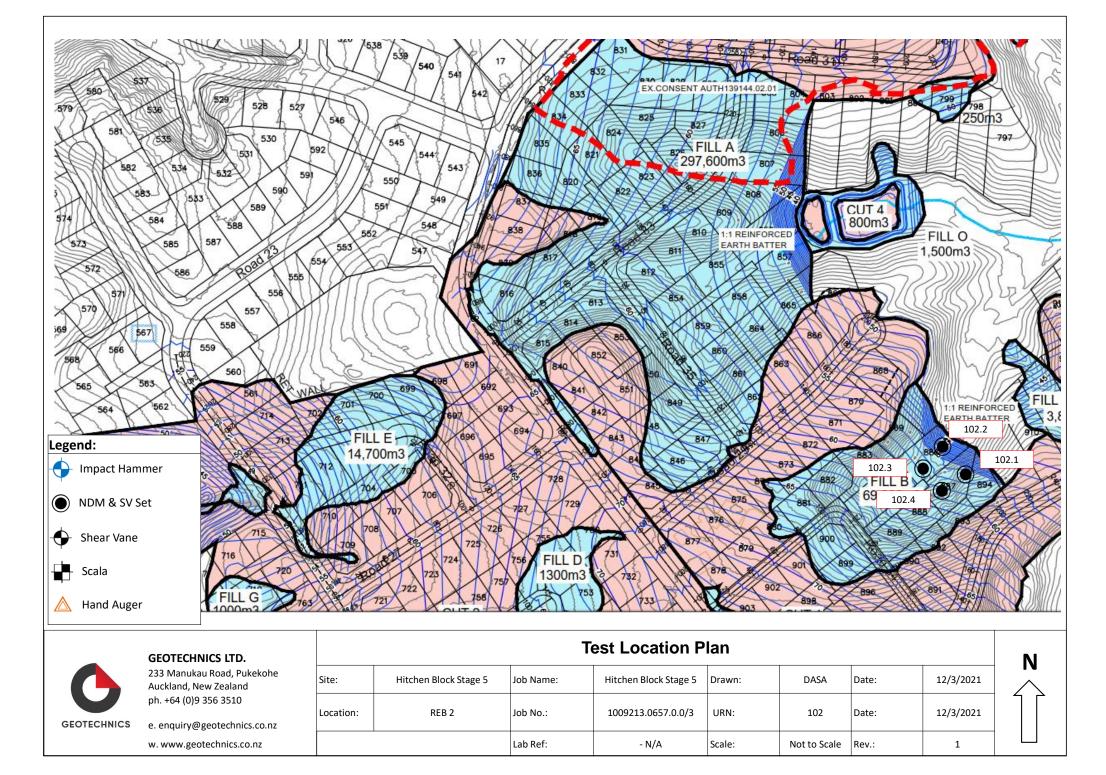


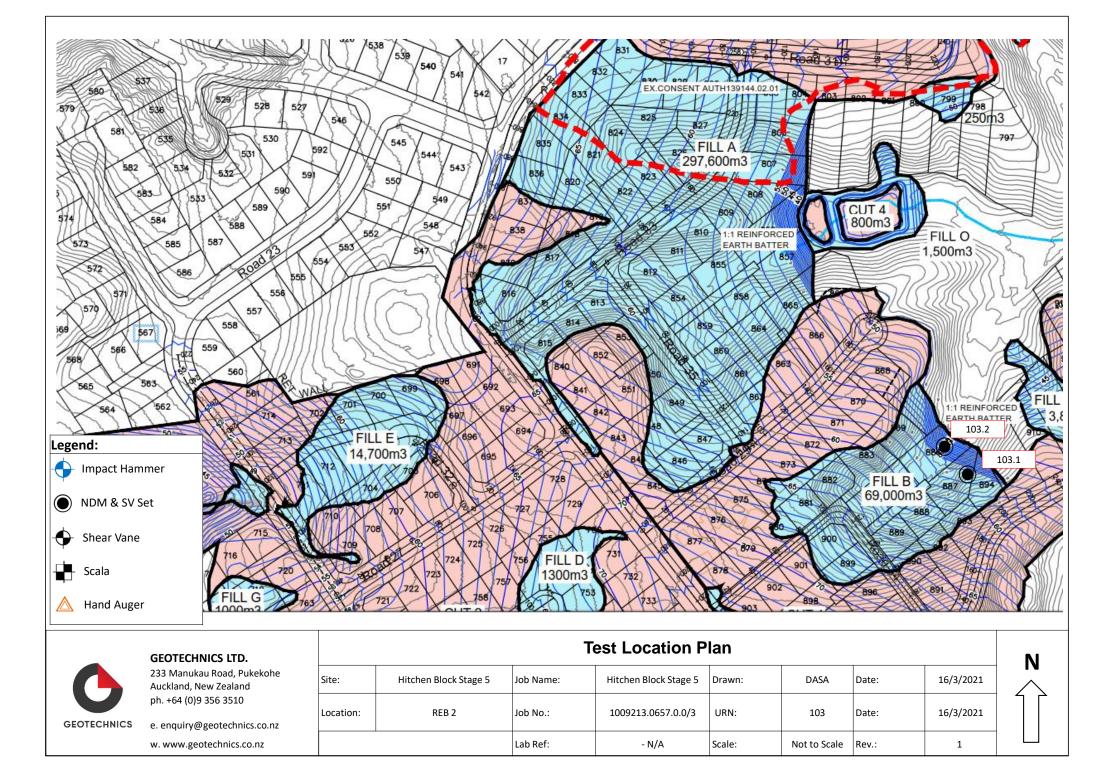


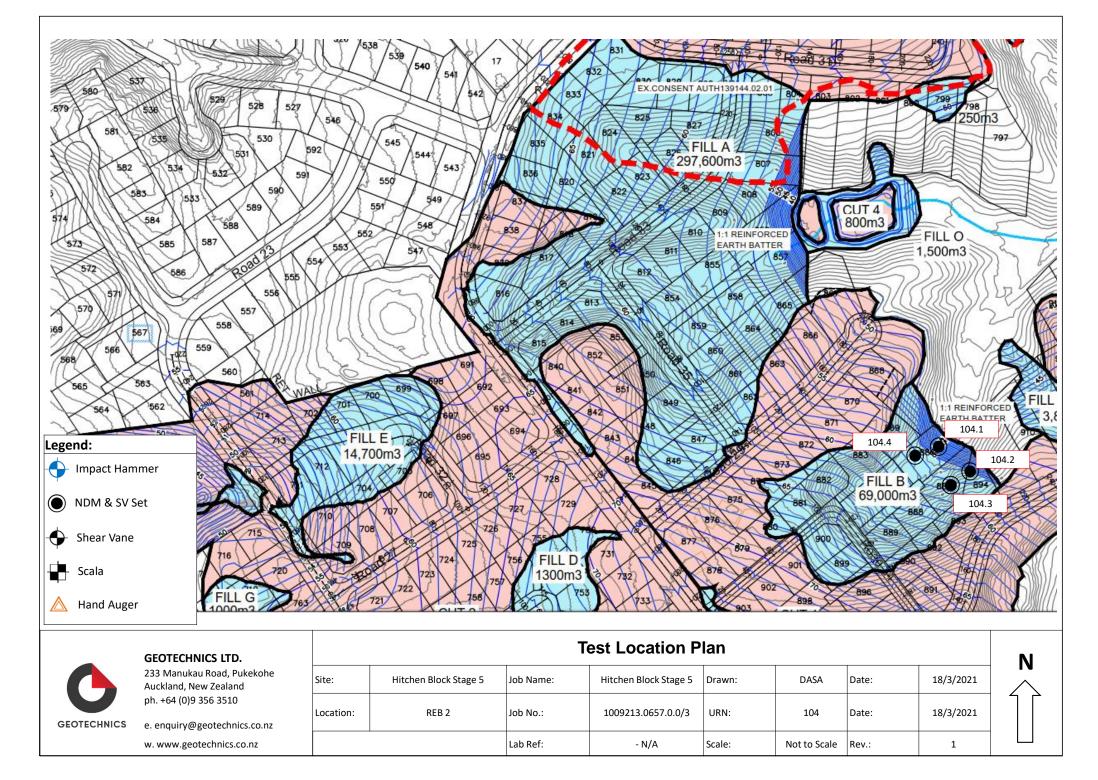


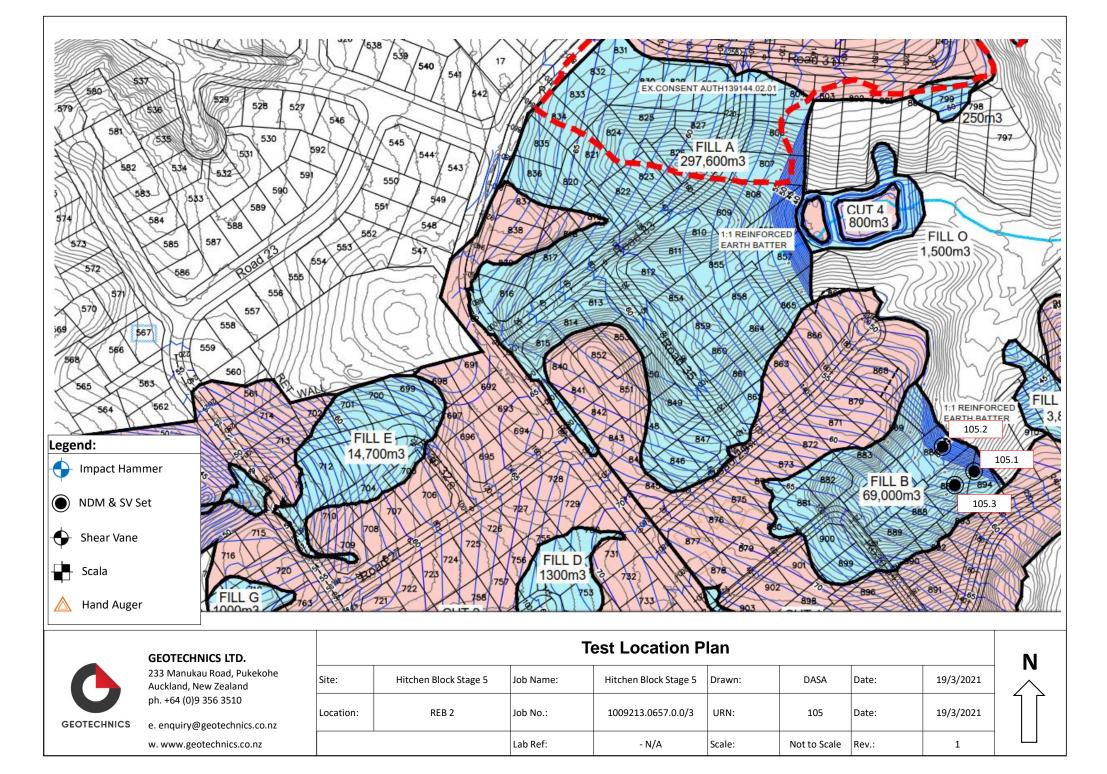


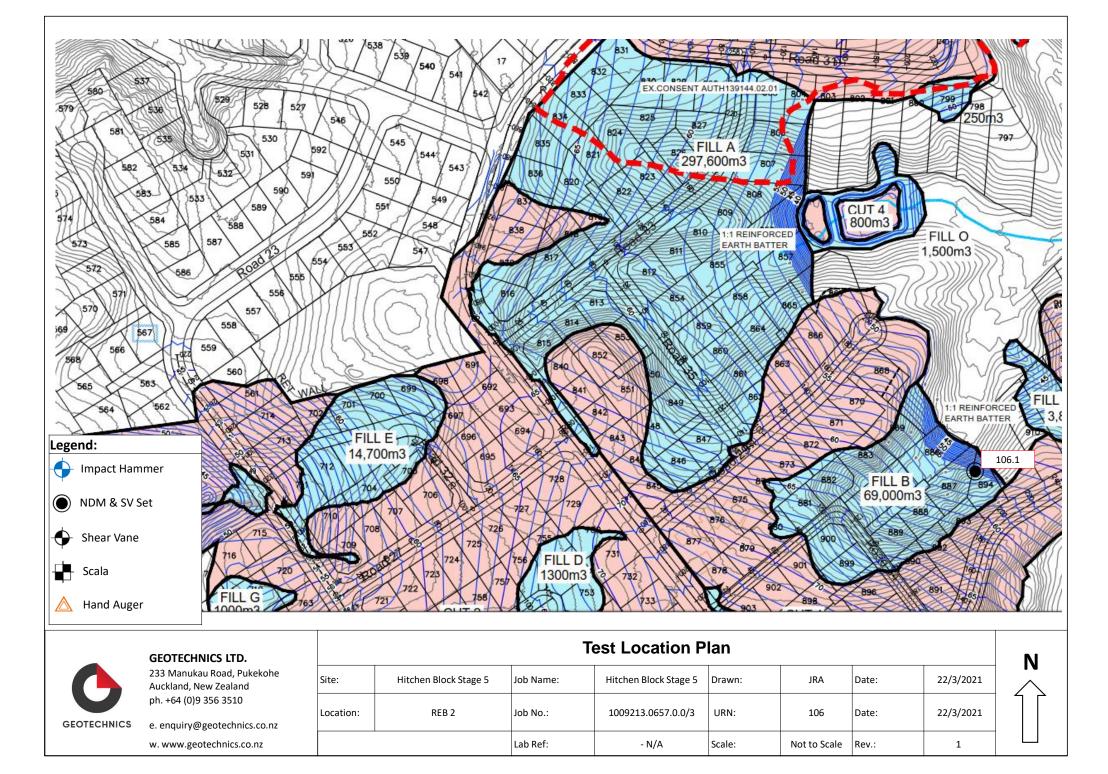


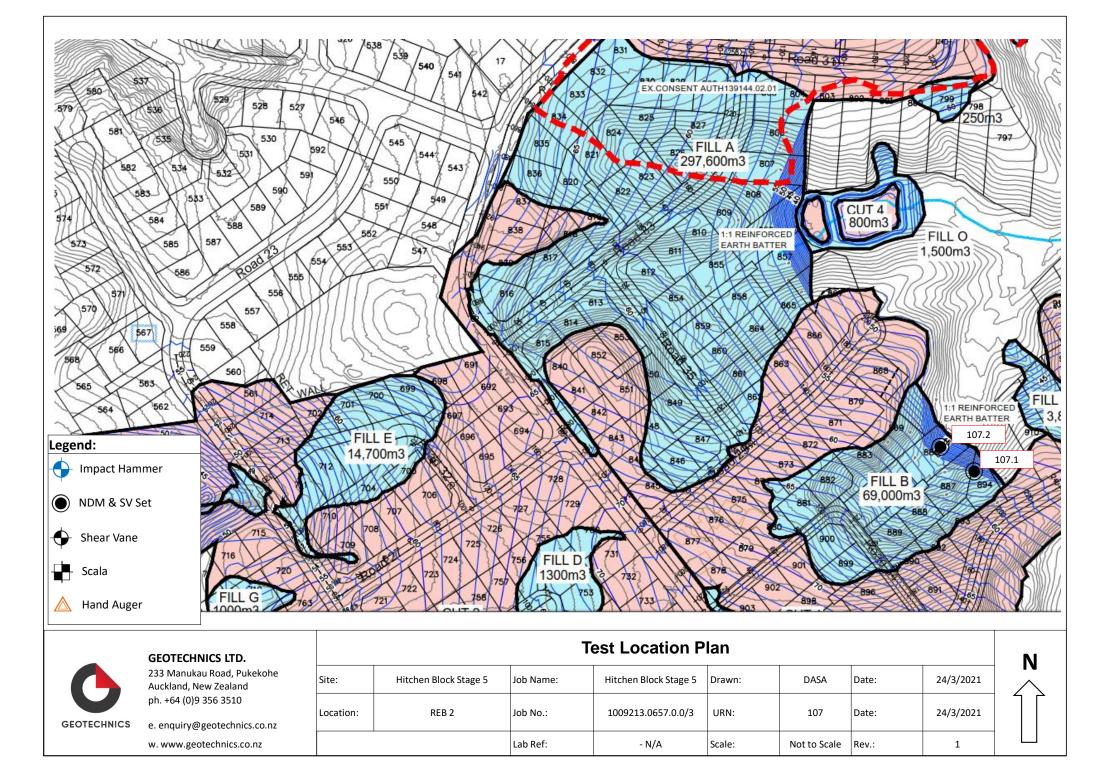


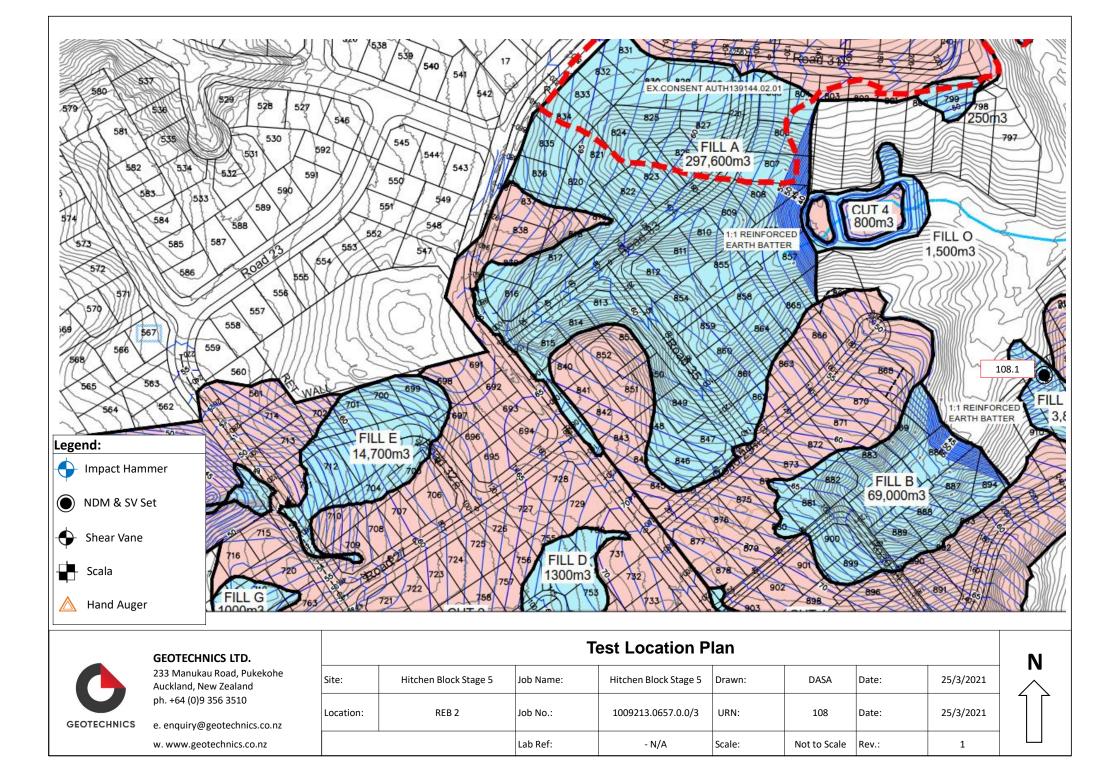


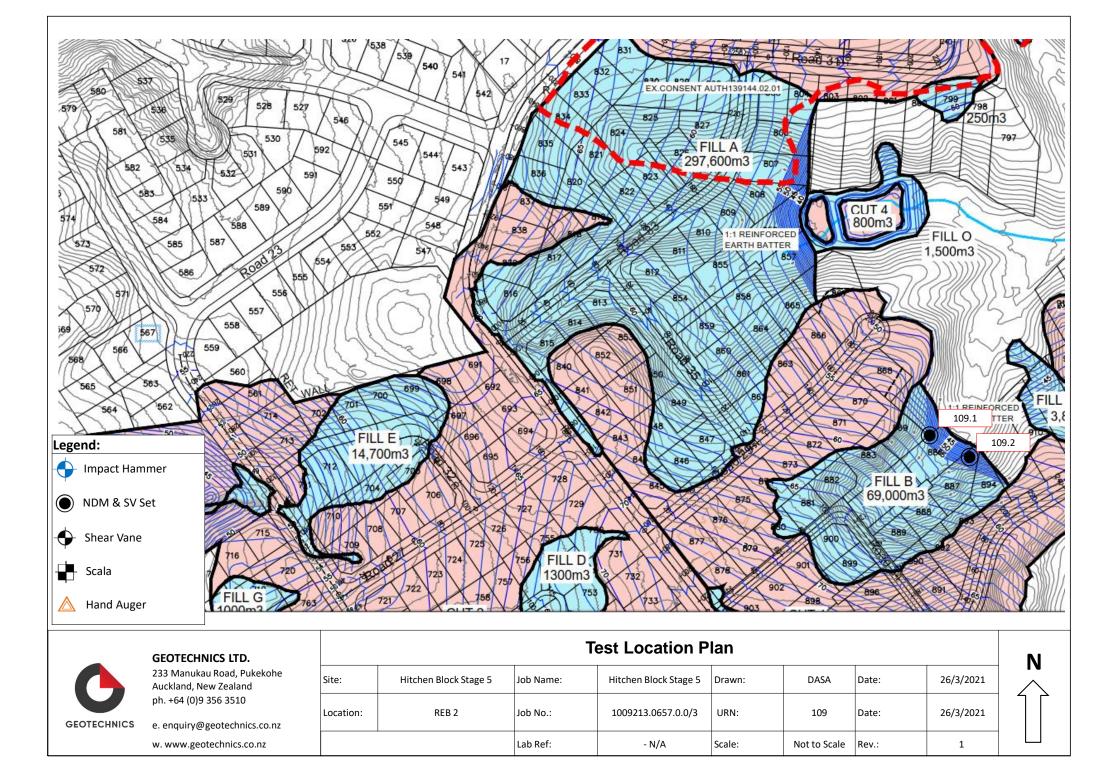


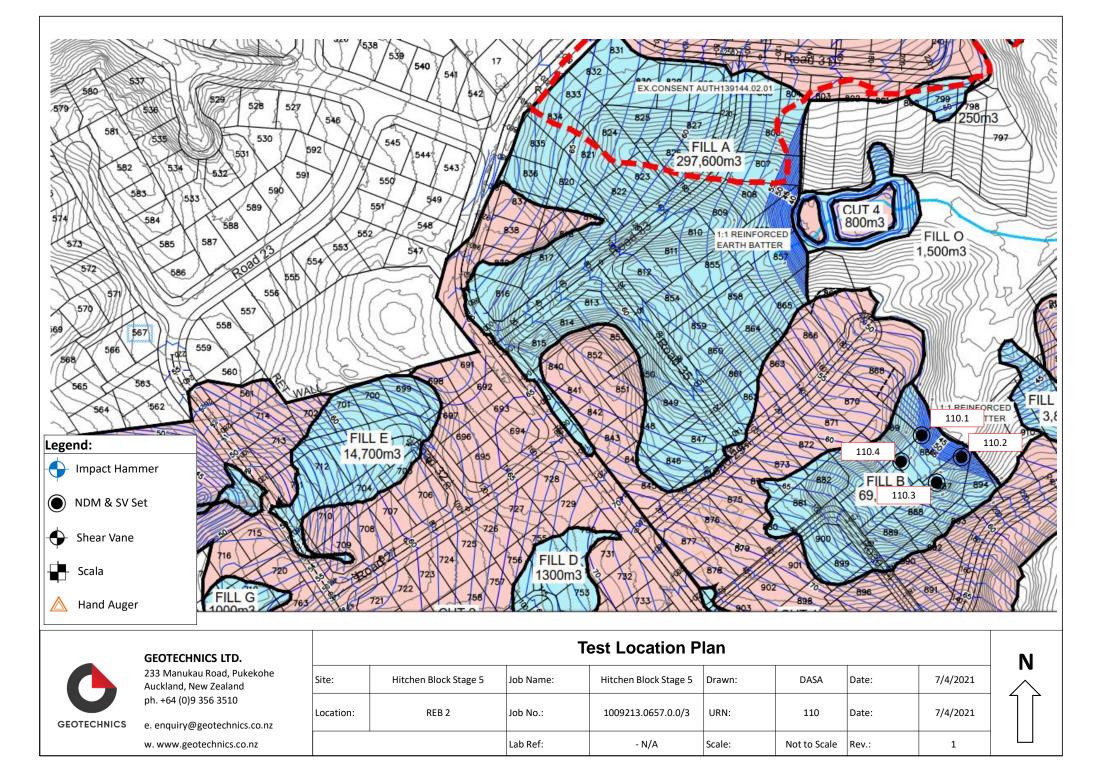


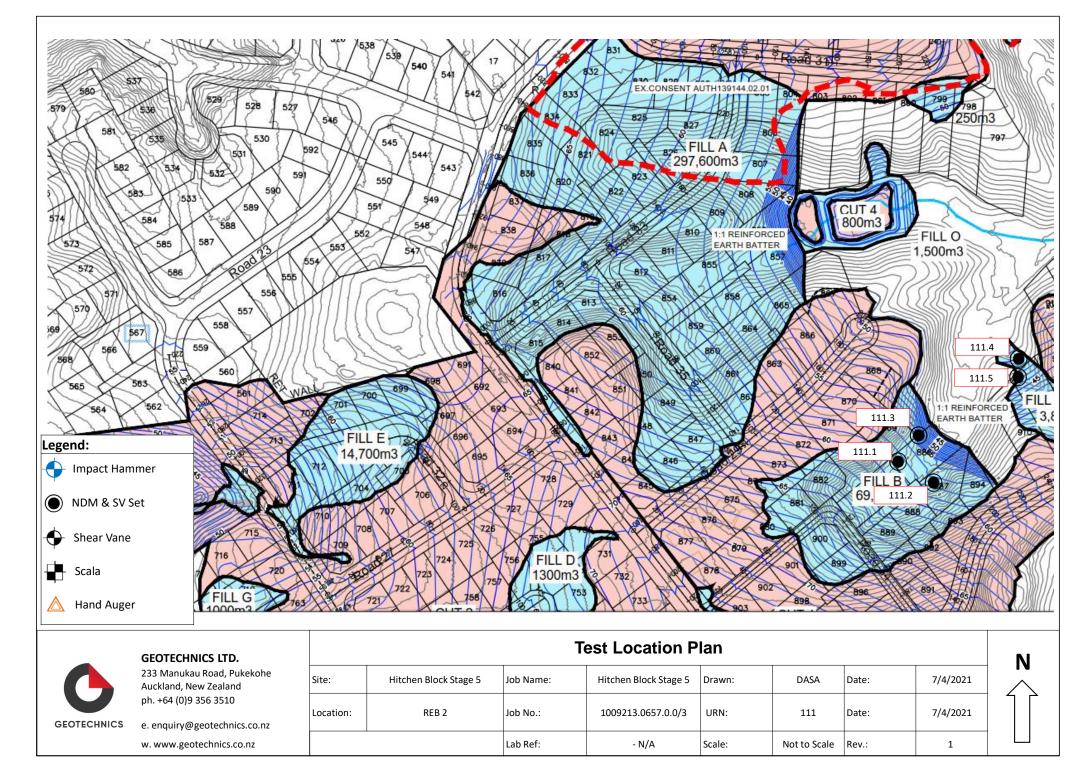


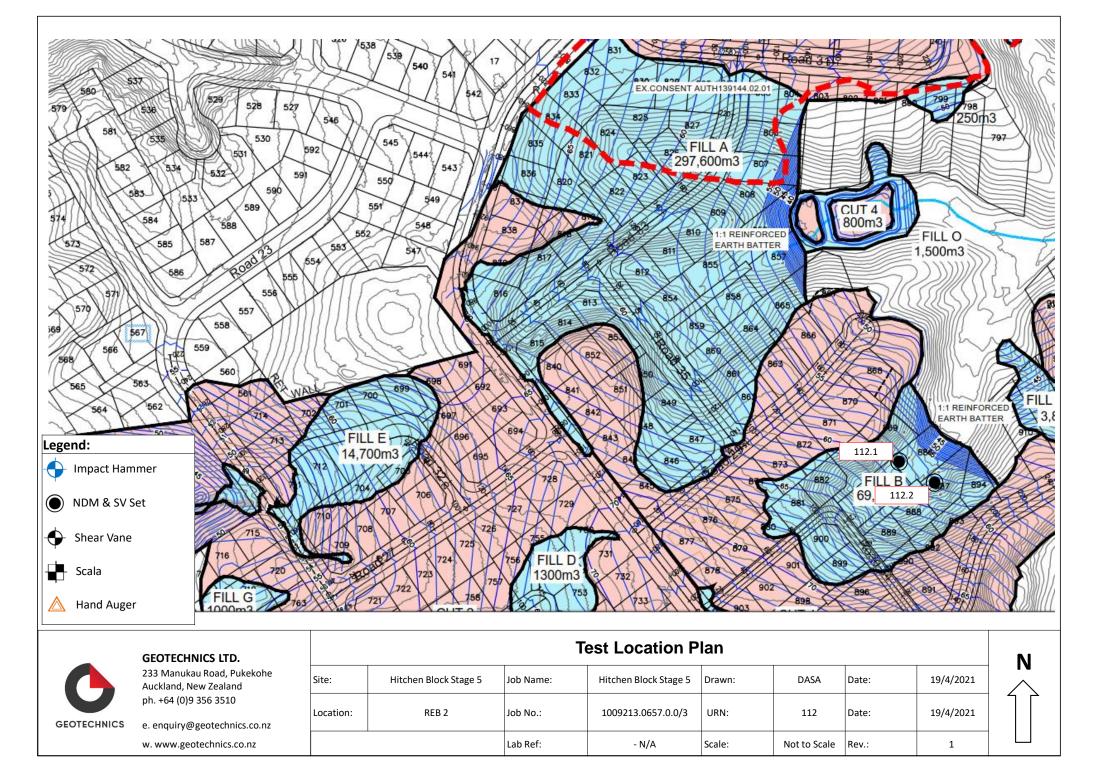


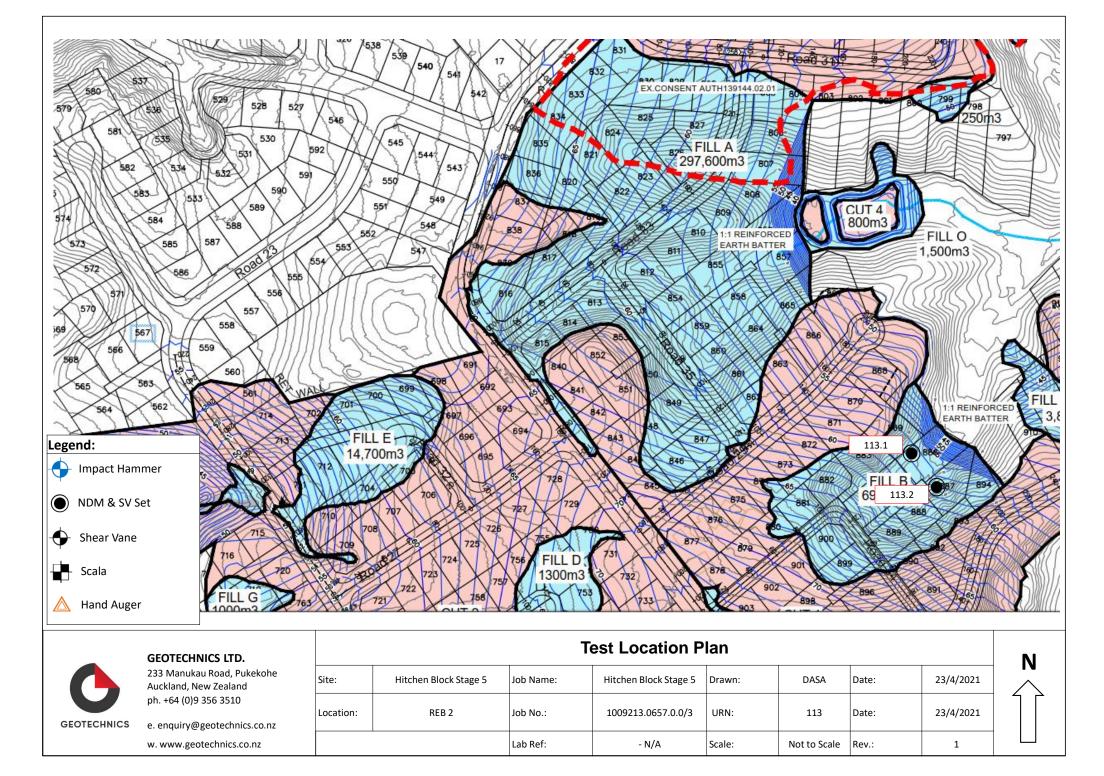


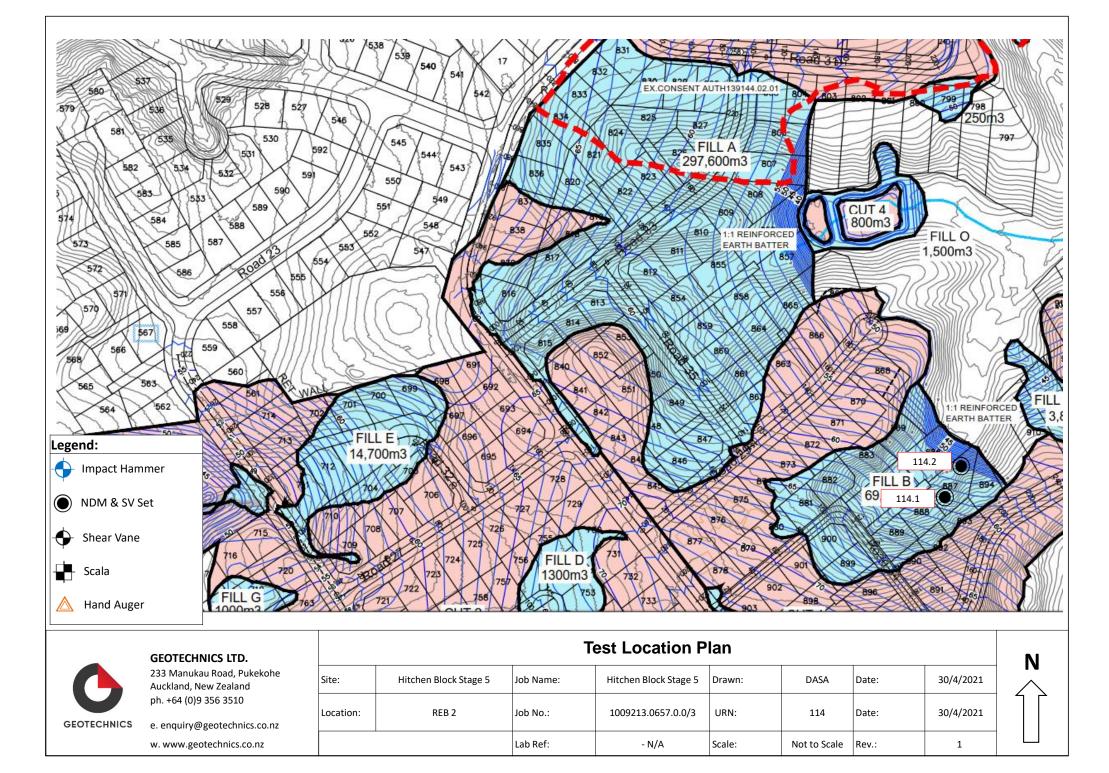


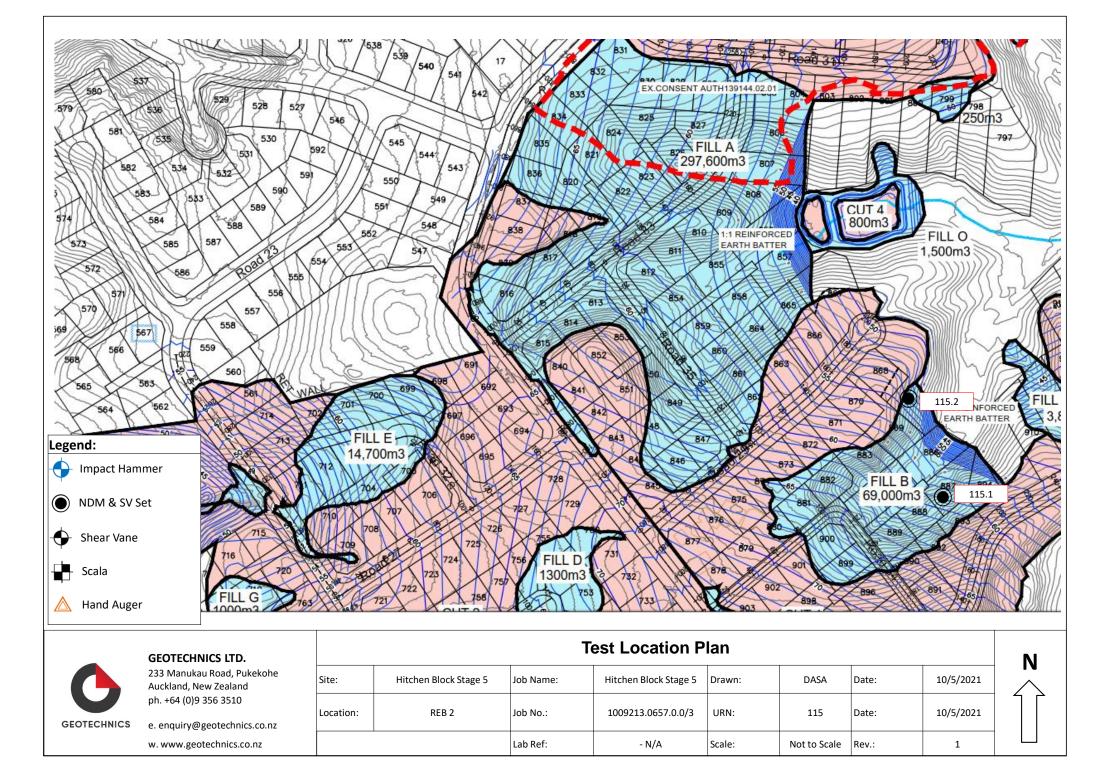














Appendix 3

Soil Classification Test Results



Our Ref: 1009479.1053.0.0/Rep1 Customer Ref: J00113 4 November 2021

Lander Geotechnical Ltd. PO Box 97385, Manukau 2241

Attention: Shane Lander

Dear Shane

Hitchen Stage 13B, Pokeno

Laboratory Test Report

The samples we collected from the above mentioned site have been tested according to your instructions and the results are included in this report. Results apply only to the sample(s) tested.

Descriptions are enclosed for your information, but are not covered under the IANZ endorsement of this report.

This report has been prepared for the benefit of Lander Geotechnical Ltd., with respect to the particular brief given to us and it cannot be relied upon in other contexts or for any other purpose without our prior review and agreement.

This report may be reproduced only in full.

Samples not destroyed during testing will be retained for one month from the date of this report before being discarded. If we can be of any further assistance, feel free to get in touch. Contact details are provided at the bottom of this page.

GEOTECHNICS LTD

Report prepared by:

Jack Singh Laboratory Technician Approved Signatory Authorised for Geotechnics by:

Vic O'Connor Project Director



All tests reported herein have been performed in accordance with the laboratory's scope of accreditation

Page 2 of 10

Report checked by:

λ .**.**....

Corey Papu-Gread Christchurch Manager

4-Nov-21 t:\geotechnicsgroup\projects\1009479\1009479.1053\workingmaterial\2021027.1009479.1053.0.0.rep1.docx



Christchurch 45A Parkhouse Road Wigram Christchurch 8042 New Zealand

p +64 3 361 0300

| GEOTECHNICS | | p · 04 0 001 0000 |
|---|---|--|
| Material Te | st Report | Report No: MAT:S21CH00069 |
| Customer: Lander Ge Address: Level 3, 3 Manukau, Project: Hitchen S Project No.: 1009479 Customer Reference Report Authorised B | eotechnical Osterley Way 2104 tage 13B Pokeno 0.1053.0.0 No.: J00113 | Approved By: Jack Singh (Laboratory Technician) Date of Issue: 3/11/2021 Please reproduce this report in full when transmitting to others or including in internal rep |
| Sample Details Location Geotechnics ID Sample Reference Sample Description Sample Depth Bottom Depth | Hitchen Stage 13B Pokeno S21CH000696 Lot 778 SILT with minor to some clay, trace sand and minor gravel, yellowish brown, mottled orange. Moist, very high plasticity. 0.5m 1.0m | |
| Test Results | | |
| Description Moisture Content [NZS Moisture Content (%) Date Tested | Method 4402:1986 Test 2.1] | Result Limits 34.4 2/11/2021 |
| Comments | | |
| This test result is IANZ acc | redited. | |

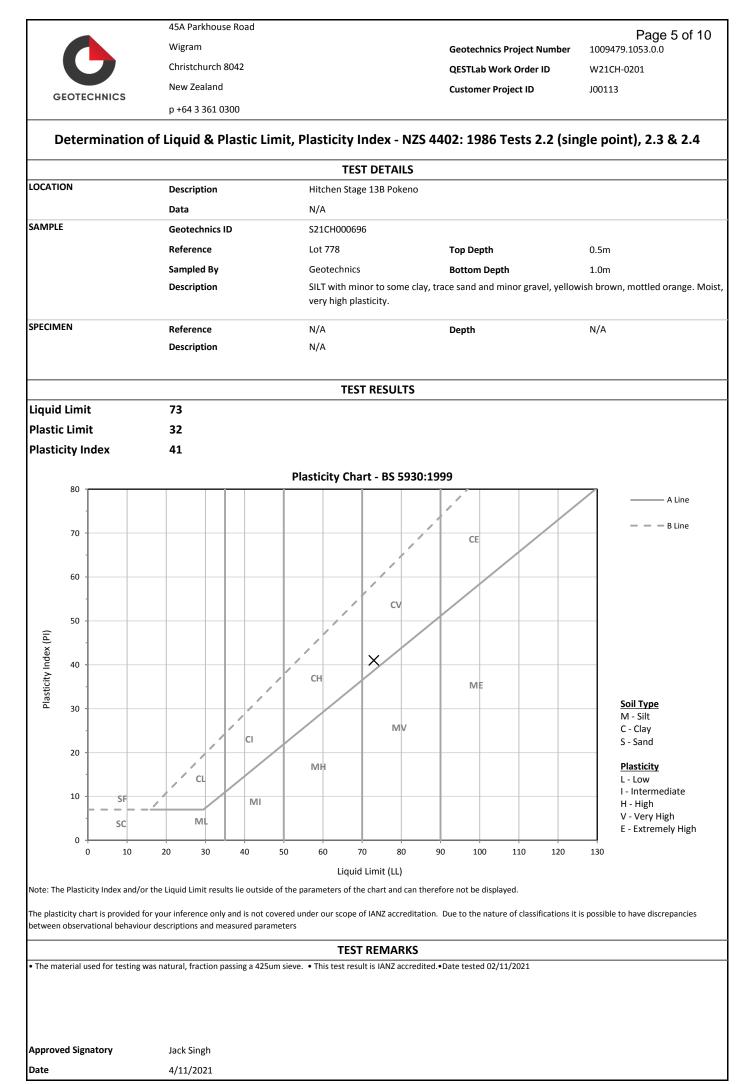
If samples have been taken, and were not destroyed during testing, they will be retained for one month from the date of this report before being discarded. Form No: 18909, Report No: MAT:S21CH000696 © 2000-2018 QESTLab by SpectraQEST.com Page

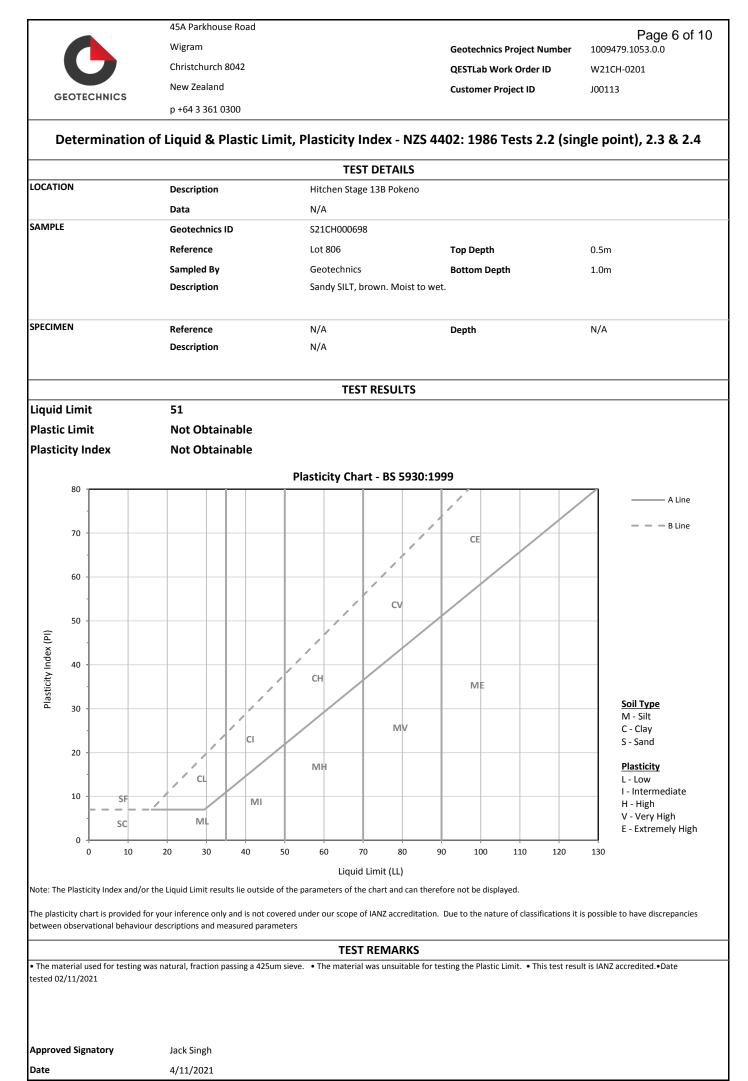


Christchurch 45A Parkhouse Road Wigram Christchurch 8042 New Zealand

| GEOTECHNICS | p +64 3 361 0300 |
|---|---|
| | Report No: MAT:S21CH000698 |
| | Issue No: 1 |
| Material Test Report | |
| Customer: Lander Geotechnical Address: Level 3, 3 Osterley Way Manukau, 2104 Project: Hitchen Stage 13B Pokeno Project No.: 1009479.1053.0.0 Customer Reference No.: J00113 Report Authorised By : Jack Singh Sample Details Location Location Hitchen Stage 13B Pokeno Geotechnics ID S21CH000698 Sample Reference Lot 806 Sample Description Sandy SILT, brown. Moist to wet. Sample Depth 0.5m Bottom Depth 1.0m | Approved By: Jack Singh (Laboratory Technician) Date of Issue: 3/11/2021 Please reproduce this report in full when transmitting to others or including in internal reports. |
| Description Method Moisture Content [NZS 4402:1986 Test 2.1] Moisture Content (%) Date Tested Date Tested | Result Limits 34.9 2/11/2021 |
| Comments | |
| Comments This test result is IANZ accredited. | |

If samples have been taken, and were not destroyed during testing, they will be retained for one month from the date of this report before being discarded. Form No: 18909, Report No: MAT:S21CH000698 © 2000-2018 QESTLab by SpectraQEST.com





| | 45A Parkhouse Road | | | Page 7 of 10 |
|--------------------------------------|----------------------------|--|---------------|----------------------------|
| \mathbf{C} | Wigram | Geotechnics Project | t Number | 1009479.1053.0.0 |
| | Christchurch 8042 | QESTLab Work Orde | er ID | W21CH-0201 |
| GEOTECHNICS | New Zealand | Customer Project IE |) | J00113 |
| | p +64 3 361 0300 | | | |
| | Determination | of the Linear Shrinkage - NZS 4402:1986 Test | t 2.6 | |
| 0017101 | | TEST DETAILS | | |
| OCATION | Description | Hitchen Stage 13B Pokeno | | |
| | Data | N/A | | |
| SAMPLE | Geotechnics ID | S21CH000696 | | |
| | Reference | Lot 778 Top Depth | | 0.5m |
| | Sampled By | Geotechnics Bottom Depth | | 1.0m |
| | Description | SILT with minor to some clay, trace sand and minor gravel, very high plasticity. | yellowish bro | wn, mottled orange. Moist, |
| PECIMEN | Reference | Depth | | |
| | Description | | | |
| | | | | |
| | | | | |
| | | | | |
| Linear Shrinkage | 16% | | | |
| | | | | |
| | | | | |
| | | TEST REMARKS | | |
| • This test result is IANZ accredite | ed.•Date tested 02/11/2021 | TEST REMARKS | | |
| This test result is IANZ accredite | ed.•Date tested 02/11/2021 | TEST REMARKS | | |
| • This test result is IANZ accredite | ed.•Date tested 02/11/2021 | TEST REMARKS | | |
