

# Geotechnical Completion Report

**HITCHEN BLOCK STAGE 13B** 

For

DFH JOINT VENTURE LIMITED

J00113 | LDE Limited | 17 January 2022

Ref No: J00113

17 January 2022

DFH Joint Venture Limited PO Box 302 877 North Harbour 1330

### Attention: Mr R Parkinson

Dear Russell

### RE: Geotechnical Completion Report for Hitchen Block Stage 13B, Pokeno

This report presents all supporting geotechnical data and our Suitability Statement in relation to land development works undertaken at the above location.

It has been prepared in accordance with instructions received from DFH Joint Venture Limited and forms part of the documentation required by Waikato District Council to achieve certification under Section 224(c) of the Resource Management Act.

If you have any queries or you require any further clarification on any aspects of this report, please do not hesitate to contact the undersigned.

For and On Behalf of LDE Limited

Prepared by:

K.moro

**K. Meffan** Engineering Geologist MEngNZ

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## 1 INTRODUCTION AND DESCRIPTION OF SUBDIVISION

This Geotechnical Completion Report has been prepared for DFH Joint Venture Limited as part of the documentation required to be submitted to the Waikato District Council following residential subdivisional development.

It contains our Suitability Statement, relevant test data and the CivilPlan Consultants Limited as-built plan set relating to Stage 13B of the Hitchen Block Residential Subdivision as follows:

Title	Reference No.	Date		
As Built Contours	126701-13B-AB200	September 2021		
As Built Cut-Fill Contours	126701-13B-AB201	September 2021		
Stormwater As Built	126701-13B-AB400	September 2021		
Wastewater As Built	126701-13B-AB401	September 2021		

### Table 1: CivilPlan Consultants Limited As-Built Plans

This report covers the construction period October 2019 to December 2021. It is intended to be used for certification purposes as follows:

- 58 residential lots numbered 752 to 809.
- 2 new roads named Harris Place and Leathern Crescent (part).
- 5 jointly owned access lots (JOAL's) numbered 5 to 9
- 1 local purpose (drainage) reserve number lot 11. This Lot contains a stormwater pond.

This stage of the subdivision is located as shown on the attached CivilPlan Consultants Limited as-built plans. As can be seen on the Cut-Fill Contours As-Built plan, approximately half of the lots have been partly or totally affected by filling, to a maximum depth of approximately 4m within Lot 11 (drainage reserve). Elsewhere, fills are generally 1m high or less.

## 2 RELATED REPORTS

A Geotechnical Investigation Report on the subject land was prepared by this Consultancy, reference J00741 (Earthworks Stage 5), dated 29 August 2018. The conclusions and recommendations of that report have been reviewed during the preparation of this document, along with the following Geotechnical Completion Reports (prepared by Lander Geotechnical Consultants Limited) on adjacent recently completed stages of the subdivision which are tabulated below:

-					
	Subdivision Title	Reference No.	Issue Date		
	Hitchen Block Stage 3A2, 3B and 3C (Residential)	J00113	20 December 2017		
	Hitchen Block Stage 5A (Residential)	J00113	26 January 2018		
	Hitchen Block Stage 4A (Residential)	J00113	23 March 2018		
	Hitchen Block Stage 6A and 6B (Residential)	J00113	23 May 2018		
	Hitchen Block Stage 4B (Residential)	J00113	28 August 2018		

 Table 2: Lander Geotechnical Consultants Geotechnical Completion Reports



Subdivision Title	Reference No.	Issue Date
Hitchen Block Stage 7A & 7B (Residential)	J00113	2 November 2018
Hitchen Block Stage 8A & 8B (Residential)	J00113	9 May 2019
Hitchen Block Stage 6D (Residential)	J00113	4 November 2019
Hitchen Block Stages 11, 12 and 14 (Residential)	J00113	13 March 2020
Hitchen Block Stage 9 (Residential)	J00113	24 June 2020
Hitchen Block Stages