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| • This test result is IANZ accredite | ed.•Date tested 02/11/2021 | TEST REMARKS | | |
| • This test result is IANZ accredite | ed.•Date tested 02/11/2021 | TEST REMARKS | | |
| • This test result is IANZ accredite | ed.•Date tested 02/11/2021 | TEST REMARKS | | |

| | 45A Parkhouse Road | | | Page 8 of 10 |
|---------------------------------------|---------------------------|----------------------------------|----------------------------|------------------|
| | Wigram | | Geotechnics Project Number | 1009479.1053.0.0 |
| | Christchurch 8042 | | QESTLab Work Order ID | W21CH-0201 |
| GEOTECHNICS | New Zealand | | Customer Project ID | J00113 |
| | p +64 3 361 0300 | | | |
| | Determination | of the Linear Shrinkage - NZS | 4402:1986 Test 2.6 | |
| OCATION | Description | TEST DETAILS | | |
| | Description | Hitchen Stage 13B Pokeno | | |
| SAMPLE | Data Geotechnics ID | N/A S21CH000698 | | |
| | Reference | Lot 806 | Top Depth | 0.5m |
| | Sampled By | Geotechnics | Bottom Depth | 1.0m |
| | Description | Sandy SILT, brown. Moist to wet. | Bottom Depth | 1.011 |
| | | | | |
| SPECIMEN | Reference Description | | Depth | |
| | P | | | |
| | | | | |
| Linear Shrinkage | 6% | | | |
| | | | | |
| | | TEST REMARKS | | |
| • This test result is IANZ accredited | d.●Date tested 02/11/2021 | | | |
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| Approved Signatory | Jack Singh | | | |
| Approved Signatory Date | Jack Singh 3/11/2021 | | | |

| | 45A Parkhouse Road | | | Page 9 of 10 | | |
|----------------|--|------------|--|--|--|--|
| GEOTECHNICS | Wigram Christchurch 8042 New Zealand p +64 3 361 0300 | | Geotechnics Project Number QESTLab Work Order ID Customer Project ID | 1009479.1053.0.0 W21CH-0201 J00113 | | |
| | Determination of the S | hrink - Sv | vell Index - AS 1289 Test 7.1.1 - 2003 | } | | |
| | | TES | T DETAILS | | | |
| LOCATION | Description | Hitchen S | tage 13B Pokeno | | | |
| | Data | N/A | | | | |
| SAMPLE | Geotechnics ID | S21CH000 | 695 BH No | Lot 766 | | |
| | Reference | Lot 766 | Top Depth | 0.5m | | |
| | Sampled By | Geotechn | ics Bottom Depth | 1.0m | | |
| | Description | SILT with | minor sand and trace clay, orangish brown. Moist | | | |
| SPECIMEN | Reference | | Depth | | | |
| | Description | | | | | |
| | | TEST | RESULTS | | | |
| | Applied Pressure | (kPa) | 25 | | | |
| | Initial Water Content | (%) | 17.6 | | | |
| | Bulk Density | (t/m³) | 1.83 | | | |
| SWELL TEST | Dry Density | (t/m³) | 1.56 | | | |
| | Final Water Content | (%) | 18.7 | | | |
| | Swelling Strain | (%) | -0.04 | | | |
| | Initial Water Content | (%) | 18.1 | | | |
| | Shrinkage Strain | (%) | 4.9 | | | |
| SHRINKAGE TEST | Inert Material Estimate in the Soil Specimen | (%) | None | | | |
| | Soil Crumbling During Shrinkage | | Minor | | | |
| | Cracking of the Shrinkage Specimen | | Minor | | | |
| | SHRINK - SWELL INDEX | (%) | 2.7 | | | |
| | | TES | REMARKS | | | |

• Estimates of inert material, soil cracking and soil crumbling are enclosed for your information, but are not covered under the IANZ endorsement of this report. • This test result is IANZ accredited. • Date tested 27/10/2021

| | | | | | Page 10 of 10 |
|----------------|---|------------------|--|------------|------------------|
| | 45A Parkhouse Road | | | | Ū |
| | Wigram | | Geotechnics Project | t Number | 1009479.1053.0.0 |
| | Christchurch 8042 | QESTLab Work Ord | er ID | W21CH-0201 | |
| GEOTECHNICS | New Zealand | | Customer Project IE |) | J00113 |
| GEOTECHNICS | p +64 3 361 0300 | | | | |
| | Determination of the S | Shrink - Sv | vell Index - AS 1289 Test 7.1 | .1 - 2003 | |
| | | TES | DETAILS | | |
| LOCATION | Description | Hitchen S | age 13B Pokeno | | |
| | Data | N/A | | | |
| SAMPLE | Geotechnics ID | S21CH000 | 697 BH No | | Lot 788 |
| | Reference | Lot 788 | Top Depth | | 0.5m |
| | Sampled By | Geotechn | | | 1.0m |
| | Description | SILT with | trace sand and trace clay, brownish re | d. Moist. | |
| SPECIMEN | Reference | | | | |
| | Description | | | | |
| | | TEST | RESULTS | | |
| | Applied Pressure | (kPa) | | 25 | |
| | Initial Water Content | (%) | | 31.6 | |
| | Bulk Density | (t/m³) | | 2.00 | |
| SWELL TEST | Dry Density | (t/m³) | 1.52 | | |
| | Final Water Content | (%) | 32.3 | | |
| | Swelling Strain | (%) | 0.16 | | |
| | Initial Water Content | (%) | | 31.8 | |
| | Shrinkage Strain | (%) | | 7.1 | |
| SHRINKAGE TEST | Inert Material Estimate in the Soil Specimen | (%) | | None | |

• Estimates of inert material, soil cracking and soil crumbling are enclosed for your information, but are not covered under the IANZ endorsement of this report. • This test result is IANZ accredited.• Date tested 27/10/2021

TEST REMARKS

(%)

Minor

Minor

4.0

Soil Crumbling During Shrinkage

Cracking of the Shrinkage Specimen

SHRINK - SWELL INDEX

3.2 Slab-on-ground in expansive soils

3.2.1 NZS 3604 Clause 1.1.2 Buildings covered by this Standard

Amend 1.1.2(a) to read:

"Buildings founded on good ground or on expansive soils where the requirements of 1.1.5 are met"

3.2.2 NZS 3604 New Clause Add new: "Clause 1.1.5 Buildings on expansive soils

Buildings on expansive soils shall be supported on slab-on-ground foundations complying with 7.5.13 and in addition to 1.1.2 shall be limited as follows:

- (a) single storey, stand-alone household unit, and
- (b)maximum length or width of floor of 24.0 m including any attached garage, and
- (c) simple plan shapes such as rectangular, L, T or boomerang, and
- (d) concrete slab-on-ground with a minimum thickness of 100 mm and a minimum concrete compressive strength of 20 MPa, and
- (e)simple roof forms, incorporating hips, valleys, gables or mono pitches, and
- (f) maximum overall height of 7.0 m to roof apex from lowest cleared ground level, and
- (g)maximum roof height of 3.0 m, and
- (h)roof slope between 10° and 35° from the horizontal, and
- (i) maximum span of roof truss 12.0 m, and
- (j) external walls maximum of 2.4 m height studs, other than gable end walls and walls to mono-pitched roofs, which shall not exceed 4.0 m.

COMMENT:

Floor plans

Where floor plans incorporate re-entrant corners then continuity of the exterior ground beam shall be maintained by continuing it as an internal beam, with the exterior beam details continued for a length of at least 1.0 m into the internal beam. This is only applicable where internal beams are specified in Tables 7.4A and 7.4B. This is aimed to bring the solution in NZS 3604 in line with Clause 5.3.8 of AS 2870:2011.

Ground movement

Provision for the additional ground movement effects from trees near to foundations in expansive soils should be considered. Trees remove moisture from the soil for a radius equal to the height of the tree. This causes expansive soils to shrink to varying degrees, and when near houses leads to differential settlement occurring under foundations. Movement of the foundations may lead to cracks in the building and door jamming.

Where existing trees (including trees that have been recently removed) are located closer to the foundations than 1.5 times the mature height of a tree, then additional geotechnical advice should be obtained. Planting of new trees should be avoided near foundations of new buildings or neighbouring buildings on sites with expansive soils.

3.2.3 NZS 3604 Clause 7.5.1

Add the following paragraph at the end of Clause 7.5.1:

"Slabs on expansive soils for buildings meeting the requirements of 1.1.5 shall, in addition to meeting the requirements of 7.5.1 to 7.5.12, meet the requirements of 7.5.13. Where there is conflict the requirements of 7.5.13 shall apply."

3.2.4 NZS 3604 New clause, tables and figures

Add new: Clause 7.5.13 Slab-on-ground in expansive soils

7.5.13.1 Identification of expansive soils 7.5.13.1.1 Should reasonable enquiry as outlined in 3.1.3 show any signs of expansive soils, the expansive soil class, as defined in AS 2870, shall be established by one or all of:

(a) enquiry to the local territorial authority, and/or

- (b) reference to the certificate of suitability issued in terms of NZS 4431, and/or
- (c) a soil test undertaken by a suitably qualified soils engineer.

7.5.13.1.2 Expansive soil class shall be defined as:

- (a) Slightly 'S', having an I_{SS} range of 0–1.9%, and a 500 year design characteristic surface movement return (y_S) of 22 mm, or
- (b)Moderately 'M', having an I_{SS} range of 2.0–3.7% and a 500 year design characteristic surface movement return (y_S) of 44 mm, or

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- (c) Highly 'H', having an I_{SS} range of 3.8–6.5% and a 500 year design characteristic surface movement return (y_S) of 78 mm, or
- (d) Extremely 'E', having an I_{SS} range of 6.6–7.5% and a 500 year design characteristic surface movement return (y_S) of 90 mm.

7.5.13.2 Maximum aspect ratio of concrete slabs

The aspect ratio of the concrete slabs or bays of concrete slabs, such as in the case of L, T or boomerang concrete slab shapes, shall not exceed 5 to 1 (length to width).

7.5.13.3 Foundation details

7.5.13.3.1 For the identified expansive soil class the foundation details, external and internal thickenings shall be as follows.

- (a) For light wall claddings refer to Table 7.4A and Figure 7.22.
- (b)For medium wall or heavy wall claddings refer to Table 7.4B and Figure 7.23.

7.5.13.3.2 Situations where no internal thickenings shall be required are limited to a rectangular slab with long side not exceeding 17.0 m. Where this limit is exceeded, add additional internal thickenings across the slab with the same cross section dimensions and reinforcing as the external footing, so that the centre to centre spacing of thickenings is always less than 17.0 m.

COMMENT:

Design constraints:

- a) The characteristic surface movements and the corresponding expansivity classifications have been calculated based on design for ultimate limit state (ULS) conditions for a 1 in 1000 year "extreme" drought event, and the serviceability limit state (SLS) conditions for a 1 in 500 year drought event.
- b) Maximum soil movements are calculated to be based on a 500 year return period for SLS, and a 1000 year return period for ULS*;

(*NB: This differed from the recommendations contained within BRANZ Study Report 120A (BSR120A) which used a 300 year return period for the design level drought conditions)

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- c) Climate parameters adopted from BSR120A of $\Delta u = 1.2$ pF, Hs = 1.5 m, and a crack depth of 0.5 Hs
- d) The I_{SS} (soil stability index) ranges attributed to the expansivity classifications as defined in 3.2.4 above have been calculated using the parameters presented in BSR120A and Equation 2.3.1 of AS 2870:2011.
- e) Sites subject to parameters that differ from those mentioned above, in particular sites where the crack depth is less than 0.75 m, such as cut natural ground or clay backfill, require specific engineering assessment to confirm their appropriate site classification.
- f) The effects of nearby trees (whether existing, recently removed, or future planting) are not considered in these solutions. It is recommended that specific geotechnical engineering advice is obtained where a tree is within a lateral distance of 1.5 times its mature height of the foundations.

Maintenance of foundations in expansive soils

Normal maintenance is that work generally recognised as necessary to achieve the expected performance over time of the foundation located on expansive soils. Unless otherwise specified by the designer and noted on the drawings, basic normal maintenance tasks should ensure that:

- a) the drainage and wetting of the site is controlled so that extremes of wetting and drying of the soils are prevented, and
- b) the position and operation of gardens adjacent to the dwelling are controlled, and the planting of trees near to foundations is suitably restricted, and
- c) any leaks which develop in plumbing, storm water or sanitary sewage systems are repaired promptly.

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| Table 7.4A Reinforced concrete foundations in expansive soils for light wall claddings Clause 7.5.13 and Figure 7.22 | | | | | | |
|--|---------------------------|---------------------------|--------------|---------------|--|--|
| Expansive soil class | Slightly 'S' | Moderately 'M' | Highly 'H' | Extremely 'E' | | |
| Soil embedment (De) | 375 mm | 525 mm | 575 mm | 625 mm | | |
| Top steel (A _s top) | 2/D 16 | 2/ D16 | 2/D16 | 2/ D16 | | |
| Bottom steel (A _s bottom) | 1/ D16 | 1/ D25 | 1/ D20 | 1/ D25 | | |
| Stirrups | R6/ 125 crs. | R6/ 125 crs. | R6/300 crs. | R6/ 300 crs. | | |
| Maximum spacing of internal thickenings | no internal thickening | no internal thickening | 2.5 m crs. | 2.5 m crs. | | |
| Depth of thickening (D1) | - | - | 400 mm | 450 mm | | |
| Base width (B1) | - | - | 300 mm | 350 mm | | |
| Top steel (A _s top) | - | - | 2/ D20 | 2/ D20 | | |
| Bottom steel (A _s bottom) | - | - | 2/ D16 | 2/ D20 | | |
| Stirrups | - | - | R6/ 150 crs. | R6/ 150 crs. | | |

| Table 7.4BReinforced concrete foundations in expansive soils for medium wall and heavy wall claddingsClause 7.5.13 and Figure 7.23 | | | | | |
|--|--------------|----------------|--------------|---------------|--|
| Expansive soil class | Slightly 'S' | Moderately 'M' | Highly 'H' | Extremely 'E' | |
| Soil embedment (De) | 500 mm | 550 mm | 775 mm | 800 mm | |
| Top steel (A _s top) | 2/ D16 | 2/ D20 | 2/ D20 | 3/ D20 | |
| Bottom steel (A _s bottom) | 2/ D16 | 2/ D16 | 2/ D20 | 2/ D20 | |
| Stirrups | R6/ 125 crs. | R6/ 250 crs. | R6/ 300 crs. | R6/ 300 crs. | |
| Maximum spacing of internal thickenings | - | 2.5 m crs. | 2.5 m crs. | 2.5 m crs. | |
| Depth of thickening (D1) | - | 350 mm | 450 mm | 500 mm | |
| Base width (B1) | - | 300 mm | 300 mm | 350 mm | |
| Top steel (A _s top) | - | 2/ D16 | 3/ D20 | 3/ D20 | |
| Bottom steel (A _s bottom) | - | 2/ D16 | 2/ D16 | 2/ D20 | |
| Stirrups | - | R6/ 125 crs. | R6/ 150 crs. | R6/ 150 crs. | |

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Appendix 4

Post-Construction Borehole Records

| Client : | | | | | Aug | er Bo | oreho | | S. Lot 753 | |
|--|--|---------------------|---------------|---|------------------|-------------------------|---|---------------------|----------------------|---|
| Project Locatio | n: HITCHEN BLOCK STA | GE 13D, POREI | NO | Vane H | lead. | Logge | d By: | Process | | of 13 |
| Job Number: | J00113 | | | 1750 | | | u by. PL | PL | | .10.21 |
| Borehole mN Location: Description: | | round R.L. | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak/ residual} | Soil Sensitivity | Sample Laboratory | |
| | SOIL DESCRIPTION | | | Leç | Dep | Sta Wate | V Shea ^{peak/} | Sens | Tes Deta | |
| | | | | | | | | | | |
| Lot 753 | | | | | | | | | | |
| TOPSOIL | | | | | - 0.0 | | | | | |
| silty CLAY, orange and d | ark grey streaked brown. Hard, r | noist, medium plas | ticity, with | | | | | | | |
| trace fine gravel [FILL] |] æd orange/brown. Hard, moist, m | edium plasticity [A | SH1 | ×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×= | - | | 070 | | | |
| becoming orange streake | - | outant plaotiony [/ | | ×=×=×=×=×=×=× ×=×=×=×=×=×=×=× ×=×=×=×=× | - 0.5 | | 270+ | | | |
| - | | | | × × × × × × × × × × × × × × × × × × × | È | | | | | |
| | ge streaked light grey/orange. Me | dium dense, mois | t, no | ******** | - | | 070 | | | |
| plasticity becoming black, dark red | and orange mottled grey | | | | - 1.0 | | 270+ | | | |
| - | | | | | F | | | | | |
| becoming sensitive | | | | | - | | 054/60 | 4.1 | | |
| - becoming sensitive | | | | | - 1.5 | | 254/62 | 4.1 | | |
| - | | | | | F | | | | | |
| - | | | | | - | | 254/42 | 6.0 | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | | - 2.0 | | 201/12 | 0.0 | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| Lot 755 | | | | | - 0.0 | | | | | |
| TOPSOIL | | | | | | | | | | |
| clayey SILT, light grey, or moderately sensitive [ASI | ange, white and dark grey mottle | d. Hard, moist, lov | v plasticity, | | F | | | | | |
| | , | | | | - | | 250/116 | 2.2 | | |
| silty CLAY, orange streak | ed orange/light grey. Hard, moist | , medium plasticity | , moderat | ely | | | | | | |
| _ sensitive | | | | ×-×-×-×-×-×-× ×-×-×-×-×-×-×-× ×-×-×-×-× | - | | | | | |
| - | | | | ×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×= | -1.0 | | 208/104 | 2.0 | | |
| becoming light grey and b | prown streaked orange | | | x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x= | È | | | | | |
| | - | | | x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x x= | E | | | | | |
| becoming insensitive | | | | ×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×=×=× | -1.5 | | 212/143 | 1.5 | | |
| becoming dark grey and one | orange mottled brown | | | ×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×= | | | | | | |
| - becoming light grey and b | prown/orange streaked orange | | | ×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×= | F | | | | | |
| | | | | x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x=x=x=x | -2.0 | | 247/158 | 1.6 | | |
| E.O.B. at 2.0m. Tar | yer Depin. | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | L | | | | •1 | +++++++ |
| | Comments: | Borehole Diameter: | Topsoil | - {<<< | and | <u></u> | Sandstone | 222222 | Plutonic | +++++++++++++++++++++++++++++++++++++++ |
| | Groundwater not encountered. (unless noted) | 50mm Checked: | Fill Clay | | ravel ganic | ****** | Siltstone | | No Core | |
| geotechnical | UTP = unable to penetrate. EOB = end of borehole. | RZ | k) | ****** | imice | ****** | Limestone Volcanic | | | |

| Client : | DFH JOINT VENTURE I | | | | Aug | er Bo | oreho | | S. Lot 75 | |
|---|--|-----------------------|-------------|--|---------------------------|-------------------------|--|---|----------------------|-----------|
| Project Locatio | n: HITCHEN BLOCK STAC | GE 13B, POKE | NO | | | | | | Sheet 2 | |
| Job Number: | J00113 | | | Vane H 1750 | | Logged P | d By: PL | Process PL | | .10.21 |
| Borehole MN Location: Description: | | ound R.L. | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak} / residual | Soil Sensitivity | Sample Laboratory | / / Other |
| | SOIL DESCRIPTION | | | Le | De | Sta Wat | She / | Ser | Tes Deta | |
| slightly clayey SILT, oran sensitive | rown and orange mottled. Hard, m ge mottled grey/brown. Medium de ige and brown mottled dark brown | | | | - 0.0 | | 270+ 212/39 270+ 212/42 | 5.5 | | |
| Lot 759 TOPSOIL clayey SILT, orange, light | grey and grey mottled. Hard, mois | st, low plasticity [/ | ASH] | | - 0.0 - - - - | | 270+ | | | |
| silty CLAY, light grey and | pink streaked orange. Hard, mois | t, medium plastic | ity | | - 0.5 - - | | 270+ | | | |
| moderately sensitive | d orange mottled grey/brown. Very rey and orange streaked light grey | | plasticity, | | - - - - | | 196/62 | 3.2 | | |
| becoming hard, sensitive - | | | | | - | | 266/54 | 4.9 | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | | -2.0 | | 270+ | | | |
| | Comments: | Borehole Diameter: | Topsoil | s: | and | | Sandstone | | Plutonic | ******** |
| | Groundwater not encountered. (unless noted) | 50mm | Fill | ///// Gi | ravel | | Siltstone | 2 | No Core | |
| | UTP = unable to penetrate. | Checked: | Clay - | Or | ganic 🔒 | ****** | Limestone | | | |
| georeennical | EOB = end of borehole. | RZ | Silt | ************************************** | ımice | | Volcanic | | ž | |

| Client : | | | | | Aug | er Bo | oreho | | S. Lot 76 | |
|---|--|----------------------------|--------------|--|---------------|-------------------------|---|---------------------|---------------------|-----------------|
| Project Locatio | n: HITCHEN BLOCK ST | AGE 13B, POKE | NO | | | ı. | - 6 | | | of 13 |
| Job Number: | J00113 | | | Vane H 30 | | Logge F | a By: RZ | Process PL | | : 5.10.21 |
| Borehole mN | | around R.L. | | p | (E | Standing Water Level | e ∢Pa) idual | vity | Sample | e and |
| Location: Description | Refer to site plan | | | Legend | Depth (m) | stand ater L | Vane Shear(kPa) _{peak} / _{residual} | Soil Sensitivity | Laborator Te | y / Other st |
| | SOIL DESCRIPTION | | | | | ° Š | S, a | Š | Deta | ails |
| | | | | | | | | | | |
| | | | | | | | | | | |
| Lot 761 TOPSOIL | | | | | - 0.0 | | | | | |
| | aked brown/orange. Hard, moist | modium plaatiaitu | | | F | | | | | |
| Silly CLAY, light grey stre | aked brown/orange. Hard, moisi | , mealum plasticity | [ASH] | ×=×=×=×=×=×=×= ×=×=×=×=×=×=×=×= ×= | ~ ~ | | | | | |
| - | | | | ×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=×=× | -0.5 | | 201+ | | | |
| - | | | | ×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=× | × × | | | | | |
| - | | | | ×=×=×=×=×=×=×= ×=×=×=×=×=×=×=×= ×= | - | | | | | |
| - | | | | x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x= | - 1.0 | | 201+ | | | |
| - | | | | ×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=× | | | | | | |
| - | | | | ×=×=×=×=×=×=×= ×=×=×=×=×=×=×=×= ×= | × | | | | | |
| - | | | | x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x= | - 1.5 | | 201+ | | | |
| - | | | | ×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=× | | | | | | |
| at 2.0m, becoming very s E.O.B. at 2.0m. Tar | | | | ×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=× | - - 2.0 | | 164/141 | 12 | | |
| Lot 763 | | | | | | | | | | |
| TOPSOIL | | | | | - 0.0 | | | | | |
| | grey mottled light brown. Very si | iff, moist, low plast | icity, extr | a 🚧 | | | | | | |
| _ sensitive [FILL] | | | | | <u>}</u> | | 1 41 /1 7 | 0.0 | | |
| - | | | | | -0.5 | | 141/17 | 8.2 | | |
| - | | | | | - | | | | | |
| becoming insensitive | | | | | - - | | 150/83 | 1.8 | | |
| - | | | | | 1.0 | | 100/00 | 1.0 | | |
| - | | | | | } | | | | | |
| becoming hard | | | | | -1.5 | | 201+ | | | |
| - | | | | | 1 | | | | | |
| | | | | | - | | | | | |
| | ant Dopth | | | | -2.0 | | 201+ | | | |
| E.O.B. at 2.0m. Tar | ует рерш. | | | | | | | | | |
| | | | | | | | | | | |
| | l | | r – | | | | 1 | | | ++++++++ |
| | Comments: Groundwater not encountered. | Borehole Diameter: 50mm | Topsoil | | and iravel | | Sandstone Siltstone | | Plutonic No Core | ******* |
| LANDER | (unless noted) | Checked: | Fill Clay | | ravel | ****** | Limestone | | | |
| geotechnical | UTP = unable to penetrate. EOB = end of borehole. | RZ | Silt | **** | umice | | Volcanic | | | |

| Client : Project | Locatio | | FH JOINT VENTU ITCHEN BLOCK S | | | NO | | | Aug | er Bo | orehol | | | 66 & 768 |
|--|---|--|---|----------|----------------------------|-----------------|----------------------|--------------|---|-------------------------|--|---------------------|------------------|------------------------|
| Job Nu | | | 00113 | | | | | Vane I 30 | | Logge F | d By: RZ | Process PL | or: Dat | |
| Borehole Location: | mN Description | : | mE Refer to site plan | Gro | ound R.L. | | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak} residual | Soil Sensitivity | Samp Laborato | ole and ory / Other |
| | | SO | IL DESCRIPTIO | N | | | | Ĺe | Det | Sta Wate | She: Peak | Sen | | est tails |
| plasticity [FI silty CLAY, becoming lig becoming ve at 2.0m, bec | LL] black mottled ght grey and n | orange. | ange and red mottled Hard, moist, high pla | d bro | | medium | | | - 0.0 - 0.5 - 1.0 - 1.5 | | 201+ 201+ 147/135 201+ | | | |
| Lot 768 TOPSOIL clayey SILT low plasticity | with trace fin y [FILL] ery stiff, sens , light brown. | ie sand, c itive Very stiff get Dep | orange and light grey | | | v sensitiv | | | - 0.0 - 0.5 - 1.0 - 1.5 - 2.0 | | 201+ 141/26 190/58 201+ | 5.4 | Plutonic | |
| | | Comme Groundy | ents: water not encountere | ed. | Borehole Diameter: 50mm | Topsoil Fill | | ₩- | and : | | Sandstone Siltstone | | Plutonic | ******* |
| | DER | (unless | noted) | <i>.</i> | Checked: | Clay | //// | | rganic | ****** | Limestone | | | |
| geotech | nical | | nable to penetrate. and of borehole. | | RZ | Silt | ×××× ×××× ×××× | < X X X | umice | | Volcanic | | Ž. | |

| Client : Project Locatio | DFH JOINT VENTU | | NO | | Aug | er Bo | orehol | | S. Lot 77(Sheet 5 | |
|--|--|--------------------------|--------------|--|-----------------------|-------------------------|---|---------------------|------------------------------|------------|
| - | | THE TOD, TONE | | Vane H | lead: | Logge | d By: | Process | | of 13 : |
| Job Number: | J00113 | | | 307 | 7 | | RZ | PL | 26 | .10.21 |
| Borehole <u>mN</u> Location: Description: | mE Bofor to site plan | Ground R.L. | | pu | (m) r | Standing Water Level | Vane Shear(kPa) _{peak} / _{residual} | il tivity | Sample | e and |
| Description | | N1 | | Legend | Depth (m) | Stane Vater | Val Shear ^{peak/re} | Soil Sensitivity | Laboratory Tes Deta | st |
| | SOIL DESCRIPTIO | | | | | > | | | | |
| | | | | | | | | | | |
| Lot 770 | | | | | | | | | | |
| TOPSOIL | | | | | - 0.0 - | | | | | |
| silty CLAY, light grey and | d red mottled orange/brown. V | ery stiff, moist, mediu | um plasticit | ty, | - | | | | | |
| _ insensitive [FILL] | | | | | - | | 150/115 | 1.2 | | |
| - | | | | | - 0.5 - | | 150/115 | 1.5 | | |
| - | | | | | - | | | | | |
| becoming moderately set | ensitive | | | | - | | 150/49 | 3.1 | | |
| - | | | | | - | | | | | |
| _ clayey SILT, grey mottled | d red. Hard, moist, low plastici | ty [ASH] | | | _ | | | | | |
| _ | | | | | - 1.5 | | 201+ | | | |
| - | | | | | - | | | | | |
| - | | | | | - | | | | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | ****** | - 2.0 | | 201+ | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| Lot 772 | | | | | - 0.0 | | | | | |
| TOPSOIL | | | | | - | | | | | |
| clayey SILT, orange/brov | vn. Very stiff, moist, medium p | lasticity, insensitive [| ASH] | ×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×= | - | | | | | |
| - | | | | ×=×=×=×=×=×=× ×=×=×=×=×=×=×=× ×=×=×=×=× | - | | 138/95 | 1.5 | | |
| - | | | | ×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×=×=× | - | | | | | |
| - | | | | ×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×=×=×=×=× | _ | | | | | |
| becoming orange and br | own mottled, low plasticity, wit | h trace fine sand | | ×-×-×-×-×-× ×-×-×-×-×-×-× ×-×-×-×-×-×-× | - 1.0 | | 150/98 | 1.5 | | |
| | own motified, low plasticity, wit | | | ×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× | F | | | | | |
| - | | | | ×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× | - | | | | | |
| becoming moderately ser | nsitive | | | ×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×= | - 1.5 | | 178/55 | 3.3 | | |
| - | | | | | _ | | | | | |
| at 2.0m, becoming hard | | | | ×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×= | - | | | | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x= | -2.0 | | 201+ | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | Comments: | Borehole Diameter: | Topsoil | Sa | and | | Sandstone | | Plutonic | ******* |
| | Groundwater not encountere | d. 50mm | Fill | ////// Gr | avel | | Siltstone | | No Core | |
| LANDER geotechnical | (unless noted) UTP = unable to penetrate. | Checked: | Clay | Or | ganic | | Limestone | | <u></u> | |
| | EOB = end of borehole. | RZ | Silt | ××××××××× ×××××××××××××××××××××××××××× | mice | | Volcanic | | <u></u> | |

| Client : | Project Location : HITCHEN BLOCK STAGE 13B, POKENO | | | | Aug | er Bo | oreho | e No | S. Lot 776 | 8 & 778 |
|---|---|----------------------------|-----------------|--|-----------------------|-------------------------|--|---------------------------------------|---------------------------|---------|
| Project Location | n: HITCHEN BLOCK ST | AGE 13B, POKE | NO | | | | | | Sheet 6 | of 13 |
| Job Number: | J00113 | | | Vane H 1750 | | Logge F | d By: PL | Process PL | | .10.21 |
| Borehole mN Location: Description: | | Ground R.L. | | p | (m) (| ding Level | ле (kPa) ^{sidual} | il ivity | Sample | |
| Location: Description: | Refer to site plan | | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak} residual | Soil Sensitivity | Laboratory Tes Deta | st |
| | | | | | | | | | | |
| | | | | | | | | | | |
| Lot 776 | | | | | | | | | | |
| TOPSOIL | | | | | - 0.0 | | | | | |
| | inge and dark grey streaked or | ange/brown. Hard, r | noist, | | - | | | | | |
| medium plasticity, with tra becoming dark grey and | | | | | - | | | | | |
| - | | | | | - 0.5 | | 270+ | | | |
| - | | | | | - | | | | | |
| plasticity [ASH] | orange/red streaked orange/b | own. Hard, moist, n | nedium | ×=×=×=×=×=×=× ×=×=×=×=×=× ×=×=×=×=×=×=× | - | | | | | |
| becoming orange and bro | own streaked light grey/brown | | | x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x-x-x | - 1.0 | | 270+ | | | |
| | k, light grey and dark grey mot | led. Hard, moist, no |) | | - | | | | | |
| _ plasticity | plasticity | | | | - | | 070 | | | |
| • | | | | | - 1.5 - | | 270+ | | | |
| | | | | | - | | | | | |
| - | | | | | - - 2.0 | | 270+ | | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | | - 2.0 | | 2701 | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| Lot 778 | | | | | - 0.0 | | | | | |
| TOPSOIL | | | | | - | | | | | |
| clayey SILT, orange, blac | k and brown mottled. Hard, mo | ist, low to no plastic | ty [ASH] | | - | | | | | |
| silty CLAY, black and light | t grey streaked orange/brown. I | Hard, moist, mediun | n plasticity | ×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× | - | | 270+ | | | |
| becoming dark red and or | range streaked orange/brown | | | ×-×-×-×-×-×-×-× ×-×-×-×-×-×-×-× ×-×-×-×-×-×-×-× ×-×-×-×-×-×-×-× | - | | | | | |
| - | nd orange streaked light grey/o | ange. Hard, moist, | medium | ×=×=×=×=×=×=× ×=×=×=×=×=× ×=×=×=×=×=×=× | - | | | | | |
| plasticity becoming orange streake | ed light grey, moderately sensiti | ve | | ×=×=×=×=×=×=× ×=×=×=×=×=×=×=× ×=×=×=×=× | -1.0 | | 254/119 | 2.1 | | |
| becoming light grey and r | ed streaked orange | | | x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x=x x= | - | | | | | |
| - | | | | ×=×=×=×=×=×=×=× ×=×=×=×=×=×=× ×=×=×=×=× | - | | | | | |
| slightly clavev SILT. orang | slightly clayey SILT, orange, light grey and black streaked brown. Dense, moist, no | | | | -1.5 | | 270+ | | | |
| plasticity | | | | | - | | | | | |
| at 2.0m, becoming sensitive | | | | | - | | | | | |
| | E.O.B. at 2.0m. Target Depth. | | | | -2.0 | | 231/35 | 6.7 | | |
| | gor D'oprin | | | | | | | | | |
| | | | | | | | | | | |
| | • · | Develope D' | [N | | | | | 1 | • Dista | |
| | Comments: Groundwater not encountered | Borehole Diameter: 50mm | Topsoil Fill | Sa Gra | nd avel | | Sandstone Siltstone | 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 | Plutonic | ****** |
| LANDER | (unless noted) UTP = unable to penetrate. | Checked: | Clay | | janic 🛱 | | Limestone | | | |
| geotechnical | EOB = end of borehole. | RZ | Silt XX | ××××××× ×××××××××××××××××××××××××××××× | nice | | Volcanic | | | |

| Client : Project Locatio | DFH JOINT VENTUR | | NO | | Aug | er Bo | orehol | | S. Lot 78 Sheet 7 | |
|---|--|------------------------------------|---------------|--------|--|-------------------------|---|---------------------------------------|-----------------------------|---|
| - | | | | Vane H | | Logge | | Process | or: Date |): |
| Job Number: | J00113 | | | 1750 | | I | 2 | PL | 2 | 6.10.21 |
| Borehole <u>mN</u> Location: Description: | mE Refer to site plan | Ground R.L. | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak/ residual} | Soil Sensitivity | Samp Laborato | e and y / Other |
| | SOIL DESCRIPTIO | N | | Leg | Dep | Sta Wate | Shea Peak | Sens | Te Det | st |
| plasticity, moderately ser at 0.4m, becoming orang | ge, light grey and brown streak isitive [ASH] e and light grey streaked brow and dark brown mottled orange | ed red/pink. Very stif n/orange | f, moist, low | | - 0.0 | | 169/81 177/73 173/65 162/81 | 2.1 2.4 2.6 2.0 | | |
| Lot 783 TOPSOIL | | | | | - 0.0 | | | | | |
| low to no plasticity, sensi | e sand, orange and light grey tive [ASH] y clayey SILT, no plasticity | streaked orange/red. | Hard, moisi | | - - - - - - - - - - | | 216/31 216/19 | 7.0 | | |
| - - - - - - - | | | | | - - - - - - - - - | | 189/23 200/15 | 8.2 | | |
| E.O.B. at 2.0m. Tar | get Depth. Comments: | Borehole Diameter: | Topsoil | Si | and | | Sandstone | 1 | Plutonic | +++++++++++++++++++++++++++++++++++++++ |
| | Groundwater not encountered (unless noted) | d. 50mm | Fill | //// G | ravel | | Siltstone | 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 | No Core | |
| LANDER geotechnical | (UTP = UTP | Checked: RZ | Clay | ××××× | ganic S Imice | | Limestone | | | |

| Project Location : HITCHEN BLOCK STAGE 138, POKENO Job Number: J00113 Workered: Larged by PUM Processor Dire: 7002153 Receive and manual space of the set plan TOPSOL Job Number: J00113 SOIL DESCRIPTION SOIL DESCRIPTION Lat 785 TOPSOL Interfere to the set plan TOPSOL Lot 785 Colspan="2">Soil DESCRIPTION Lot 786 Interfere to the set of the s | Client : | DFH JOINT VENTURE | | | | Aug | er Bo | oreho | e No | S. Lot 78 | 5 & 786 |
|--|--|--------------------------------------|-------------------------|--------------|---|----------------------------|------------------|----------------------------------|----------------|------------------|---|
| JOD NUMBER: JOD 113 Trop2153 PLUM PL 28.10.21 Borehold Location mit Rear to ale glan mg g <t< th=""><th>Project Locatio</th><th>n: HITCHEN BLOCK ST</th><th>AGE 13B, POKE</th><th>NO</th><th></th><th></th><th></th><th></th><th></th><th>-</th><th></th></t<> | Project Locatio | n: HITCHEN BLOCK ST | AGE 13B, POKE | NO | | | | | | - | |
| Lot 785 0.0 < | Job Number: | J00113 | | | | | | | | | |
| Lot 785 0.0 < | Dorenole | | Ground R.L. | | pue | (m) ti | iding ' Level | tne r(kPa) esidual | oil itivity | Sampl | e and v / Other |
| TOPSOIL -0.0 -0.0 sightly dipky SIT with trace fine sand, orange streaked yellow/brown. Very stiff, molet, it with an plasticity (ASH) -0.5 154.99 4.0 silly CLAY, orange streaked orange/ight grey -0.5 154.99 4.0 becoming orange streaked orange/ight grey -0.5 154.99 4.0 clayer, SIT, orange streaked orange/ight grey. -1.5 216.92 3.5 E.O.B. at 2.0m. Target Depth. -0.0 -1.5 270+ E.O.B. at 2.0m. Target Depth. -0.0 -0.5 181/135 1.3 becoming well -0.5 181/135 1.3 -1.5 2.0 2.0 E.O.B. at 2.0m. Target Depth. -0.5 181/135 1.3 -1.5 1.5 2.2 becoming high plasticity -0.5 181/135 1.3 -1.5 2.2 2.0 1.0 -1.5 2.2 2.0 1.0 -1.5 2.2 2.0 1.0 -1.5 2.2 2.0 1.0 -1.5 1.5 1.5 -1.5 1.5 2.2 2.0 1.0 -1.5 1.5 2.2 -1.5 1.5 2.2 | | • | | | Leg | Dept | Star Watei | Va Shea _{peak/ 1} | S. Sens | Te | st |
| TOPSOUL shipty Gray SIT with trace fine sand, orange streaked yellow/brown. Very stiff, moist, it with on a plasticity (ASH) -0.0 154.39 4.0 slipty CLAY, orange streaked orange/light grey -0.0 -0.5 154.39 4.0 becoming orange streaked orange/light grey -0.0 -0.5 154.39 4.0 clayer, SIT, orange streaked orange/light grey. -1.5 270.4 -1.5 270.4 E.O.B. at 2.0m. Target Depth. -0.0 -1.5 270.4 -1.5 270.4 E.O.B. at 2.0m. Target Depth. -0.5 191/135 1.3 becoming high plasticity -0.0 -1.5 191/135 1.3 becoming high plasticity -1.5 191/135 1.3 tat 2.0m. Target Depth. -1.5 190/11 122 becoming high plasticity -1.5 190/11 1.2 tat 2.0m. becoming intensitive -1.5 190/11 1.5 tat 2.0m. becoming intensitive -1.5 2.0 170/116 1.5 tat 2.0m. becoming intensitive -1.5 1.5 1.5 1.5 tat 2.0m. becoming intensitive -1.5 1.5 1.5 | | | | | | | | | | | |
| LDPSOIL Eighthy days SILT with trace fine sand, orange streaked yellow/brown. Very stiff, moist, involve no plasticity, sensitive -0.5 154.03 4.0 silptiv days SILT, orange streaked orange/light grey. -1.5 216.62 3.5 ecoming orange streaked orange/light grey. -1.5 270.4 -1.5 270.4 ecoming orange streaked orange/light grey. -1.5 270.4 -1.5 270.4 E.O.B. at 2.0m. Target Depth. -0.5 151/135 1.3 becoming high plasticity -0.5 151/135 1.3 becoming high plasticity -0.5 151/135 1.3 in Constraint -0.5 151/135 <td>Lot 785</td> <td></td> | Lot 785 | | | | | | | | | | |
| Low to no plasticity (LSH) 54.0 1.54.09 4.0 becoming orange streaked orange/light grey. Very stiff, moist, medium plasticity, moderately -0.5 1.54.09 4.0 clawy SIL r, orange streaked orange/light grey. -1.5 216.62 3.5 clawy SIL r, orange streaked orange/light grey. Hard, moist, low plasticity, moderately -1.5 270+ clawy SIL r, orange streaked orange/light grey. Hard, moist, low plasticity, moderately -2.0 270+ E.O.B. at 2.0m. Target Depth. -0.6 181/135 1.3 becoming high plasticity -0.5 181/135 1.3 thild CLAY, redorange and orange motiled light grey. Very stiff, moist, medium to high -0.5 181/135 1.3 becoming high plasticity -0.5 181/135 1.3 -1.0 182/96 1.3 tat 2.0m. becoming insensitive -1.5 198/61 3.2 -1.5 198/61 3.2 tat 2.0m. becoming insensitive -1.5 198/61 3.2 -1.5 198/61 3.2 tat 2.0m. becoming insensitive -1.5 198/61 3.2 -1.5 1.5 <t< td=""><td>TOPSOIL</td><td></td><td></td><td></td><td></td><td>- 0.0</td><td></td><td></td><td></td><td></td><td></td></t<> | TOPSOIL | | | | | - 0.0 | | | | | |
| becoming orange streaked orange/light grey -10 216/62 3.5 clayey SLT, orange streaked orange/light grey. Hard, moist, low plasticity, moderately -10 216/62 3.5 e.o.B. at 2.0m. Target Depth. -20 270+ -15 270+ E.O.B. at 2.0m. Target Depth. -00 -00 -00 -00 silly (2.4V, redorange and orange motied light grey. Very stiff, moist, medium to high plasticity -00 181/135 1.3 becoming wat becoming motiently sensitive -1.5 198/61 2.2 198/61 2.2 tat 2.0m. Target Depth. -1.5 198/61 1.3 -1.5 198/61 2.2 becoming wat becoming insensitive -1.5 198/61 2.2 170/116 1.5 E.O.B. at 2.0m. Target Depth. -2.0 170/116 1.5 1.5 1.5 1.6 becoming wat becoming insensitive -2.0 170/116 1.5 1.5 1.5 1.6 E.O.B. at 2.0m. Target Depth. 5mm 5mm 5mm 5mm 5mm 1.6 1.6 1.6 E.O.B. at 2.0m. Target Depth. 5mm 5mm 5mm 5mm </td <td></td> <td></td> <td>yellow/brown. Ver</td> <td>y stiff, moi</td> <td>st,</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td> | | | yellow/brown. Ver | y stiff, moi | st, | - | | | | | |
| clayey SLT, orange streaked orange/light grey. Hard, moist, tow plasticity, moderately -1.0 216/92 3.5 ensitive -1.5 270+ -1.5 270+ E.O.B. at 2.0m. Target Depth. -2.0 270+ -2.0 270+ silly CLAY, redorange and orange motited light grey. Very still, moist, medium to high plasticity, insensitive [ASH] -0.0 -0.0 -0.0 silly CLAY, redorange and orange motited light grey. Very still, moist, medium to high plasticity, insensitive [ASH] -0.5 181/135 1.3 becoming high plasticity -1.5 189/61 3.2 -1.5 198/61 3.2 at 2.0m, becoming insensitive -2.0 170/116 1.5 -2.0 1.0 1.5 -2.0 1.0 1.5 becoming wet -2.0 120/116 1.5 -2.0 1.0 1.5 -2.0 1.0 1.5 E.O.B. at 2.0m. Target Depth. -2.0 5 5 5 5 -2.0 1.0 1.5 -2.0 1.0 1.5 Bartele Damete: Comments: Commentare Commentare Commentare Commentare 1.0 5 5 -1.0 | silty CLAY, orange strea | ked light grey. Very stiff, moist, r | nedium plasticity, s | sensitive | 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - | - | | 154/39 | 4.0 | | |
| seisitive 10 2.10 2.10 0.3 E.O.B. at 2.0m. Target Depth. 20 270+ 2 E.O.B. at 2.0m. Target Depth. -0.0 -0.0 -0.0 Silv CLAY, redorange and orange motiled light grey. Very still, moist, medium to high plasticity. -0.5 181/135 1.3 becoming high plasticity -1.5 199/61 3.2 -1.5 199/61 3.2 becoming wet -0.0 -1.5 199/61 3.2 -1.5 199/61 3.2 to comming insensitive -1.5 199/61 3.2 -2.0 170/116 1.5 to comming insensitive -2.0 170/116 1.5 1.5 1.5 to comming insensitive -2.0 170/116 1.5 1.5 1.5 1.5 to comming insensitive -1.5 199/61 3.2 -2.0 170/116 1.5 to comments: Groundwater not encountered, UTP = unable to ponetrate. Topol Sand Sand Sand Sand Sand Sand Sand Sand | becoming orange streake | ed orange/light grey | | | | - | | | | | |
| E.O.B. at 2.0m. Target Depth. 20 270+ Lot 786 -0.0 -0.0 TOPSOIL -0.0 -0.0 sity CLAY, redorange and orange motiled light grey. Very still, moist, medium to high plasticity -0.5 181/135 1.3 becoming high plasticity -0.5 181/135 1.3 becoming wet becoming moderately sensitive -1.0 182/96 1.9 becoming moderately sensitive -1.5 198/61 3.2 E.O.B. at 2.0m. Target Depth. -2.0 170/116 1.5 FORMERTS: Sorthole Damter: -2.0 Sandatore -2.0 Connents: Sorthole Damter: -2.0 Sandatore -2.0 File - end of benchole. -2.0 20 -2.0 -2.0 | | aked orange/light grey. Hard, mo | pist, low plasticity, r | moderatel | y xxxxxxxx xxxxxxxxx xxxxxxxxxx xxxxxxxx | - | | 216/62 | 3.5 | | |
| E.O.B. at 2.0m. Target Depth. -2.0 270+ Lot 786 -0.0 -0.0 TOPSOIL -0.0 -0.0 sitty CLAY, redorange and orange motiled light grey. Very stiff, moist, medium to high plasticity -0.5 181/125 1.3 becoming high plasticity -0.5 181/125 1.3 becoming moderately sensitive -1.0 182/36 1.9 becoming moderately sensitive -1.5 198/61 3.2 E.O.B. at 2.0m. Target Depth. -2.0 170/116 1.5 Comments: Sorthole Dameter Topsal Sard Sard target Plance Plance Groundwater not encountered, UTP = unable to penetrate. RZ Paraget Market Plance Plance Plance DECONTING RZ Paraget Market Plance Plance Plance Plance Plance | - | | | | | - | | | | | |
| E.O.B. at 2.0m. Target Depth. | - - | | | | | - | | 270+ | | | |
| E.O.B. at 2.0m. Target Depth. | - | | | | | - | | | | | |
| TOPSOIL 0.0 0.0 0.0 silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH] 0.0 181/135 1.3 becoming high plasticity 0.0 181/135 1.3 1.3 becoming wet 0.0 182/96 1.9 becoming moderately sensitive 1.5 198/61 3.2 at 2.0m, becoming insensitive 2.0 170/116 1.5 E.O.B. at 2.0m. Target Depth. 50mm 5mm 5mm Sand Sandatore 9000000000000000000000000000000000000 | - E.O.B. at 2.0m. Tar | get Depth. | | | | - | | 270+ | | | |
| TOPSOIL -0.0 -0.0 -0.0 silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH] -0.5 181/135 1.3 becoming high plasticity -0.5 181/135 1.3 becoming wet -0.5 182/96 1.9 becoming moderately sensitive -1.0 182/96 1.9 at 2.0m, becoming insensitive -1.5 198/61 3.2 et 2.0m, becoming insensitive -2.0 170/116 1.5 E.O.B. at 2.0m. Target Depth. Sand Sand to 2 Sand to 2 Groundwater not encountered. (nees noted) Othecked: Checked: Carvet Sand to 2 UTP = unable to penetrate. Sand Sand to 2 Sand to 2 No Core Checked: Cay Carvet Sand to 2 Sand to 2 No Core | | | | | | | | | | | |
| TOPSOIL -0.0 -0.0 -0.0 silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH] -0.5 181/135 1.3 becoming high plasticity -0.5 181/135 1.3 becoming wet -0.5 182/96 1.9 becoming moderately sensitive -1.0 182/96 1.9 at 2.0m, becoming insensitive -1.5 198/61 3.2 et 2.0m, becoming insensitive -2.0 170/116 1.5 E.O.B. at 2.0m. Target Depth. Sand Sand to 2 Sand to 2 Groundwater not encountered. (nees noted) Othecked: Checked: Carvet Sand to 2 UTP = unable to penetrate. Sand Sand to 2 Sand to 2 No Core Checked: Cay Carvet Sand to 2 Sand to 2 No Core | | | | | | | | | | | |
| TOPSOIL -0.0 -0.0 -0.0 silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH] -0.5 181/135 1.3 becoming high plasticity -0.5 181/135 1.3 becoming wet -0.5 182/96 1.9 becoming moderately sensitive -1.0 182/96 1.9 at 2.0m, becoming insensitive -1.5 198/61 3.2 et 2.0m, becoming insensitive -2.0 170/116 1.5 E.O.B. at 2.0m. Target Depth. Sand Sand to 2 Sand to 2 Groundwater not encountered. (nees noted) Othecked: Checked: Carvet Sand to 2 UTP = unable to penetrate. Sand Sand to 2 Sand to 2 No Core Checked: Cay Carvet Sand to 2 Sand to 2 No Core | | | | | | | | | | | |
| silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH] becoming high plasticity -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 182/96 1.9 -1.5 198/61 3.2 -1.5 198/61 3.2 -1.5 198/61 3.2 -1.5 198/61 3.2 -2.0 170/116 1.5 Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = not of borehole. BZ Stat During Comments: C | | | | | | - 0.0 | | | | | |
| plasticity, insensitive [ASH] becoming high plasticity -0.5 181/135 1.3 becoming high plasticity -1.0 182/96 -1.0 182/96 -1.0 182/96 -1.0 182/96 -1.0 182/96 -1.0 182/96 1.9 -1.5 198/61 3.2 -1.5 198/61 3.2 -2.0 170/116 1.5 2.0 170/116 1.5 2.0 170/116 1.5 2.0 170/116 1.5 2.0 170/116 1.5 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 2.0 | | nd orange mottled light grev. Ver | v stiff moist medi | um to hiat | × | - | | | | | |
| becoming high plasticity = 1.0 182/96 1.9 = 1.0 182/96 1.9 = 1.0 182/96 1.9 = 1.5 198/61 3.2 = 1.5 198/61 3.2 = 2.0 170/116 1.5 E.O.B. at 2.0m. Target Depth. = 2.0 170/116 1.5 = 2.0 170/116 1.5 1.5 = 2.0 170/116 1.5 | | | y stin, moist, mean | uni to nigi | ×=×=×=×=×=×=× ×=×=×=×=×=×=× ×=×=×=×=×=× | - | | | | | |
| becoming wet becoming moderately sensitive at 2.0m, becoming insensitive E.O.B. at 2.0m. Target Depth. Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole. Borehole Diameter: Somm Fill Checked: Clay Corganic Sand Sand Sand Sand Sand Sand Sand Sand Sand Sittsone Corganic Sittsone Corganic Sittsone Sitt | becoming high plasticity | | | | | - 0.5 - - | | 181/135 | 1.3 | | |
| becoming wet becoming moderately sensitive at 2.0m, becoming insensitive E.O.B. at 2.0m. Target Depth. Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole. Borehole Diameter: Somm Fill Checked: Clay Corganic Sand Sand Sand Sand Sand Sand Sand Sand Sand Sittsone Corganic Sittsone Corganic Sittsone Sitt | - | | | | × × × × × × × × × × × × × × × × × × × | - - | | 100/00 | | | |
| becoming moderately sensitive 198/61 3.2 at 2.0m, becoming insensitive -2.0 170/116 1.5 E.O.B. at 2.0m. Target Depth. -2.0 170/116 1.5 Image: Sensitive Sensitive -2.0 170/116 1.5 Image: Sensitive Sensitive Sensitive -2.0 170/116 1.5 Image: Sensitive Senst | - - - | | | | | | | 182/96 | 1.9 | | |
| at 2.0m, becoming insensitive -2.0 170/116 1.5 E.O.B. at 2.0m. Target Depth. -2.0 170/116 1.5 Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole. Borehole Diameter: 50mm Topsoil Sand Sand Sandstone Plutonic Edited to the penetrate. EOB = end of borehole. | | | | | | - | | | | | |
| E.O.B. at 2.0m. Target Depth. Plutonic E.O.B. at 2.0m. Target Depth. Plutonic Image: Second Secon | becoming moderately se | nsitive | | | | - 1.5 - - | | 198/61 | 3.2 | | |
| E.O.B. at 2.0m. Target Depth. Plutonic E.O.B. at 2.0m. Target Depth. Plutonic Image: Second Secon | - at 20m becoming incom | sitive | | | | - | | | | | |
| LANDER Groundwater not encountered. (unless noted) 50mm Fill Gravel Siltstone 222222222222222222222222222 No Core UTP = unable to penetrate. EOB = end of borehole. RZ Silt Silt Values of the second of the | - | | | | <u>x-x-x-x-x-x-x</u> <u>x-x-x-x-x-x-x</u> | - 2.0 | | 170/116 | 1.5 | | |
| LANDER Groundwater not encountered. (unless noted) 50mm Fill Gravel Siltstone 222222222222222222222222222 No Core UTP = unable to penetrate. EOB = end of borehole. RZ Silt Silt Values of the second of the | | | | | | | | | | | |
| LANDER Groundwater not encountered. (unless noted) 50mm Fill Gravel Siltstone 222222222222222222222222222 No Core UTP = unable to penetrate. EOB = end of borehole. RZ Silt Silt Values of the second of the | | Commente | Borehole Diamator | Tanas | | nd 🖸 | | Condeter | | Plutonia | +++++++++++++++++++++++++++++++++++++++ |
| LANDER geotechnical UTP = unable to penetrate. EOB = end of borehole. RZ Silt ******** Pumice ************************************ | | | | | - <u>K<<<<</u> | | 22222 | | 222222 | | ****** |
| geotechnical EOB = end of borehole. RZ | | (unless noted) | | | | | ****** | | | | |
| | | | | | ×××××××× ×××××××× | 64 | | 5. | | <u>루</u> | |

| Client : Project Locatio | DFH JOINT VENTURE I | | NO | | Aug | er Bo | oreho | | S. Lot 788 Sheet 9 | |
|---|--|----------------------------|-----------------|---|--|-------------------------|---|---------------------|------------------------------|-------------------|
| Job Number: | J00113 | - | Vane H | | Logge | - | Process | or: Date: | : | |
| | | ound R.L. | | 2153 | | NL e | | PL | 26 | .10.21 |
| Borehole MN Location: Description: | | | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak} / _{residual} | Soil Sensitivity | Sample Laboratory | / / Other |
| | SOIL DESCRIPTION | | | Leç | Dep | Sta Wate | V Shea peak∕ | Sen | Tes Deta | |
| moist, medium to high pla [FILL] becoming yellow/brown, find becoming hard silty CLAY with trace fine | eaching | trace fine to med | ium gravel | | - 0.0 | | 191/88 216+ 216+ 216+ | 2.2 | | |
| Lot 790 | | | | | - 0.0 | | | | | |
| TOPSOIL | | | | | - | | | | | |
| | range, yellow/brown and grey/brown trace fine to medium gravel [FILL] | | rey. Hard, | | - - - - - - | | 216+ | | | |
| silty CLAY, orange mottle FORMATION] | ed light grey. Hard, moist, high plas | ticity [PUKETOK | A | X-X-X-X-X-X-X X-X-X-X-X-X-X X-X-X-X-X-X | - 1.0 | | 216+ | | | |
| becoming orange and rec becoming very stiff, inser | | | | | - - - - - - - - - - | | 198/136 185/142 | 1.5 | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | | | | | | :1 | <u> +++++++++</u> |
| | Comments: Groundwater not encountered. | Borehole Diameter: 50mm | Topsoil Fill | }}}} | and avel | | Sandstone Siltstone | | Plutonic | ******* |
| LANDER | (unless noted) UTP = unable to penetrate. | Checked: | Clay | | ganic | ******* | Limestone | | | |
| geotechnical | EOB = end of borehole. | RZ | Silt | ××××××× ×××××××× ×××××××× | mice | | Volcanic | | ~ | |

| Client : | DFH JOINT VENTURE L | | | | Aug | er Bo | oreho | | S. Lot 79 | 5 & 797 |
|---|--|----------------------|--------------|----------------------|----------------------------|--|---|---|--|--|
| Project Locatio | n: HITCHEN BLOCK STAC | GE 13B, POKEI | NO | | | | | | | of 13 |
| Job Number: | J00113 | | | Vane H 2153 | | Logge J | d By: M | Process PL | | .10.21 |
| Borehole mN | mE Gro | ound R.L. | | | Ê | ng evel | e Pa) ^{dual} | ity | Sample | and |
| Location: Description: | Refer to site plan | | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak} / _{residual} | Soil Sensitivity | Laborator | / Other |
| | SOIL DESCRIPTION | | | Le | De | St Wat | She | Sei | Deta | |
| | | | | | | | | | | |
| Lot 795 | | | | | | | | | | |
| | | | | | - 0.0 | | | | | |
| silty CLAY, orange and yo [FILL] | ellow/brown mottled light grey. Ver | y stiff, moist, higł | n plasticity | y | | | | | | |
| silty CLAY with minor fine becoming high plasticity, | e sand, yellow/brown. Hard, moist, with trace fine sand | medium plasticity | / [ASH] | | -0.5 - | | 216+ | | | |
| becoming very stiff, insen | sitive | | | | - | | 193/130 | 1.5 | | |
| - | | | | | | | | | | |
| becoming hard . | | | | | - 1.5 - | | 216+ | | | |
| becoming orange and rec | d/orange mottled light grey | | | | F | | | | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | <u>x=x=x=x=x=x=x</u> | - 2.0 | | 216+ | | | |
| Lot 797 | | | | | - 0.0 | | | | | |
| TOPSOIL | | | | | | | | | | |
| silty CLAY with trace fine moist, medium plasticity, | sand, orange and red/orange mot insensitive [ASH] | ted light grey. Ve | ry stiff, | | - - - - - - | | 178/102 | 1.7 | | |
| - becoming light yellow/bro - | wn, stiff, wet, with trace fine to me | dium sand | | | - - - | | 142/73 | 1.9 | | |
| - | | | | | - | | 77/42 | 1.8 | | |
| - | | | | | - | | | | | |
| E.O.B. at 2.0m. Tar | get Depth. | | | <u>b-x-x-x-x-x-x</u> | -2.0 | | 99/56 | 1.8 | | |
| | Comments: | Borehole Diameter: | Topsoil | Sa State | and | | Sandstone | , | Plutonic | ++++++ +++++++ +++++++++++++++++++++++ |
| | Groundwater not encountered. (unless noted) | 50mm | Fill | G | ravel | | Siltstone | 2 | ² 2 2 2 2 2 2 | |
| LANDER geotechnical | UTP = unable to penetrate. | Checked: RZ | Clay | < | ganic 🕄 | | Limestone | | <u>異</u> | |
| | EOB = end of borehole. | ΠĽ | Silt | XXXXXXXX Pu | ımice | ************************************** | Volcanic | | | |

| Client : Project Locatio | NO | | Aug | er Bo | oreho | | S. Lot 798 Sheet 11 | | | |
|---|---|-----------|-----|--------|---|-------------------------|--|--|-------------|---------|
| Job Number: | Job Number: J00113 Borehole mN mE Ground R.L. | | | | | Logge F | d By: RZ | Process PL | sor: Date | |
| Borehole mN Location: Description: | | ound R.L. | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak} / residual | Soil Sensitivity | Sample | / Other |
| | SOIL DESCRIPTION | | | Le | Del | Sta Wate | ∨ She _{peak} | Sen | Tes Deta | |
| Lot 798 silty CLAY, orange. Very stiff, moist, medium plasticity, sensitive [ASH] becoming high plasticity becoming moderately sensitive becoming black mottled brown, low plasticity, with some magnese oxide inculsions becoming hard E.O.B. at 2.0m. Target Depth. | | | | | - 0.0 - - - - - - - - - - - - - | | 115/29 161/46 201+ 201+ | 4.0 | | |
| TOPSOIL silty CLAY, orange and li [FILL] | | | | | -0.0 -0.5 -1.0 -1.5 -2.0 | | 201+ 201+ | | Plutonic | ******* |
| Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole.Borehole Diameter: 50mmTopsoilChecked: RZClay sitt | | | | | Gravel Organic | | Sandstone Siltstone Limestone | 2 | No Core | |

| Client : | DFH JOINT VENTURE I | | | | Aug | er Bo | oreho | le No | S. Lot 80 |)2 & 806 |
|--|--|----------------------------|-----------------|---|-------------------------|-------------------------|---|---------------------|---|---|
| Project Locatio | n: HITCHEN BLOCK STAC | GE 13B, POKE | NO | | | | | | Sheet 12 | |
| Job Number: | J00113 | | | Vane I 307 | | Logge F | d By: RZ | Process PL | | e: 6.10.21 |
| Borehole mN | mE Gro | ound R.L. | | | | | | | | |
| Location: Description | | | | Legend | Depth (m) | inding er Lev | Vane Shear(kPa) _{peak/ residual} | Soil Sensitivity | Sampl Laborator | ry / Other |
| | SOIL DESCRIPTION | | | Leç | Dep | Standing Water Level | Shea Peak | Sen | Te Det | |
| Lot 802 | | | | | - 0.0 | | | | | |
| TOPSOIL | | | | | 10.0 | | | | | |
| _ (FILL) | AND, orange mottled brown. Mediu | ım dense, moist, | low plastic | sity | - - - - 0.5 | | UTP | | | |
| EOB at 0.7m. Too hard t - - - | o auger further | | | | - - | | | | | |
| - - - - | | | | | - - - 1.5 - | | | | | |
| - - - | | | | | - - - 2.0 | | | | | |
| Lot 806 | | | | | | | | | | |
| TOPSOIL | | | | | - 0.0 - | | | | | |
| silty fine SAND, brown. N | ledium dense, dry, no plasticity, wi | th trace fine grav | el [FILL] | | | | | | | |
| EOB at 0.4m. Too hard to | o auger further | | | | -0.5 | | | | | |
| - | | | | | F | | | | | |
| - | | | | | F | | | | | |
| - | | | | | - 1.0 | | | | | |
| - | | | | | F | | | | | |
| - | | | | | | | | | | |
| - | | | | | - 1.5 | | | | | |
| - | | | | | F | | | | | |
| - | | | | | -2.0 | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | Developing in | 4 | | E | | | | | +++++++++++++++++++++++++++++++++++++++ |
| | Comments: Groundwater not encountered. | Borehole Diameter: 50mm | Topsoil Fill | ***** | and Gravel | | Sandstone Siltstone | | Plutonic ²/₂ No Core | ++++++++ |
| LANDER | (unless noted) UTP = unable to penetrate. | Checked: | Clay | | rganic | ******* | Limestone | 222222 | 2 | |
| geotechnical | EOB = end of borehole. | RZ | Silt | ××××××××× ×××××××××××××××××××××××××××× | umice | | Volcanic | | | |

| Client : | DFH JOINT VENTURE I | | | | Aug | er Bo | oreho | le No | S. Lot 80 | 7 & 809 |
|---|---|--|-------------------------|--------------|--------------------------------------|-------------------------|---|---------------------|------------------|--------------|
| Project Locatio | n: HITCHEN BLOCK STAC | GE 13B, POKEI | NO | | | 1 | | - | Sheet 1 | |
| Job Number: | J00113 | | | Vane H 30 | | Logge F | d By: RZ | Process PL | | : 6.10.21 |
| Borehole mN | mE Gro | ound R.L. | | | Ê | ng evel | ⊃a) tual | ity | Sampl | e and |
| Location: Description | Refer to site plan | | | Legend | Depth (m) | Standing Water Level | Vane Shear(kPa) _{peak/ residual} | Soil Sensitivity | Laborator | y / Other |
| | SOIL DESCRIPTION | | | د | ă | Na Si | Sh. Pea | Se | Det | |
| [FILL] becoming low plasticity, | orown. Medium dense, moist, low p | | n plasticity | | - 0.0 | | UTP | | | |
| insensitive [FILL] becoming brown, with training | e mottled brown, low plasticity, with | | m plasticity | | - 0.0 - 0.5 - 1.0 - 1.5 | | 187/141 201+ | 1.3 | | |
| LANDER geotechnical | Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole. | Borehole Diameter: 50mm Checked: RZ | Topsoil Fill Clay | Gi | and ravel ganic | | Sandstone Siltstone Limestone Volcanic | | No Core | |



Appendix 5

Construction Observation Records

09/10/19 - Stage 5 begins - Fill E/F

Wednesday, 9 October 2019 10:52 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 09/10/19 - 10:30am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to inspect gully muckout for Fill E and F. Gully stripped to generally inorganic stiff ground.

Small area of organics observed and contractor will chase this out.

Contractor also to install nominal toe key. Discussed with Trevor that this should be a buckets width wide at base and should extend across the face of the gully. Trevor to send photos once this is complete.







Photos from Trevor (10/10/19):







Photos from Trevor (14/10/19):





16/10/19 - Fill E/F

Wednesday, 16 October 2019 12:13 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 16/10/19 - 11AM |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

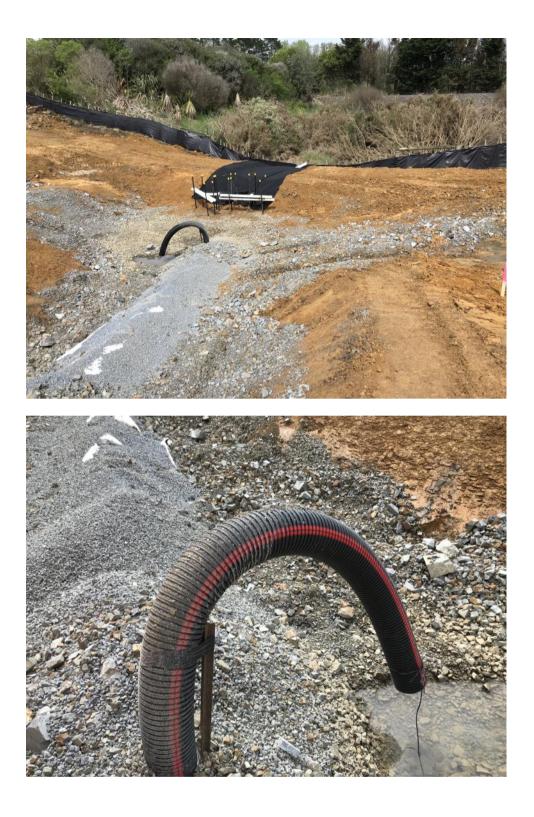
Visited site with Chris and met with Trevor and Dillan (who will be replacing Trevor soon).

Discussed likely outcomes for this season and next at Hitchen Stage 5 and also discussed toe key requirements - these being nominal keys 4-5m wide at base and keyed into the sides of the gullies.

Will return to site tomorrow morning to inspect remediated toe key in fill F which is being widened to meet the 5m width criteria.







Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 17/10/19 11:30PM |
| Author: | KM |
| Plant Operating: | 1x 20T excavator |
| Weather: | Showers |

Site Observations and Instructions:

Visited site to inspect excavation of toe key in Fill F. Key excavated 5m at base with very stiff material at base as per our requirements. Some water ponding at base which will require pumping prior to filling.





21/10/19 Monday, 21 October 2019 10:17 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 21/10/19 - 8:30AM |
| Author: | КМ |
| Plant Operating: | Moxys and excavators |
| Weather: | |

Site Observations and Instructions:

Visited site to check earthworks operations in Fill E/F areas. Toe key has been filled up and tesed since last visit and contractors are beginning to form permanent pond wall.









24/10/19 - Fill K

Thursday, 24 October 2019 3:02 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 24/10/19 - |
| Author: | |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan (Dines) to inspect stripping in area of Fill K / Cut 2. Only a small area within the area of the silt pond had been stripped back to very stiff natural ground. The gully for Fill K had no been mucked out yet, and no excavations to form the silt pond had been started either.

Discussed with Dylan that we should return to site once the gully is mucked out and we can carry out a shallow trial pit in the area of the proposed toe key prior to excavations.









Tuesday, 29 October 2019 10:13 AM

Site Inspection Record

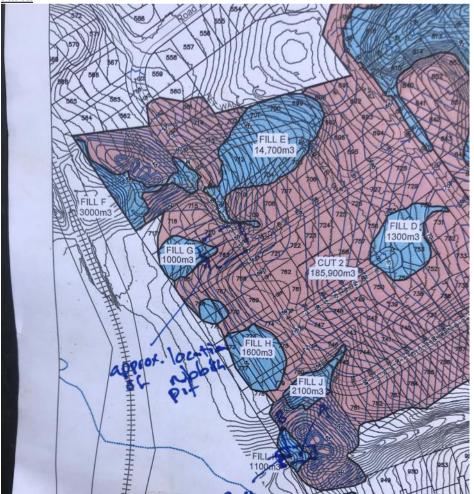
| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 29/10/19 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

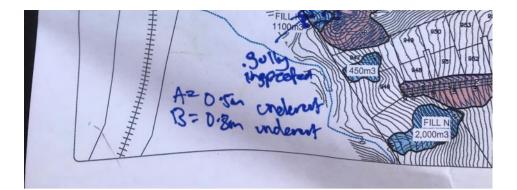
Visited site to inspect Fill K gully muckout. Inorganic, natural soils exposed throughout, however, soils were firm along a large part of the gully invert. Drilled two shallow hand augers which revealed stiff natural ground approximately 0.5m to 0.8m below current invert levels and we recommended an undercut to this depth. Refer plan attached.

Discussed the gully undercut with Dylan on site and he requested we do the undercut while we were on site to save time. See photos showing undercut of between approximately 0.5m and 1.2m.

Also found an old rubbish pit exposed which is marked out by the contractors. This will require undercutting also.



Fill K:











Rubbish pit:





Fill F walkover:





Fill K undercut/pit photos:



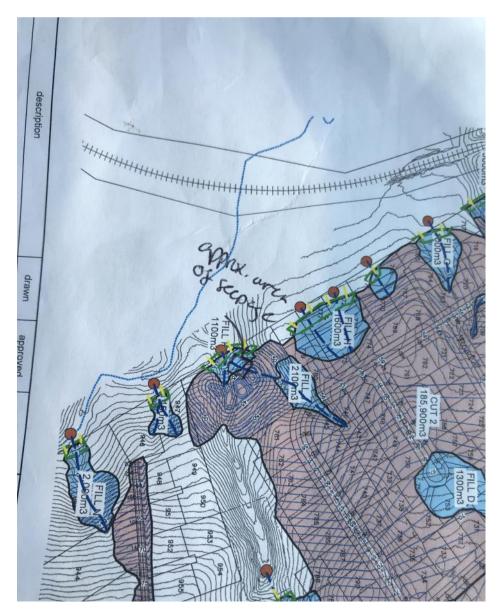


| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 30/10/19 - 12pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

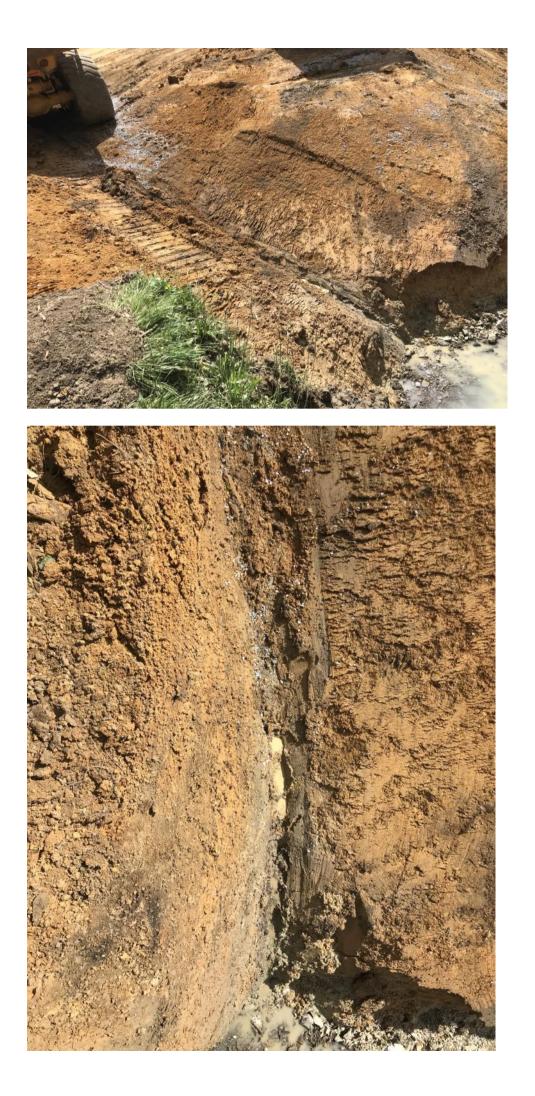
Site Observations and Instructions:

Visited site to observe gully muckout and drain installation. Muckout was deeper than our previous recommendation due to a 1m clay cap needed below the pond.

There is a small area of seepage near the base of the gully muckout which showed some seepage. We recommended that an extra drain is installed here to connect into the main underfill, ensure a 1m clay cap beyond the pond is satisfied. We will prepare a CAN detailing this.











01/11/19 - Fill A gully

Friday, 1 November 2019 10:12 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 01/11/19 - 9:30am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan to provide guidance on REB shear key undercut requirements. I was not made aware that I would be looking at this before arriving on site.

Asked contractors/surveys to peg the far edge of the shear key alignment and also the silt pond so we can dig some trial pits next week. I will discuss a time to come to site with Chris next week.

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 05/11/19 - 10am |
| Author: | KM |
| Plant Operating: | 1x 20T excavator |
| Weather: | Fine |

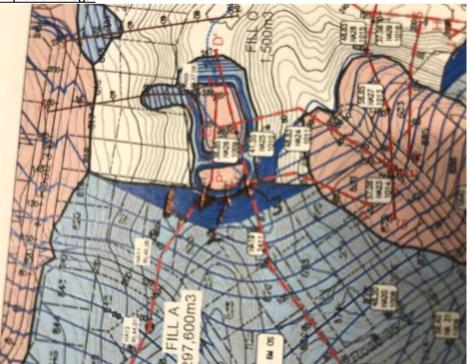
Site Observations and Instructions:

Visited site with Chris to carry out a series of trial pits at the toe of the REB1 fill batter. Our general logs are shown below, with hard natural soils (UTP) generally being encountered at around 3m to 3.5m depth below existing ground level.

The contractors will be continuing to excavate the first section of the gully undercut / installing drainage, and we will return to site at a later stage to prove the hard ground encountered (as our pits ended at the depths above due to digger reach).

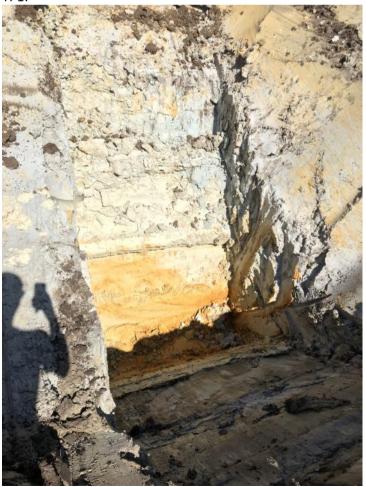
While on site we also visited the pond area in Fill K. There are groundwater seepages, however, are are at the base of the pond. Advised the contractors they should continue preparing the pond, with the likely outcome being a underfill drain connecting into the primary collector drain.

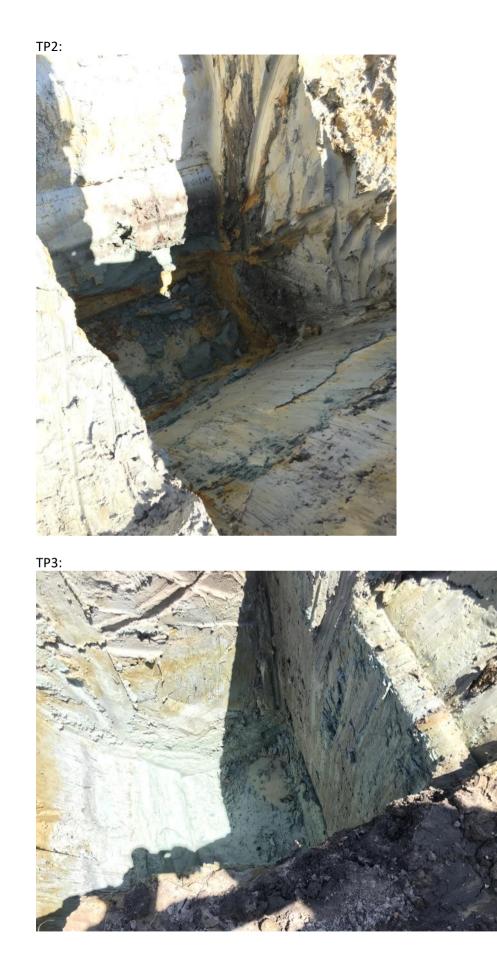
REB plan and logs:



TPI ~ GL FL = 38.85. -> Base approx - 3m. (RL 35:56?) -> avange sad manhad @ -3m approx. with - No grand water TP2 - 0 GL RL = 38.1 -D Base approx - 3.2m -> or nge sid encaled Q -2.8 m aprox UTP - Gw seapzye @ In TP3 -> GL RL = 37 - Base approx - 3m -D Grey/Blue smaly laye with in base -Duw sellinge Colon & Zun

TP1:







Fill K photos:





07/11/19 - Fill K and Fill A

Thursday, 7 November 2019 2:27 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 07/11/19 - 11:30am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan to carry out an inspection of the toe key in Fill K and also to conduct further trial pits below REB1 (to provide the hard layer below the shear key).

Fill K had been excavated approx. 3m depth and shear vanes in excess of 100kPa were recorded at the base of the key. The key is still to be excavated further, however, the lower level in the photos below (which is the be the final key depth) had sufficiently removed all gully mullock.

Carried out three trial pits within the area of the shear key and one within the pond area. The ground had been brought down approximately 2m from our previous visit when the initial trial pits were carried out. The additional pits found the hard layer identified during the previous visit at approximately 1m below the top of the pit, and this was proven for approximately 3m beyond this depth (the excavator was struggling to remove the material at the base of the pit with a rock bucket).

Within the pond area the contractors had already excavated about 2m - 2.5m from original ground level, exposing stiff ground at this level. The hard layer identified in the previous trial pits was identified approximately 2m from this level.



Fill K:







REB 1 area:



D Stored appart 2n below parting/orisin-1 fact Hand e ulm depth excurse enasting to remark TP depth for both = him 2) Storm approx 2-3- below previous lovel Hard e ~ I'm depth such = 4m min 3 stall eporor 2n below provins level Hard e ~ In dupth Dever = m (b) Apprel 2m brides original GL ~ bolips at this has Herd & N2m from This level = 6 are of p.1







Pit 2:













| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 12/11/19 - 2:30PM |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site with Chris and Shane to discuss construction methodology at REB1 shear key (involving subgrade depth of shear key and requirements for bench and drainage of key, especially into the ridge/flanks of the key).

There is still some work for Dines to do to facilitate access into this area. We will return to site once a section of the shear key has been opened to approve the subgrade.

15/11/19 - REB1 pond wall

Friday, 15 November 2019 4:29 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 15/11/19 - 4pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to inspect toe key embedment for pond wall below permanent pond at REB1.

Key had been excavated 1m into very stiff to hard ground (>170kPa).

Recommended hardfill be placed until above wet ground (say 0.5m to 1m) ensuring that as much clay is placed as possible to ensure a good lining around the pond. As told contractors that cleggs need to be carried out on top of the hardfill prior to compaction with clay.

Sides of batter are also sub-vertical. Recommended these be battered to a safe angle to provide a safe working area below.







| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 20/11/19 - 2pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan to inspect pond wall key subgrade. Hard natural ground observed throughout and keyed minimum 1m into the hard ground.

Previous subgrade inspected had been backfilled with SPR.

Also discussed batter with contractors. They will be keying in with fill soon, however, they were advised to batter back / cover with plastic if it is be left open for much longer.

Ponds have been formed in Fill F / K areas. In Fill E the contractors are laying out the clay to dry.









22/11/19 - REB1 pond base

Friday, 22 November 2019 10:19 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 22/11/19 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan to inspect base of pond which will be filled approx. 0.5m to 1m to get to design level.

Ground had softened at the base of the pond due to water seepages. Recommended that a nominal undercut of say 0.2m to 0.5m be carried out (based on shallow hand augers drilled), ensuring the area remains drained (i.e. leave the pumps on while filling) to ensure the subgrade stays dry. We will need to inspect the final subgrade and so recommended this should be carried out in small stages with filling operations commencing immediately after we have seen the subgrade.

Also brought up the idea of laying some trenches to catch water seepages. I will discuss this with Chris later and prepare a CAN outlining our recommendations.



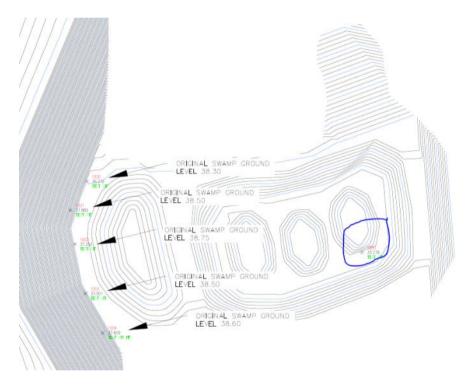




| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 26/11/19 - 2:30pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to inspect first area of pond undercut (cut 4) as shown in blue on the plan below. Stiff natural ground in excess of 60kPa throughout. There is a small area of water ponding in the area of deepest cut and we recommended that that be pumped out / dug out prior to placing fill.



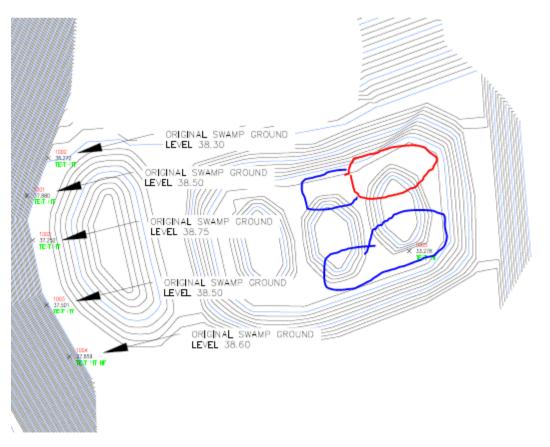


| Project # & Name: | J00113 - Hitchen Block |
|-------------------|-------------------------------|
| Date & Time: | 27/11/19 - 2:30pm |
| Author: | КМ |
| Plant Operating: | 2x20T excavators, 4x30T moxys |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan to inspect next section of pond base subgrade to fill up to pond invert. Stiff to very stiff, inorganic natural ground exposed throughout (blue). The hardfill track still remains to be removed and we will look at this once they remove this. The area in red was not fully stripped yet, however, several areas were exposed with ground being consistent throughout. Asked contractor to send photos once complete so they commence filling to stabilise a small slip which had formed in the side of the bank.

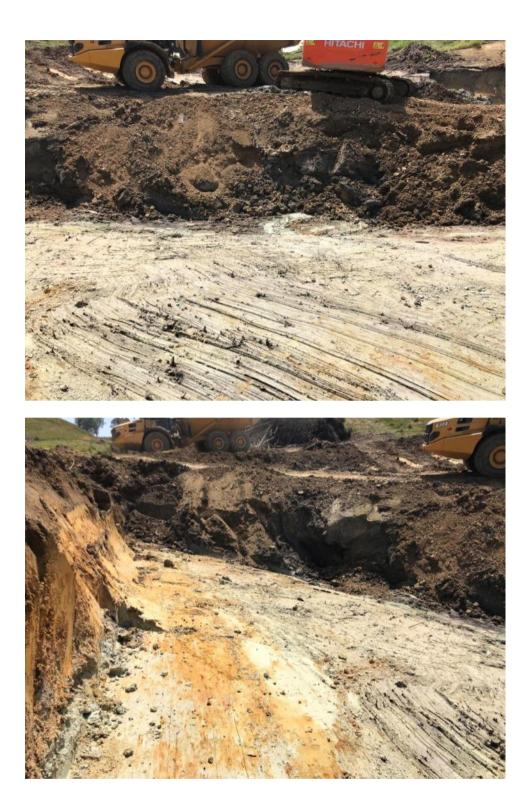
Also had a look at fill G and Fill E. Fill G and surrounding areas of Cut 2 stripped back to competent natural ground. Fill E near design level. This area currently being used to dry out the wet materials coming from cut 2 pond.















Photos from Dylan 29/11/19:





| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 29/11/19 - 12PM |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan to inspect next section of pond base fill subgrade. Very stiff ground throughout. Some areas had softened where water is ponding and we recommended that these areas be undercut to remove the softer soils. We have now seen almost all of the pond base. There is only a small area and Dylan will send photos once this is down to subgrade level.

Pond wall has been filled to near final level.





Photos from Dylan 2/12/19:



03/12/19 Tuesday, 3 December 2019 12:20 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 03/12/19 - 11pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

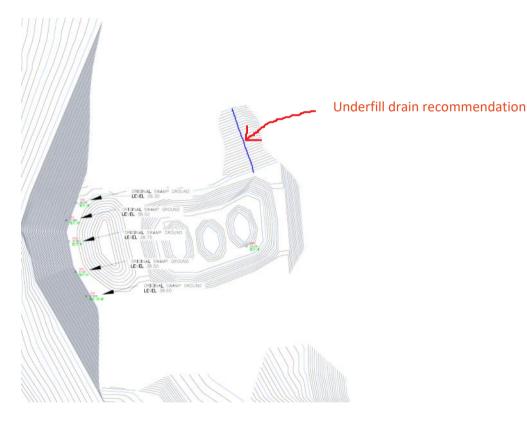
Site Observations and Instructions:

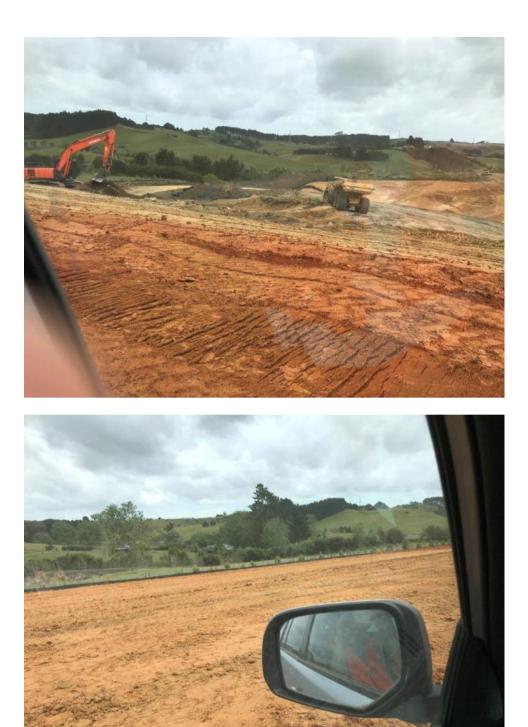
Visited site with Chris to see what is happening on site.

Observed start and outlet of underfill drain which outlets below Fill K.

Observed Cut 4 pond. This area has been filled with clear and is now near pond base invert level. While down here discussed a proposed fill batter which we recommended has an underfill drain installed to tap any groundwater pressures (area shown on the plan below).

Contractors also removing fill from an old rubbish pit found on site at the start of the season.















09/12/19 - Fill A

Monday, 9 December 2019 3:16 PM

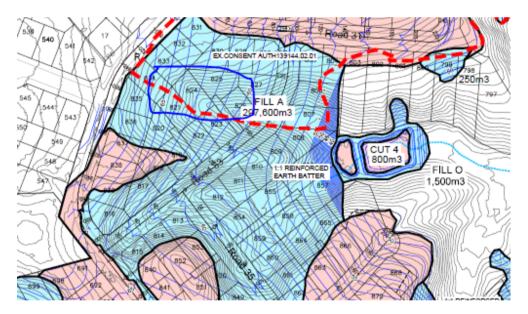
Site Inspection Record

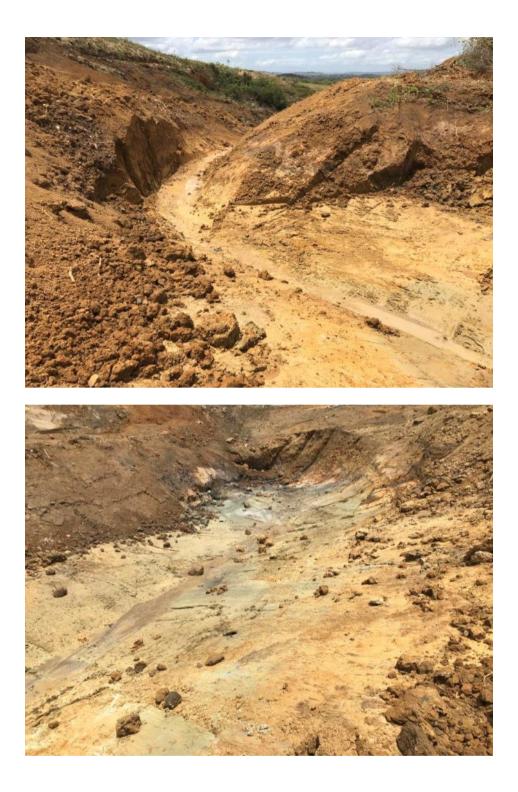
| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 09/12/19 - 1:30pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

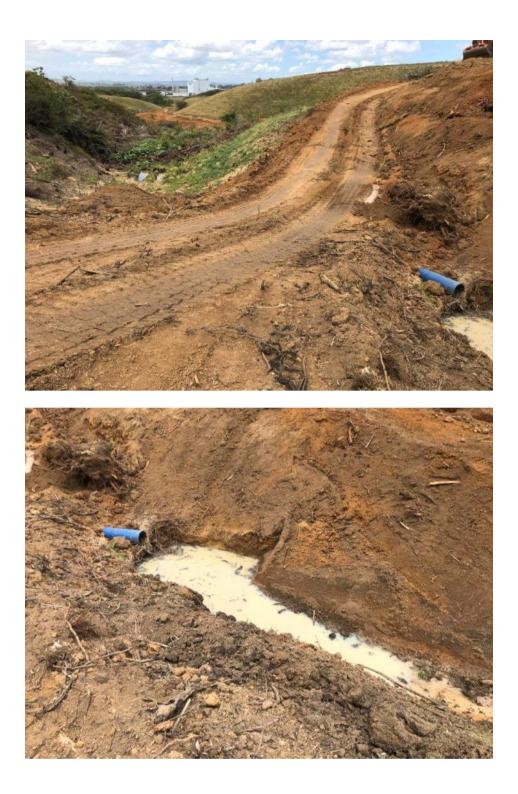
Site Observations and Instructions:

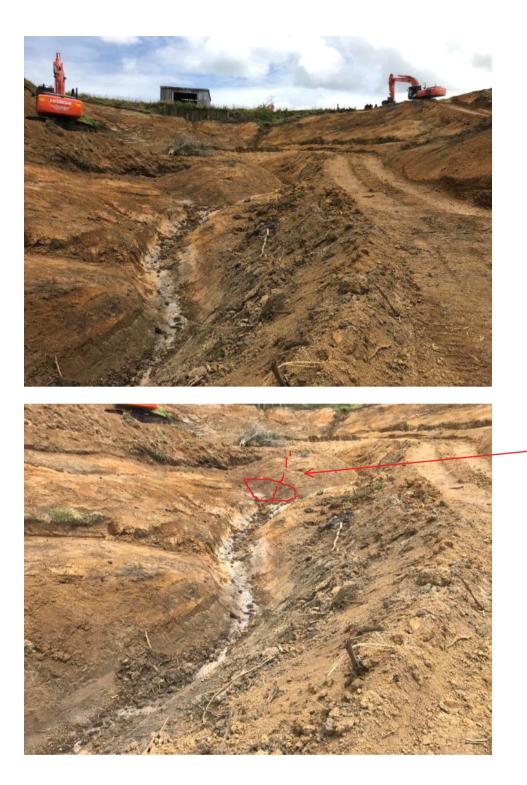
Visited site at the request of Dylan to inspect gully stripping in the area outlined on the plan below (Fill A).

Generally stiff ground was exposed throughout, however, at the base of the gully invert (lower half) water seepages were causes the ground to soften. We recommend an undercut of approx. 0.5m to 1m here prior to placement of the underfill drain. There is some material in place which is going to be undercut as there is seepage coming out below and the underfill drain will be aligned through here.









Groundwater seepage



| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 10/12/19 - 12pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect muckout of lower portion of gully as per yesterdays recommendation. Generally stiff soils exposed at the base of the gully.







Monday, 6 January 2020 10:55 AM

06/01/20

Site Inspection Record

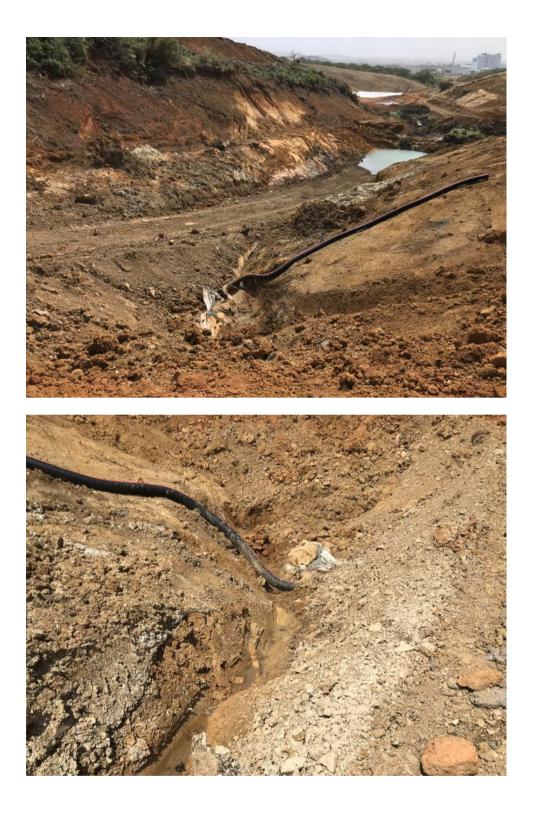
| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 06/01/20 - 10:30am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to catch up on site works within the REB1 fill area. Contractors have filled up the areas sighted prior to the Christmas break. They will send through photos showing the removal of soft soils within the gully inverts.

Contractors plan to extend the underfill drainage so an inspection later this week is likely.







Friday, 10 January 2020 2:12 PM

10/02/20

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 10/01/20 - 2pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect gully muckout in Fill A. Stiff to hard natural ground observed throughout.





13/01/20 Monday, 13 January 2020 3:28 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 13/01/20 |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

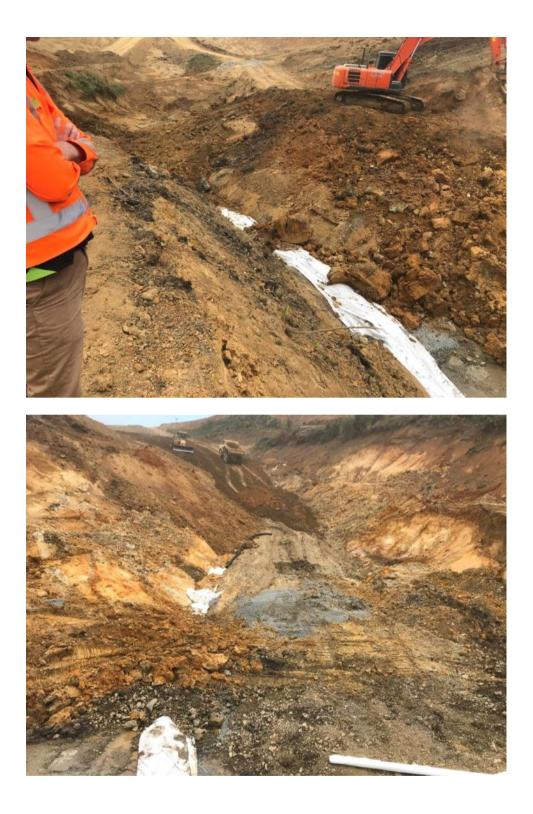
Site Observations and Instructions:

Visited site witch Chris to observe general site earthworks.

Underfill drain has been placed within Fill A gully observed during previous inspection.

Elsewhere, the site is mostly in the same condition as our previous inspections.





| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 15/01/20 - 8:45am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to catch up around the shear key in Fill A. Dylan was asking about drainage / outlet requirements. Confirmed that that underfill drains are to run beneath the MSE fill and the shear key.

21/01/20 Tuesday, 21 January 2020 2:14 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 21/01/20 - 1:30pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited old slip site with Dylan and discussed benching the slip debris out.



| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 22/01/20 - 2pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site with Chris to meet with Dylan to discuss remediation of the slip that we inspected yesterday.

Advised Dylan that the slip should be fully benched out to the natural ground (i.e. removing all the slip debris) at which point we can observe and provide further recommendations. This will likely be to recommend a series of benched drains running parallel along the slope with one or two collector drains running along the extend of the slip area. We also advised that the lateral drains be installed within the vicinity of the existing chimney drain as per our design (this was never done during the earlier earthworks stage).

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 22/01/20 - 2pm |
| Author: | CE + KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

CE - photos from my visit below. They appear to be progressing with REB1 pre construction work (i.e. filling in gullies upslope of it and remediating slip on site of Stage 4 which will be filled as part of REB1.



REB1 area







REB1





Monday, 3 February 2020 3:25 PM

03/02/20

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 03/02/20 - 3pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect first portion of shear key excavation for REB1 / Fill A. Shear key excavated to 5m depth (which involved using a rock ripper). Advised contractors to excavate at least 0.5m into hard or until it can't be removed with a rock bucket. Also advised a plateau ready be carried out in the SPR when fill compaction commences.

Wednesday, 5 February 2020 2:39 PM

05/02/20

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 06/02/20 |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Inspected next section of shear key undercut. Hard materials exposed at base as per previous sighting.









12/02/20

Wednesday, 12 February 2020 1:51 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 12/02/20 - 1:30pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Shear key excavations proving hard.





19/02/20

Wednesday, 19 February 2020 2:19 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 19/02/20 - 1pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Shear key excavations completed. Competent hard ground exposed throughout. Plateau test being undertaken while we were on site.









26/02/20

Wednesday, 26 February 2020 10:29 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 26/02/20 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect top two benches into the remediated slip adjacent to REB1. Benches have been excavated into the natural ground and the contractors have further a small trench along the rear face to installed drainage. Attached below is their proposed plan.

Shear key filling still coming up. Photo below.













Thursday, 27 February 2020 10:29 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| | 27/02/20 - 10:30am |
| | |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Slip remediation works are continuing and they are down to the final bench. Drainage has been installed on the top two benches, with a collector also placed down to the second bench. The material on the bottom bench is a stiff, wet alluvium. Recommended to Dylan that the batter faces be supported by backfilling or temporary face support and also be covered with polythene if they are to remain open.











| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 06/03/20 - 11am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to check in on shear key. The contractors are excavating the 2m beyond the fill batter now. Will return to site with Chris early next week to REB requirements.

Also checked outlets for Hitchen 11, 12, 14 GCR. All outlets correctly formed with concrete rip rap.

















| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 10/03/20 - 2:30pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site with Chris to discuss REB1 backfill, benching and grid requirements with Dylan, Mitchell and Miles. Discussed that all benches need to be at least 2m below original ground level to key in the fill batter

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 16/03/20 - 10am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited sure at the request of Dylan to inspect first gully muckout for Fill B. Generally stiff ground is exposed at the base of the gully. The contractor was advised to strip wet/softer ground where encountered.

Also discussed installing another underfill drain up the gully branches as there has been some groundwater seepage / slippage observed which the contractor has cleared out prior to our visit. Advised we can look at this which stripping operations commence in this area.









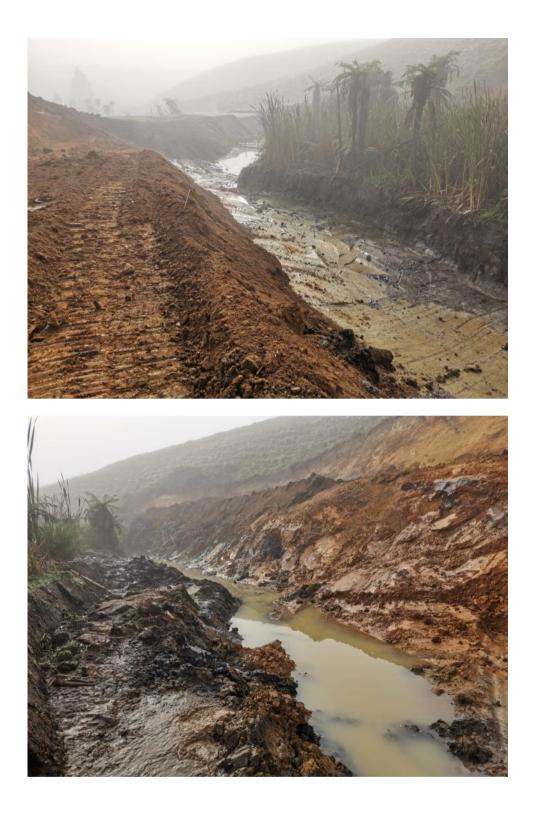
| | SITE INSPECTION RECOR | D |
|---|--|---|
| | Project no. and name: Jeol13 Hitchen Block | Date & Time: 29.4.20 |
| | Client/contractors present: Dylan (Dines) | Lander staff: 1cm SV no: 502396 |
| | Plant operating: 12207 TXLCUDER | |
| | Inspection for (<i>tick all that apply</i>): Shallow Foundations Pile Foundations Site Mee Subgrade Other | |
| | Observations/Notes: | |
| | -Visited site at the request of Rylan to inspect gully muchaut for REB 2 undefill obtain. | ter key beching |
| | -Generally shift (hother) Soils exposed at bese of sully. Freesance AZ soils. | the terre |
| | - Contrain also eduised that seeper groundwater Seeperges were also noted in source place Coming from the sully flooks. | |
| | Fully wrepped underfill drea a beneath engineer | |
| | | colled a |
| | | 1 6 |
| | | Sulus fill previous Creaked Kanuskauts |
| | | H. |
| | Discussions/Recommendations given: | |
| - | Preasumendy some of the softer / himer sols | be shipped beels |
| | -Other price to please undefill drain. Also recommended that localised seerings b collecter undefill drains where observal main undefill guilty altrin. | and compacted into |
| | main water in group of the | |
| - | | 1 |















13/03/20

Wednesday, 13 May 2020 12:25 PM

Site Inspection Record

Project # & Name: J00113 - Hitchen Block

| Project no. and name: Joza13 - Hitchen Black | Date & Time: 13/5/20 |
|---|---|
| Client/contractors present: | Lander staff: Km |
| Dylan (Dires) | SV no: |
| Plant operating: | |
| Inspection for (tick all that apply): Shallow Foundations Pile Foundation Subgrade Retaining Wall | ns Site Meeting Other <u>FEB1</u> Shear Key |
| Observations/Notes: | |
| -Visited FEB I area at have filled the Shar K | the request of Quan. Canbed eg to find height with SPR. |
| mext plas are to install | the latural drains perpindular to |
| The stope face and they , blanket drein. | -ill ran connect into the rea |
| + Next the initial lager | of srid will be placed and |
| GAP65 will be used for th | te first by tith |
| | |
| | m site (phone) issues about how is well better starts to ark awa |
| from the both face. Ch | |
| | he contact has been adviced to |
| overlap the grid by a m | innorm of 200mm and 10kg. |
| extre layers if and what | the grid separates (doesn't over |
| I they don't want to ve | |
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15/05/20 Friday, 15 May 2020 4:26 PM

Site Inspection Record

| Project no. and name: | Date & Time: |
|--|--|
| 200113 - Hitchen Block Client/contractors present: | 15/5/20 - 2:30pm Lander staff: 2m SV no: |
| Plant operating: | |
| Inspection for (<i>tick all that apply</i>): Shallow Foundations Pile Foundations Subgrade Retaining Wall | Site Meeting Other REB 2 Sheer Key |
| Observations/Notes: - Virted site to observe site e Competitue of the REBI Shear lea - Lipk underfill drains fully wrepped drainage assingit sighted at ba faces de the shear leag. - Converty rollers are competing GA first Im of the VZEVB bette. - Will refurn to site with CE geogrid placement. | 5. I and bockfilled with oth the fort and rea 1965, which comprises the |

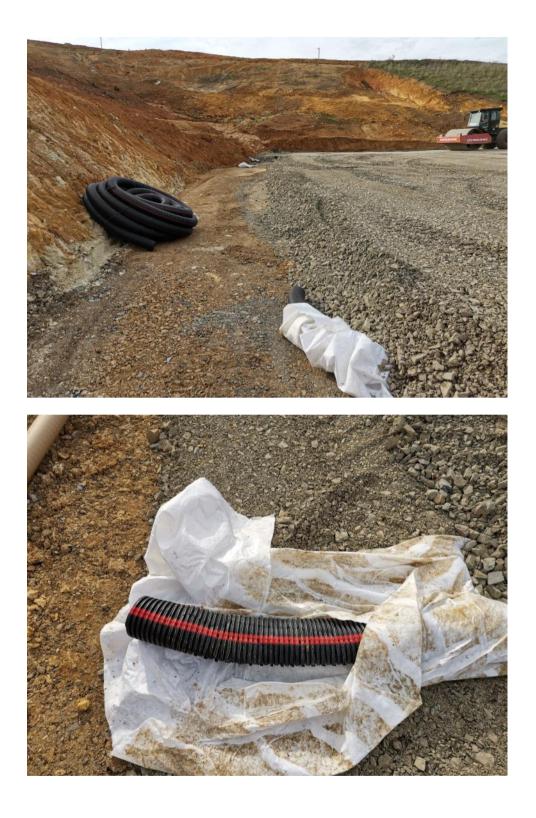














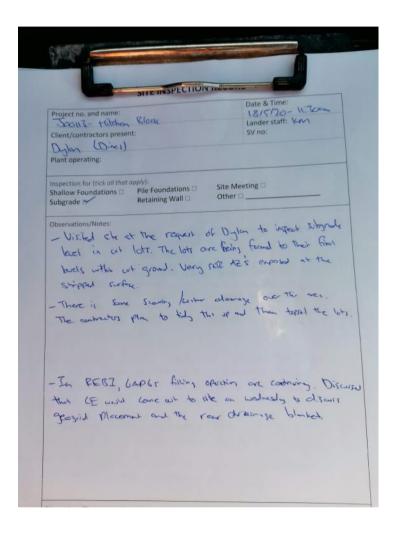




18/05/20 Monday, 18 May 2020 3:31 PM

Site Inspection Record

Project # & Name: J00113 - Hitchen Block





















p. i





21/05/20

Thursday, 21 May 2020 9:59 AM

Site Inspection Record

Project # & Name: J00113 - Hitchen Block

Site Inspection Record

| Project: | J00463 - 125B Murphys Rd |
|------------------|--------------------------|
| Date & Time: | 21/05/20, 9am |
| Author: | KM / CE |
| Plant Operating: | |
| Weather: | Fine |

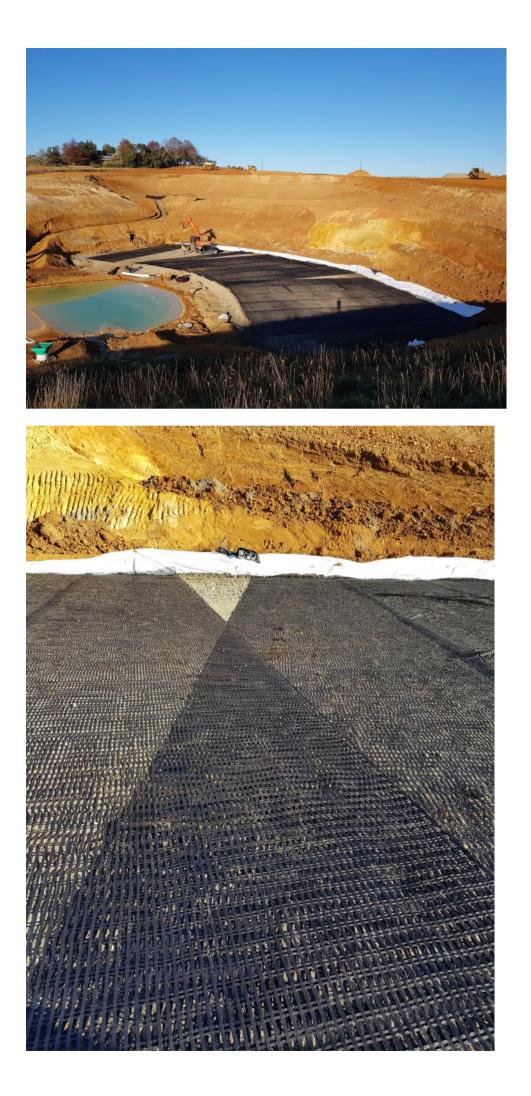
Site Observations and Instructions:

Meeting with Dylan to observe second layer of primary grid on REB1. They are having issues with grid coverage as the wall goes round the curve. Have queried this with Michael Sorrenson and will get him to comment on the best way to fix this issue. Meeting with Cirtex set up for tomorrow morning to discuss this, no more grid to be placed until issue resolved.









Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 22/05/20 9am |
| Author: | CE/ KM |
| Plant Operating: | |
| Weather: | Fine |

Site Observations and Instructions:

Meet with Dylan and Cirtex (Paul Lombard) to discuss placement of geogrid for REB 1

The contractor had re-laid the primary grid layer.

Discussed the requirements for the grid as follows:

- 1. On straight sections of slope maintain a 200mm to 300mm overlap
- 2. On curved sections, increase the overlap at the face to up to 1m and minimise the 'wedge' of area at the rear of the reinforced section where there is no grid covering to less than 1m
- 3. On the next lift of grid (800mm) the areas where 'wedges' were are to be covered. A CAN to be sent on this matter.



4.

22/05/20 Friday, 22 May 2020 11:28 AM

Project # & Name: J00113 - Hitchen Block

Site Inspection Record

Joons - Hitchen Plack 22/05/20 - 8:30cm KMICE - Visited site with CE and met with Paul of Circle, and Dylan and state from Dines to discuss geogrid placement in the REBI better - Paul drivered that the grid needed to be perpeolide to the face and that the gost between grids at the rear of the sharkey could be up to In making with my of these gass being suncycl and the caracters ennous, that these gaps are torened during the next tight by of primary goid - In coursel Dections, the contractors will be within helf lengths of grid (pureller to the thick should) to try ad reduce the gop and minimic any wastege of grid.

27/05/20 Wednesday, 27 May 2020 12:51 p.m.

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 27/05/20 |
| Author: | CE |
| Plant Operating: | |
| Weather: | Showers |

Site Observations and Instructions:

Stopped by to observe construction of REB1. See photos below. Grid layer appears to have been placed as per our directions last week and is now being overlaid with hardfill.



24/06/20

Wednesday, 24 June 2020 11:39 a.m.

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 24/06/20 10am |
| Author: | CE |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Quick stop by to see progress of REB1. Looks like they have stopped due to the weather.



30/07/20 - Stage 12D

Thursday, 30 July 2020 3:07 p.m.

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 30/07/20 |
| Author: | тт |
| Plant Operating: | |
| Weather: | Fine |

Site Observations and Instructions:

Observed muck out of portion of gully circled below. Decant to be formed here to allow further earthworks up slope within gully. Muck out looked suitable with benches to be formed 2 compactors width in flanks to lock this fill batter in. Batter only temporary until next season when earthworks occur downslope. Underfill drain to be placed through this area (discussed specs which are consistent with other drains across site) that will lead into the earthworks upslope and eventually downslope as well. Fill materials to cut from cut area above which looked fine to use. Testing to take place every 1m lift. Contractor to get in touch when gully upslope is being stripped.













04/08/20 - Stage 12D

Tuesday, 4 August 2020 8:15 AM

Site Inspection Record

Project # & Name: J00113 - Hitchen Block

Site Observations and Instructions:

| JOOUNS - H.F. | | 4-08-20 |
|--|--|----------------------------|
| Client/contractors pres | sent: | Lander staff: Km SV no: |
| Plant operating: | | |
| 12 excivity, 1x | | |
| Inspection for (tick all th Shallow Foundations T Subgrade | nat apply): Pile Foundations Retaining Wall | Site Meeting Other |
| Observations/Discussion | ns: | |
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| | 1 | |
| Inspection Result: | Pass Tail | |



08/09/20 - REB1

Tuesday, 8 September 2020 11:28 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 08/09/20 - 11:15am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

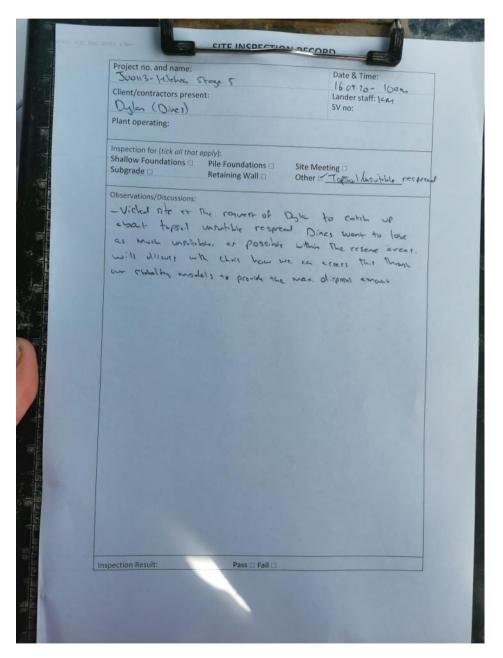




Jouris - Hitchen stoges 08.09, 20 - 11:15cm Km -Visited site to catch up on earthwardy at Hitchen Black - Causht of ut Trever and discussed earthwarter. Dires have recently started Pleaks fill in RER I, with terms ongoing with bestechnick - Converting enabling works are underway to extend your free of better to allow a compadu to fit against the real balls liese. - West loger of grid is planned for tomorrow and we cull referre to

Site Inspection Record

Project # & Name: J00113 - Hitchen Block



Site Inspection Record

Project # & Name: J00113 - Hitchen Block

| Droite | |
|--|---------------------------------|
| Project no. and name: Doo113- Hitchan Black | Date & Time: 7.10.20 |
| Client/contractors present: | Lander staff: Kint LE SV no: |
| Plant operating: | |
| Inspection for (<i>tick all that apply</i>): Shallow Foundations Dile Foundations Subgrade Retaining Wall | Site Meeting |
| Observations/Discussions: -Viriled Site with Chris to Alister From Dires. | mar with Dyle, withher and |
| - Discussed requirement for d REB better, with clay fill be the 2m toe key. No benched these will probably be installed | cing one to use for filling in |
| -Discussed requirement for fill Im 172 and also for ensures. to observe site worky shallong to | tering to be close every |
| Roads are amoney being G areas. | |
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19/10/20 Monday, 19 October 2020 9:39 AM

Site Inspection Record

Project # & Name: J00113 - Hitchen Block

| _ | | CITE HOI LC | HONTALCOI | | |
|--------|--|---|---|---|--|
| | Project no. and name: Dooll 3 - Hitchen | Block | | Date & Time: | I GN |
| | Client/contractors preser | nt: | | Lander staff: Y | ~ |
| | Plant operating: | | | | |
| | Inspection for (<i>tick all that</i> Shallow Foundations Subgrade | r apply): Pile Foundations Retaining Wall 🗆 | | ting Draining | _ |
| | Observations/Discussions: | | | | |
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Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 23/10/20 - 9am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to observe earthworks operations in REB1. Clay fill is progressing and they had just covered up a layer of grid at the time of our inspection. Batter slope appears to be approx. 1(v) in 1(h) as per the design.









28/10/20 - REB1 / 12D

Wednesday, 28 October 2020 1:38 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 28/10/20 - 10am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to observe earthworks in REB1. Contractors had finished fill placement ready for the next layer of primary geogrid placement. The fill batter is now entering zone 2 and SG500 grid is being used for these portions of the batter as per our design.

Rear drain blanket trench was open and the top of the blanket and metal backfill was present, as well as the rear of the grid in approx. 0.4m layers.

Earthworks are also commencing in the large slip area at the far end of the site. Silt controls are currently being formed for these works prior to stripping operations commencing.

In Stage 12D, earthworks filling operations are progressing, and the underfill drain is likely to be extended in the next week or two.























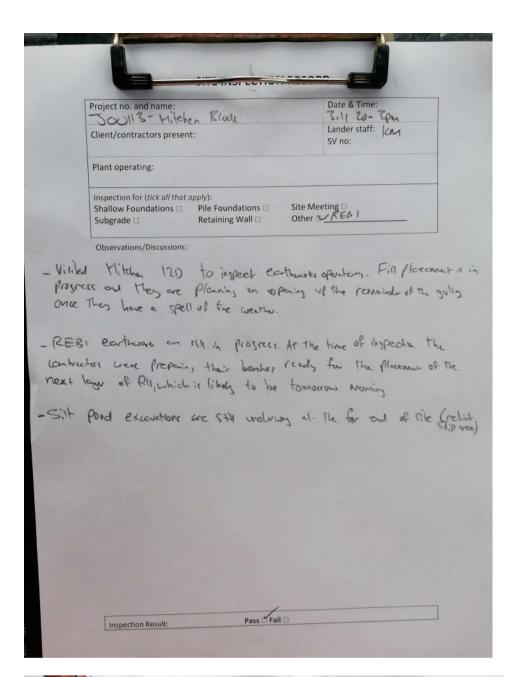
03/11/20 Tuesday, 3 November 2020 6:27 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|









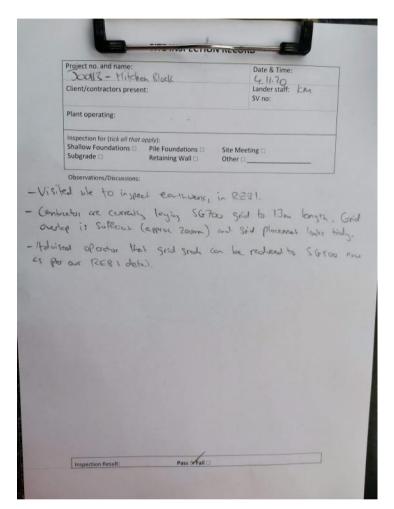


04/11/20 - REB1

Wednesday, 4 November 2020 2:10 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|---------------------------|------------------------|
| r roject <i>n</i> & Nume. | JOOTTO THICHCHENDIOCK |











13/11/20 Friday, 13 November 2020 1:04 p.m.

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 13/11/20, 1pm |
| Author: | CE |
| Plant Operating: | |
| Weather: | Fine |

Site Observations and Instructions:

Works have re-started on REB1 after the wet weather. Contractor was about to commence placement of another grid layer. Clay material still being dried out/ conditioned.





16/11/20 - AM

Monday, 16 November 2020 3:07 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 16/11/20 - 11:30am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to observe earthworks operations in REB1. The REB is now up to chainage 49.5 and a layer of intermediate grid had just been placed and covered by clay. The next bench was being prepared at the time of inspection. Contractor informed that the next grid would likely be placed tomorrow morning so will do another inspection of the grid then.

Was also informed that the lateral drain was trenched and installed mid-last week, for which we received the as-builts. Advised we should try inspect the next section of drainage installation if possible.







16/11/20 - PM - Stage 12D

Monday, 16 November 2020 3:19 PM

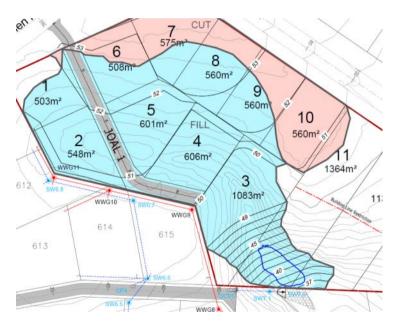
Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 16/11/20 - 1:30pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

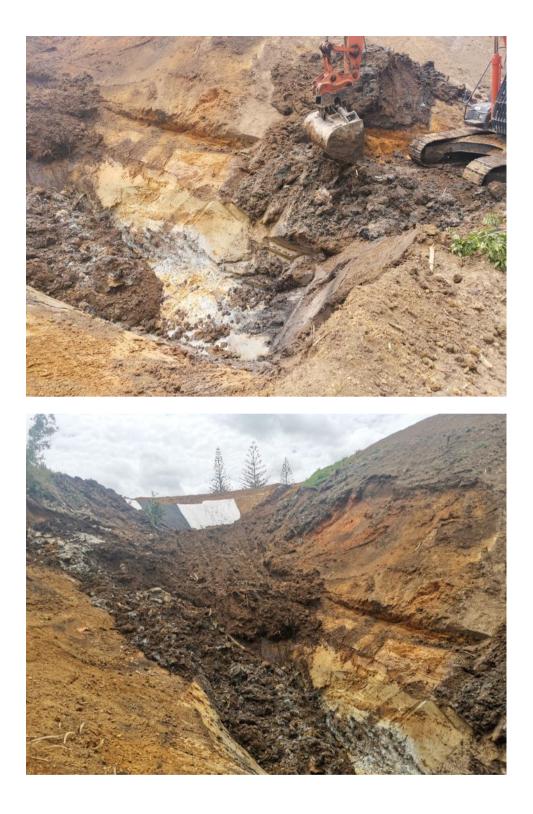
Visited site to observe the gully muckout within Stage 12D. The muckout is commencing from the lower portion of the gully and will tie in with the existing underfill drain once some engineered fill has been placed in the area to allow digger access. Approx. area sighted shown in blue annotation below. Will return to site tomorrow to observe progress here.

Also inspected the removal the stockpile materials within Fill D. Very stiff, orange/brown ash soils were exposed consistent with the stripped soils previously sighted within the adjacent cut areas.











Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 17/11/20 - 12:30pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to observer general earthworks operations. Within Stage 12D the base of the gully had been mucked out slightly further upslope and the drain coil and bidim cloth were in place at the base of the muckout. SPR was being placed over the drain to be compacted.

Within REB1 clay fill was still being compacted. It appeared that the geogrid had already been placed.











18/11/20 - REB2

Wednesday, 18 November 2020 3:46 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 18/11/20 - 12:30pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect the undercut for the shear key in REB2. The contractors had cut down approx 4m from original ground level and had excavated down to hard orange silts.

Checked these works against our model which confirmed that the approximate depths and materials were consistent with our model. Informed Dylan that shear key operations could continue as planned, but 2 or 3 trial pits will be required in the base of the shear key once it has been opened up to further validate the ground conditions beneath the base of the shear key.









20/11/20 - REB1&2, Stage 12D

Friday, 20 November 2020 12:41 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 20/11/20 - 11am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Stage 10:

Visited site to inspect that underfill drain outlets have been installed. Both drains were sighted and have been formed as scruffy dome or rip-rap channel outlets.

<u>REB2:</u>

Observed shear key operations in this gully. The shear key looked that same as our previous inpsection, and the gully is still being mucked out for the shear key. From what we can see the base appears hard, however, we waven't been able to test the base of the shear key yet due to the muckout still needing to progress.

Informed the contractors that we will inpsect once a larger section of the shear key has been opened up, and that they will need to create a level surface for the shear key and then bench up the sides of the gully when backfilling with hardfill.

<u>REB1:</u>

Primary geogrid has been laid out to 13m length. Grid layout and overlap is sufficient. The rear fill drain was also being connected and has been sufficiently linked up.

Stage 12D:

Not much progress has been made since our last inspection in this area. The SPR in the base of the gully is still loose and requires compaction. The contractor is planning to bring a pad-foot roller down to this area to start backfilling the gully.





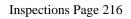














24/11/20 - REB2

Tuesday, 24 November 2020 3:53 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 24/11/20 - 12pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect earthworks operations in REB2 shear key.

The shear key has been opened up slightly more since our last inspection, however, more works are still required to enable proper access into the shear key. Once this has been done we will observe a series of trial pits along the base of the shear key to confirm the ground model.







30/11/20 - Stage 12D

Monday, 30 November 2020 12:58 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 30/11/20 - 9:45am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to inspect earthworks operations in Stage 12D area.

Not much progress has been made since our previous inspection and there is spoil along the gully which is still to be cleared. Also discussed that a toe key is required at the base of the proposed fill batter, which the contractors are proposing to install prior to building the fill batter.

Also discussed the stripping in Lots 611 and 612 which Russell emailed about last week. This area is still covered by a topsoil stockpile. Advised the operator to get Dylan to get us in for an inspection prior to placing fill in this area.





Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 03/12/20 - 1:15pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Trevor to discuss toe key requirements for the Sage 12D fill batter. Discussed that the toe key needs to be embedded 0.5m into very stiff, 100kPa soils, and needs to extend along the face of the batter and be benched up the sides of the fill. Dines are going to use SPR in the base of the toe key and then use clay fill once above the water / underfill drain line. The toe key will likely be ready for us to inspect early next week.

The remained of the gully is still being stripped, and excess spoil is still placed along the gully waiting to be loaded out. There is a vertical cut along the gully flank which is located just below an existing dwelling. Advised this should be backfilled and supported in the short term to provide stability to the dwelling.

Filling has also occurred over lots 612 and 613 from the adjacent GCR stage. We will drill several 2m boreholes here to confirm adequate stripping. Asked Trevor to also organise a fill test in this area for inclusion in our addendum GCR letter.







Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 07/12/20 - 7:30am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Trevor to determine extent of toe key in Stage 12D. Decided that the key should run along the toe of the batter, be 3m wide and at least 0.5m deep into very stiff natural soils. The key will be backfilled with SPR and the underfill drain will then run on top of the key (i.e. along the gully floor) and outlet into the gully below.

The gully has also been stripped and the underfill drain is to be placed next.

Dines has also placed some temporary fill in the cut that was open below the existing house.





08/12/20 - REB2, Stage 12D

Tuesday, 8 December 2020 2:05 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 08/12/20 - 8:45am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect the following:

<u>REB2:</u>

The front/downslope part of the shear key has been excavated to depth, with hard, orange/brown silty soils exposed at the base. A trial pit was excavated with the rock bucket and went to a depth of approximately 2m before it became too hard to excavate further.

A small section of softened soils remains to be removed in part of the base.

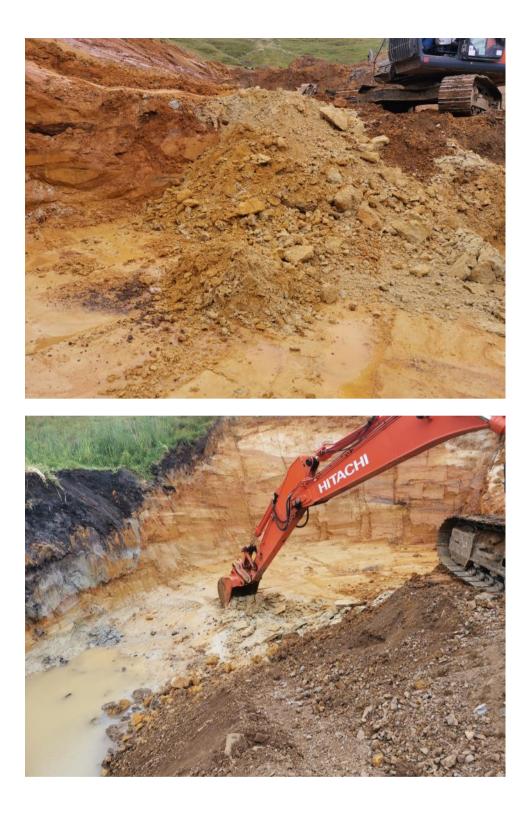
A section of the existing underfill drain behind the shear key has been flattened due to plant tracking over this area. Advised that this section of the underfill drain would need to be chased out and a new drain relayed here. The contractor also had concerns about the soils under the drain being soft, so we will look at this when the gully has been opened up.

<u>12D:</u>

Toe key construction is still in progress. The contractors have excavated down approximately 3m and have encountered slop. Advised them the clean this out and continue to muck out the area to very stiff soils as per our detail.

REB2:







REB1 progress:





12D toe key:





Damaged underfill drain in REB2:





09/12/20 - REB1&2, Stage 12D

Wednesday, 9 December 2020 3:08 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 09/12/20 - 11am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect the following:

- Inspected toe key in Stage 12D. Contractors have excavated to approx. 4m with no improvement in ground conditions. Advised that they could place a 500mm raft of compacted SPR to allow them to start backfilling.
- Inspected shear key excavations in REB2. Hard ground exposed throughout along the base of the key which had been further excavated since yesterday. Advised that backfilling can commence with SPR100 once they have pumped the water out of the excavation. Plateau test from REB1 will still remain relevant.

REB 2 shear key:









Stage 12D:



10/12/20 - Stage 12D

Thursday, 10 December 2020 12:45 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 10/12/20 - 10:30am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to check on toe key works in Stage 12D. The first section of the toe key has been backfilled with hardfill.

Also, drilled at hand auger in Lot 612. Encountered approx. 0.6m of fill overlying very stiff, natural ash soils.







14/12/20 - Welded tuff area

Monday, 14 December 2020 1:39 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 14/12/20 |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect an area of cut which the contractors are struggling to excavate. Dylan was concerned that the material may cause limitation for future end-users / builders. Dines were querying whether an undercut backfilled with certified clay would be a good solution.

On site, the material comprised of a welded tuff. Advised that I would talk to Chris or Shane before providing any advice.



| Client : DFH JOINT VENTURE LIMITED Project Location : HITCHEN ROAD STAGE 5, POKENO | | | | Machine Borehole No. MH01 | | | | | | | | |
|---|---|--------------------------------|-------------|---|--------------|------------|----------------------------|-----------------------------|----------|----------|--|---|
| | Project Location | on: HITCHEN | RUAD STAGE | D, PUK | ENU | Vane Head: | ogged | nu D | 00051 | | eet 2 of | |
| | Job Number: | J00741 | | | | 2007 | KM | | RG | ior . | Start Date Finish Dat | : 19.03.1 e: 19.03.1 |
| h | Borehole mN | mE | | | Ground R.L. | | ter/ | pour | 1 | | 2 2 5 | 1.0000 |
| Stratigraph | Location: Descriptio | n: Refer to site plan | | | Orientation: | vertical | Sroundwater | asin Mel | very | 100 | Sample and Laboratory Test Details | Vane Dial / Sensitivity & SPT |
| Strai | CORE | DESCRIPTION | a second | Depth (m) | DEF | ECTS | Groundwater/ Piezometer | Drilling Method & Casing | Recovery | ROD | Sam Lab | Van Ser |
| | | | | 1 8.5 1 9.0 | | | | OPEN BARREL | 41 | | Sample 9 Distarbed | UTP |
| | encountered medium gr inclusions | avel sized silt carbonace | routs | Piezonel | | uuuuu | | SPT | | 9.0-9.1m | SPT at 9.0-9.46m 7/9/12 N=21 | |
| | | | | E 10.0 | | | annan ann | | 92 | | | |
| | becoming dark grey and | | rown/grey | 10.5 | | | 1111111111 | | SPT | | | UTP SPT at 10.5-10.95 10/10/7 N=17 |
| In | with very thin bed of stiff | sity CLAY | | L | | | | | 76 | | | |
| | | | | 12.0 | | | anan anan | | SPT | | | UTP SPT at 12.0-12.45 9,13,15 N=28 |
| | | | | 113.0 | | | | | 100 | | | |
| | becoming dense | | | - 13.6 - 14.0 | | | | | SPT | | | UTP SPT at 13.5-13.95 9/13/18 N=31 |
| | - | | | 14.5 | | | | | 94 | | | |
| | at 15.0m, becoming med EOB at 15.0m. Target De Piezometer | | leasured in | 15.0 - - - - - - - - - - - - - - - - - - - | | | | | SPT | | | UTP SPT at 15.0-15.45 10/12/14 N=26 |
| 1 | LANDER | Comments: Core loss from 7. | 95m-8.65m | Drilling wa | tor Fal | Sand | - 666665 | s | ndstor | 1111 | Pluto | |



15/12/20 - Stage 12D

Tuesday, 15 December 2020 2:24 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 15/12/20 - 12pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to inspect 12D toe key. Contractor had already filled with SPR to 1.5m without getting us to inspect or getting fill testing so asked them to dig back to subgrade in the centre. Firm soils exposed at the base, however, the toe key will be about 2-3m thick so no undercut was recommended. Contractors said they needed to backfill the area immediately. We advised that they need to give adequate notice for geotech and fill test inspection in this case.

Also, inspected stripping over Lots 612 & 612. Generally very stiff natural ash soils were exposed, however, some topsoil/rubbish was observed which we recommended be undercut to natural ground. Will look at this again tomorrow afternoon with Chris.

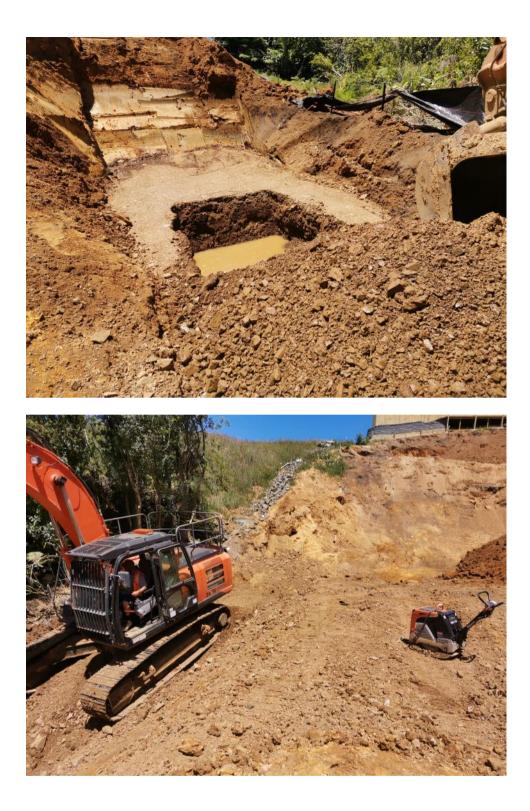


12D topsoil stripping:



12D fill/toe key area:







16/12/20 - REB1&2, welded tuff area, Stage 12D

Wednesday, 16 December 2020 5:56 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 16/12/20 - 1pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site with Chris and Shane for a senior/principals site visit. Observed the following:

- Welded tuff materials are still being cut behind the site office. We need to discuss with Russell options here. Ether the materials can be left in place and the end users will need undercut the materials with bigger diggers etc when doing footings and drainage lines, or the material can be undercut by approx. 0.6-1m and backfilled with certified clay, however, this will raise the expansive class in these lots (which would currently be good ground).
- REB2 shear key excavations. They have filled about 2m and have missed testing again. Advised Dylan to excavate some areas down to 1m for a test prior to placing further fills. Will inform Josh.
- REB1 is near level and is set to be completed in the new year. Will catch up with Trevor on the first week back to organise inspections for the final geogrid layers here to ensure they are placed properly around drainage lines.
- Stage 12D toe key. The contractors failed their previous test here, achieved CIV readings of around 10. They removed the material, discarded the wetter SPR and refilled with a heavier roller. Josh to retest. The model also needs to be rechecked to confirm minimum toe key requirements are adequate.
- Also observed the stripping area in lots 611 and 612. Rubbish/pre-existing fills are still present. Reiterated that these materials be fully undercut. Will return to site tomorrow to observe general earthworks operations.





17/12/20 - Stage 12D

Thursday, 17 December 2020 1:23 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 17/12/20 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Inspected backfill in 12D toe key. Compaction appears to be going well with no obvious deflections noted under a 12T smooth drum roller. The latest fill test passed on both clegg hammer and backscatter testing.

Also observed undercutting of topsoil/pre-existing fill in the top section of Stage 12D. The base of the undercut is generally down to natural ground, although there is some minor topsoil mottling to chase out. The existing fill cut shows a layer of topsoil within the fill placed by Dines which is within 2m of surface level. Advised that this should be undercut and removed from the fill. Will return to site again tomorrow morning to inspect.







18/12/20 - REB1&2, Stage 12D

Friday, 18 December 2020 12:21 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 18/12/20 - 10:45am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Inspected the following at the request of Dylan:

- At top of Stage 12D, topsoil horizon has been chased out and is ready to be filled again.
- At Stage 12D toe key, they have filled up another metre and are waiting for the next test. Hardfill surface appears well compacted visually. Asked Dylan to get the extent of the key surveyed to ensure that the key matches the full extent of the fill batter.
- At REB1, geogrid is being laid out, with grid lengths of around 5m and 14m being recorded as per our design for the top layers.
- At REB2 shear key, contractor has informed that additional testing was undertaken at 1m and 2m fill heights. Also observed stripping to the base of the key in the rear portions. Hard surface encountered here. A trial pit at the base was only able to be excavated approximately 1m with a rock bucket, consistent with the previous test.



Stage 12D toe key:

REB1:



REB 2 shear key:









12D topsoil horizon stripping:





21/12/20 - Stage 12D

Monday, 21 December 2020 2:09 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 21/12/20 - 12:15pm |
| Author: | |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at request of Dylan to observe site works prior to the Christmas break. Not much has progressed since our previous visit. Toe key has been completed and clay fill for the 1(v) in 3(h) batter is now commencing.

Also requested that some more fill be placed to buttress the site cut at the top of the gully flank to ensure stability is maintained to the house.



06/01/21 - REB2, Fill K, welded tuff area

Wednesday, 6 January 2021 4:22 PM

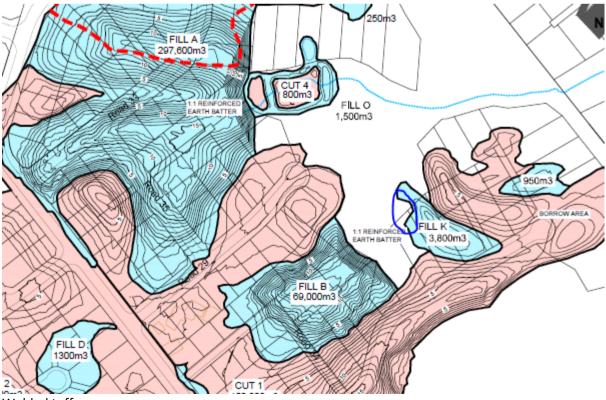
Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 06/01/20 - 2pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Trevor to inspect the following areas:

- REB 2 shear key: Inspected the excavation to the base of the shear key in the rear/southern corner of the key. Hard, orange silts exposed consistent with the previous observations. There is still an area in front of the key which needs to be fully pumped out an backfilled also.
- Extent of works adjacent to Fill K: Approximate area shown in blue on the annotated drawing below. Trevor discussed that the area is to be extended down to the main gully now. Advised that any stripping needs to be down to the stiff natural ground and any mullock or colluvium removed. Further advice to be given once the area is opened up and we can observe ground conditions. Trevor will send some plans through to us for our initial thoughts on what engineering solution may be required.
- Welded tuff area: The contractors have about 1m of further cut required to achieve design subgrade level. Trevor advised that they will survey the welded tuff once they reach this depth so we can advise Dines.



Welded tuff area



Addition to extent of works



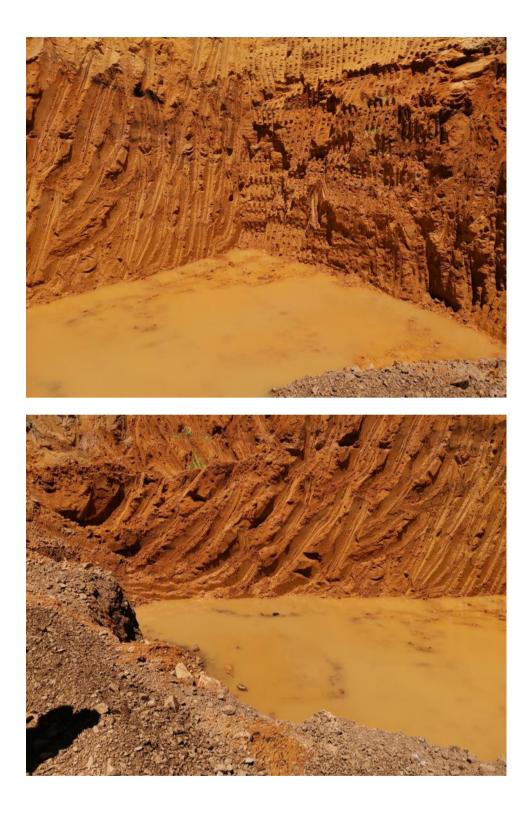




REB2 shear key







12/01/21 - REB2, old slip area

Wednesday, 13 January 2021 12:45 PM

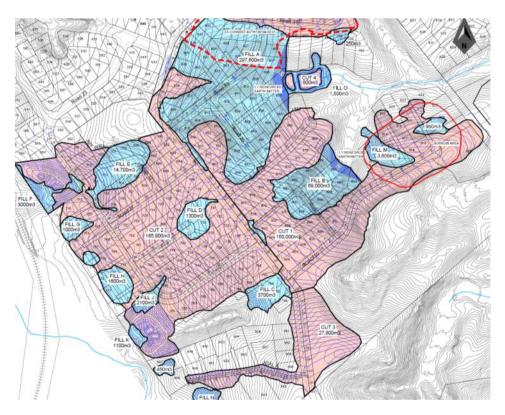
Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 12/01/21 - 2:45pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect stripping for cut 1 in the approximate area outlined in red on the image below. Cut exposed very stiff natural ground. No seepages or soft ground observed though this area. Minor fills to be placed in this area next.

Also inspected REB 2 shear key progress. Backfill of the shear key in progress. Fill testing was being carried out at the time of our inspection of the area beyond the key where the groundwater was being pumped from.















13/01/21 - REB1&2

Wednesday, 13 January 2021 4:31 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 13/01/21 - 2:30pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site with Chris to carry out general inspection of the site.

In REB1, drainage is being installed and the contractors have followed our advice by only cutting the grid perpendicular to the slope face and installation additional perforated drainage in the service trench as an additional measure.

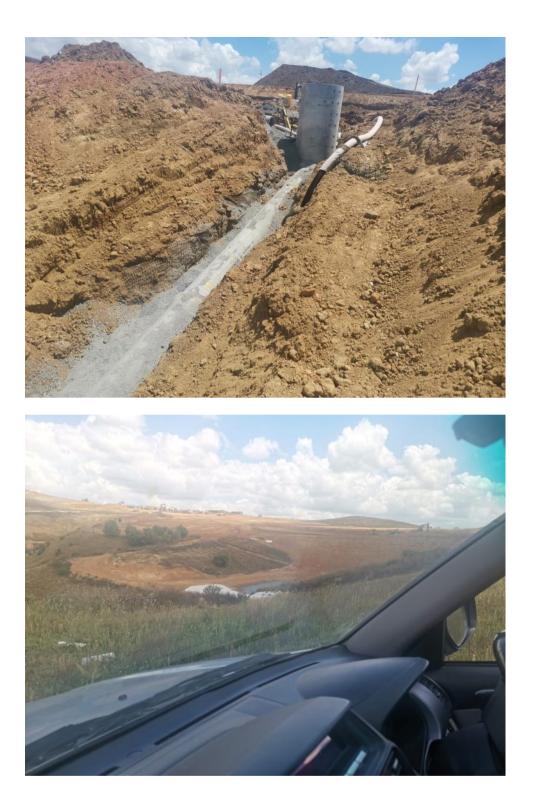
REB 2 shear is continuing with backfill operations.

Observed cut 1 stripping (which was observed with Dylan yesterday). Will have a look at recent reporting and assessments that Matt has done recently with Chris tomorrow to determine if there are any additional requirements here.

















15/01/21 - REB2

Friday, 15 January 2021 2:06 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 15/01/21 - 12:15pm |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect the final section for the base of the REB2 shear key. The shear key was extended down to the hard, orange silty materials previously identified at the base of the shear key. A scratch with diggers rock bucket confirmed hard materials.

On site Mitchell confirmed that 1.4m of SPR hardfill is required until the base of the GAP65 layer at the base of the REB batter.

Mitchell is also planning on the setout for the counterfort drains next week. Advised we will inspect the setout and modify this if required.





18/01/21 - REB1

Monday, 18 January 2021 4:52 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 18/01/21 - 11am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Trevor to catch up on site wide earthworks that occurred while he was on leave last week.

Visited the old slip adjacent to the silt pond below REB1. Dines plan to start remediating this area and placing a minor fill batter. Advised that an underfill drain is required here below the fill. The plan is the bench up the existing feature, ensuring any old slip debris or otherwise unsuitable soils are removed prior to fills being placed. Advised that we should be on hand regularly for inspections while these works are being carried out.

Elsewhere, not much has progressed since last weeks inspections. Some clay is being laid out to dry in the large cut area adjacent to the REB2 shear key.

25/01/21 - REB2, old slip area

Monday, 25 January 2021 12:45 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 25/01/21 - 10:30am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to inspect the following:

- Observed placement of underfill drain within Fill M. Advised contractors that they could remove the top half of the drain as this is above the natural gully feature.
- Contractors wanted to know whether they should put an underfill drain in the eastern part of Fill M. 1.5m of clay capping is proposed. Advised the contractors to trench this in 0.5m to enusre a 2m cover is maintained to avoid any tags on the lots (920, 926, 927).
- Observed stripping operations below proposed Fill C. The topsoil strip revealed hard ash soils and tuff materials consistent with the materials observed in our trial pits from the GIR. Advised that an underfill drain should be trenched in here similar to below Fill M.
- Observed muckout of organic and soft alluvium materials below the underfill drainage line upslope of the REB2 shear key. The contractor had excavated down to the hard materials exposed at the base of the shear key. Advised him that this could be reduced to the base of any major organic layers, provided that the inorganic soils are competent enough the place and compact engineered certified fills upon. Will return in a day or two to observe progress.









26/01/21 - REB2

Tuesday, 26 January 2021 1:44 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 26/01/21 - 11am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect REB2 earthworks operations.

The undercut of organic alluvium has now been backfilled with SPR and the undercut had extended back into the gully to intercept the undisturbed/intact underfill drain. A drainage line has been trenched through the centre of the shear key to allow this drain to outlet into the main gully. The contractors plan to place two coils in this trench for redundancy.





27/01/21 - REB2, Stage 12D

Wednesday, 27 January 2021 2:03 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 27/01/21 - 10:15am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to inspect general earthworks operations. Observations and recommendations as follows:

- Within Stage 12D the 1(v) in 3(h) batter has been formed to final height and has been trimmed and topsoiled.
- Stripping has commenced up the flanks of the gully in REB2 for preparation of the fill batter placement. Recommended the installation of 3 additional underfill drains in areas of seepage, two up the eastern flank and one up the southern (rear) which will connect into the existing underfill drain.
- Discussed that benching into the flanks will require an average bench depth of 2m which Dines have already programmed up.
- Will meet on Friday with Cirtex and Chris to discuss topsoiling the face of REB1 as the contractors are having difficulty getting the topsoil to stay in place.







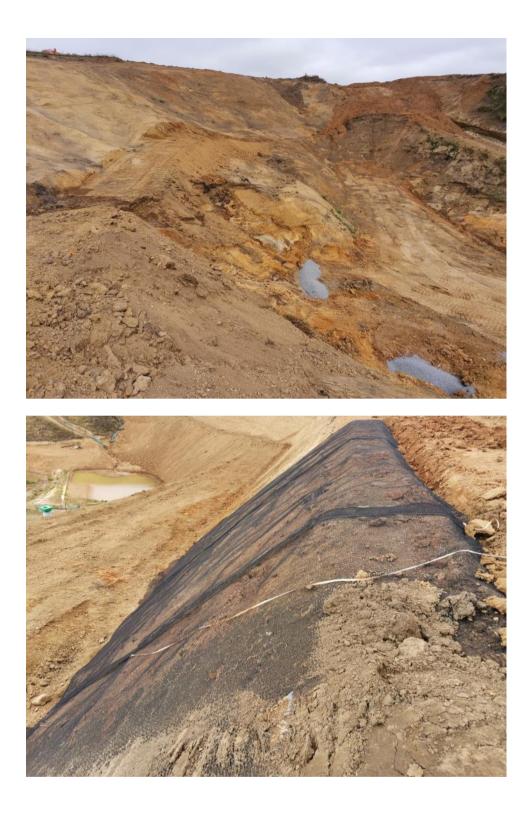












29/01/21 - REB1&2

Friday, January 29, 2021 5:27 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 29/01/21 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site with Chris to inspect the construction of the REB1 batter. Discussed that it would be difficult to get a 150mm layer of topsoil to bind to the face / the fortrak grid system. Will organise for Cirtex to inspect and provide advice.

At REB 2, construction of the shear key is almost complete and several underfill drains have been installed as per our recommendations.

Also inspected final works for lots 611 and 612, adjacent to stage 12D. Both lots have been topsoiled, with 100mm of topsoil measured in both lots.







05/02/21 - REB 1

Friday, 5 February 2021 2:52 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 05/02/21 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to meet with Grant (Cirtex) and Dylan to discuss the facing for REB1.

Grant advised the product Dines are using is not the best for such a steep slope, however, he is going to discuss options with others at Cirtex and provide further advice as to how the facing mat should be installed and how they are joined down the slope.

Also inspected REB2 - the 1m GAP65 layer has been placed and tested via clegg hammer, and the 19m long layer of grid is laid over top as per our design recommendations.







Advice email from Cirtex:



Kyle Meffan

| From: Sent: | Grant Suckling Cirtex Industries Ltd <grant@cirtex.co.nz> Tuesday, 9 February 2021 5:55 AM</grant@cirtex.co.nz> |
|----------------|---|
| То: | Kyle Meffan |
| Cc: | DylanH@dinesgroup.co.nz; trevorm@dinesgroup.co.nz |
| Subject: | CIRTEX - Hitchen Road Topsoil Batter |

Hello Kyle,

Good to meet with you onsite last week. The Fortrac 3D on the first cut looks good, some additional small pins on overlaps may help hold he Fortrac in place while topsoil is placed. The purpose of the Frotrac is to initially help retain top soil then long term is to reinforce the root zone of the vegetation to stop vegetation sliding off the face. The top soil being currently used is very dry and powdery so very little soil retention is being achieved. As discussed there is an area at one end where wetter soil had been packed on with excavator which may help achieve a better outcome with the vegetation long term.

As to placing he next runs of Fortrac 3D we would suggest the Fortrac is pinned in the 300mm x 300mm V Trench already cut with Platipus Anchors or 400mm DuraMat Pins. The advantage of the Platipus Anchors being higher pull out resistance and also the anchor plate that would hold the Fortrac better.

https://cirtexcivil.co.nz/product/platipus-args-anchors/ https://cirtexcivil.co.nz/product/duramat-pins/

Cirtex does not get involved with hydro-seeding however Jon Tomsett from Cirtex has had a lot of experience in the past and has some suggestions:

Grant,

Assuming the goal on these slopes is long term grass establishment I would recommend the following: Engage a reputable Hydroseeding contractor to tailor an application with a broad spectrum seed mix inclusive of Kikuyu for long term establishment and stability. The immediate application ideally will include a heavy application and tackifier that will assist with bonding the lose topsoil providing some protection against a significant rainfall event.

I would then suggest applying a tackified hay mulch over the top to retain moisture and encourage germination and establishment in these drier months. A Hessian overlay could be considered but a Hay mulch would be my preference if a contractor is comfortable they can achieve coverage.

Happy to come to site or discuss further if required.

Do not hesitate to contact me if you have any questions.

Kind Regards,

Grant Suckling Technical Sales

T 0800 247 839 W www.cirtexcivil.co.nz





All information and suggestions in this email are defined as 'Technical Support' as described in the Cirtex Industries Ltd **Project Design Options** document unless otherwise stated. To request this document please contact us. The information in this email and any attached files is confidential and/or legally privileged. It is intended for the addressee only. Access to this email by anyone else is not authorised. Any distribution, disclosure, copying out or use of this communication by anyone else than the addressee is prohibited and may be unlawful. Should you receive this email by mistake please notify the sender immediately and then delete this email from your system. All information and advice is submitted subject to Cirtex Industries Ltd Standard <u>Terms and Conditions</u>, available upon request. **Please consider the environment before printing this email!**

09/02/21 - REB2

Tuesday, 9 February 2021 4:46 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 09/02/21 - 9:30am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site to observe earthworks progress over the weekend, however, not much had been done since last week.

There is still one more layer of GAP65 to place in REB2 prior to clay filling starting.





11/02/21 - REB1&2, welded tuff area

Thursday, 11 February 2021 1:02 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 11/02/21 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Visited site at the request of Dylan to inspect site works. The following observations were made:

- Within the area of welded tuff, Dines have decided to change the subgrade levels and build up the area with terraced platforms utilising clay fills. Competent, hard subgrade exposed at base suitable to place fill upon.
- Within REB2, the 1m GAP65 layer is now complete and clay filling is about to commence. The rear blanked drain is being installed, however, the main underfill drain has been severed and the contractors were advised to repair this prior to backfilling.
- At the REB1 wall face, the contractors are still having difficulty placing topsoil along the facing. Advised the contractor to keep talking to Cirtex and their hydroseeders for advice as necessary.
- The old slip above the REB1 silt pond is about to be backfilled. Advised contractors to trench in an underfill drain prior to benching into the slope and filling. The underfill drain will outlet at the same point at the REB underfill drains, but with a different outlet point.

























19/02/21 - REB2

Friday, 19 February 2021 8:42 AM

Site Inspection Record

Project # & Name: J00113 - Hitchen Block

Site Observations and Instructions:

Joolis - Hitchen Block 18/2/21 Km 1:30pm - observed contraction of fills (class fille) within REB2 - Adequite benching into sully flacks observed.



24/02/21 - REB1&2

Wednesday, 24 February 2021 12:11 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 24/02/21 - 10am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

- Fills are progressing in REB2. All 4x underfill drains can still be observed at the toe of the batter.
- Slip remediation works progressing below DEB10 (adjacent to REB1 silt pond). Underfill drain observed below current fill batter as per our recommendations.



















26/02/21 - REB2

Friday, 26 February 2021 4:42 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 26/02/21 - 2pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

- Observed progress of REB2 batter fill operations.
- Geogrid laid out to approx. 19m length as per our design. A gap remains in the centre section which is to be picked up and covered at the next primary grid lift.
- Fill compaction results have generally been passing, after a series of failed fill tests a few weeks ago (failed on air voids).
- Adequate benching exposed along the gully flanks.







08/03/21 - REB2, Stage 12D

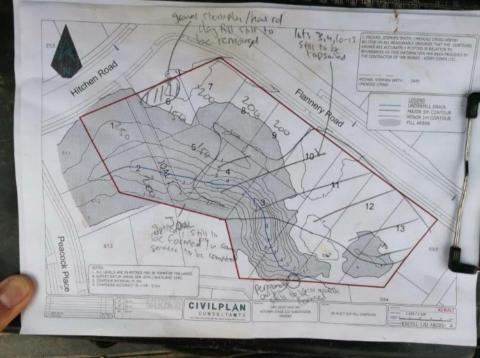
Monday, 8 March 2021 1:08 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 08/03/21 - 10:15am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

- REB2 fill placement is progressing. 2m face tightening grid currently exposed and waiting for the next layer of clay fill.
- At stage 12D carried out topsoil measurements. The following still needs to be addressed:
 - No permanent concrete outlet formed for underfill drain at toe of batter.
 - Lots 3,4, 10-13 are still to be topsoiled
 - $\circ~$ Stockpile of gravel / loose clay present adjacent to JOAL within lot 6, to be cleared and topsoiled









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REB2:





12/03/21 - REB1, REB2

Friday, 12 March 2021 4:29 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 12/03/21 - 1pm |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

- Filling is now complete in the old slip adjacent to the stormwater pond below REB1. The area is now being trimmed and topsoiling is planned soon.
- Inspected filling operations in REB2. The REB is now in the middle grid area and 10m lengths of SG500 grid were recorded throughout.
- Cut progressing above REB2.

















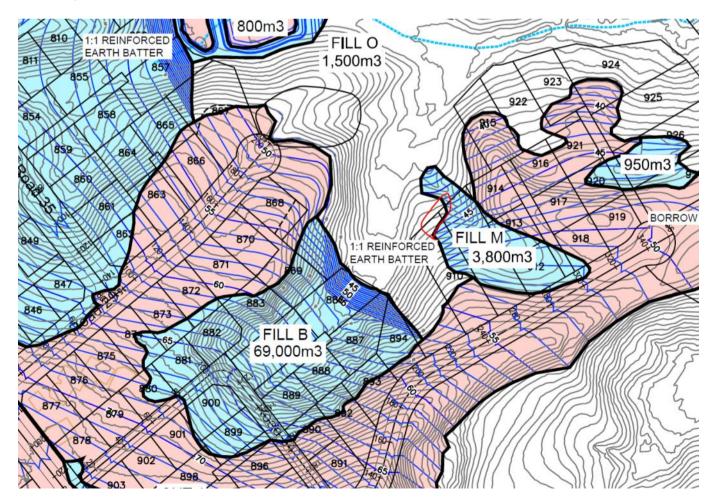
24/03/21 - Fill M&O, REB2,

Wednesday, 24 March 2021 3:25 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 24/03/21 - 9am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

- Inspected topsoil stripping in Fill O. Approximate location shown by black annotation. Very stiff ash soils exposed throughout. Recommended that an underfill drain to be placed along the existing overland flowpath here.
- Observed progress of REB 2 fill works. Approx. 3.5m of fill left to place before trimming of the 1(v) in 1(h) batter begins. Rear face drain is now complete, with the final section of this drain being installed today.
- Observed permanent stormwater pond below Fill M, approximate position shown in red below. Contractors had mucked out the pond and backfilled without notifying us. Several trial pits dug in this area which identified SPR to beyond 3m and at least 1.5-2m below the adjacent fully invert.































25/03/21 - Trevors photos of REB2 / Fill M pond

Thursday, 25 March 2021 8:48 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | |
| Author: | |
| Plant Operating: | |
| Weather: | |









26/03/21 - Fill M&O, REB2

Friday, 26 March 2021 12:42 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 26/03/21 - |
| Author: | |
| Plant Operating: | |
| Weather: | |

- Filling operations continuing in REB 2. Another layer of primary grid was in the process of being laid out. Grid measured at 10m as per the design.
- Trial pits in the Fill M stormwater pond have been partially backfilled with SPR and fill testing has come back indicating adequate compaction and density. The contractors intend to backfill the remainder of the pond with at least 1m of clay as per our advice. Pond construction will be finalised next season.
- Fill O underfill drain has not been installed yet due to bad weather forecast next week.











07/04/21 - REB1, REB2

Wednesday, 7 April 2021 2:25 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 07/04/21 - 12pm |
| Author: | KM + CE |
| Plant Operating: | |
| Weather: | |

- REB 1 face is currently being trimmed. The contractors are working along a bench at approx. mid slope height and trimming back to design level. No grid was observed on the face of the excavation. Requested that up to date as-builts be supplied to us to ensure that design requirements are being met
- REB 2 has been filled to near full height and a layer of secondary grid was exposed at the time of inspection.

















16/04/21 - Stage 12D

Friday, 16 April 2021 4:50 PM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 16/04/21 - 10am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Refer photos below.

Telecom with Trevor, discussed that underfill drain was not visible. He is going to install a solid pipe to avoid blockage of the drain through the topsoil at the toe of the 1(v) in 3(h) batter, and then form the permanent outlet structure at the end of the solid pipe. Advised to return on Monday to sight this.

underthill Sight 200 ain worked 250 Som 200 lot 150 100 200











20/04/21 - Stage 12D, REB2, Fill M

Tuesday, 20 April 2021 11:59 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 20/04/21 - 8am |
| Author: | KM |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

- Sighted underfill and permanent outlet structure in Stage 12D.
- Filling progressing in REB2, with fill placement near final subgrade level. Several more layers of fill to be placed near the crest to meet design heights.
- Underfill drain in Fill M is to outlet into the permanent stormwater pond. Advised that this should be protected with a rock channel or similar to present scour along the side of the pond.













Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 30/04/21 - 10:15am |
| Author: | КМ |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

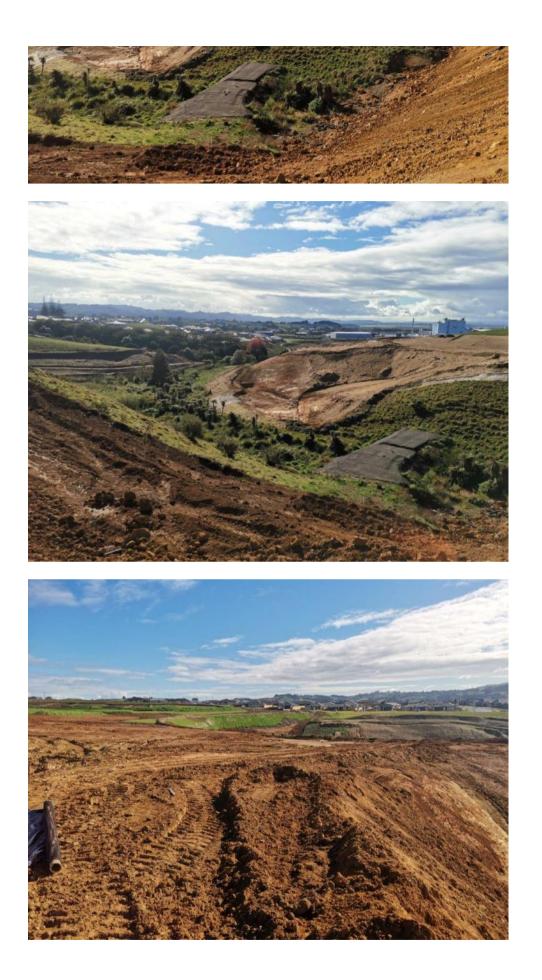
- Received a phone call from Mitchell Rutter (surveyor) this morning advising that the second to last layer of primary grid has been placed using SS20 secondary grid instead. Advised that this should be removed and the appropriate primary grid be placed. He said they would undercut the clay fill over this layer and install the correct grid.
- Primary grid had already been placed and covered at the time of arrival on site. Secondary grid was being trimmed and excavation was at least 2m wide as per the design.











28/05/21 - REB1 & 2, Stage 13

Friday, 28 May 2021 11:03 AM

Site Inspection Record

| Project # & Name: | J00113 - Hitchen Block |
|-------------------|------------------------|
| Date & Time: | 28/05/21 - 10am |
| Author: | |
| Plant Operating: | |
| Weather: | |

Site Observations and Instructions:

Observed progress at Hitchen Block. Observations as follows:

- REB 1 has been formed to a 1(v) in 1(h) face and has been hydroseeded. The initial section of hydroseeding is showing good vegetation cover.

Below REB 1, connection of the underfill drains into the manholes is about to commence. Dines will place 2x solid pipes between the manholes and a perforated novacoil within the same trench wrapping around the outside of the manhole to ensure any additional water seepages are captured.

- REB 2 has also been formed at a 1(v) in 1(h) face. Hydroseeding still planned.

Both REB's required check dam installation.

- Stage 13 nearly complete. Observed permanent underfill drain outlet beneath the permanent stormwater pond.



































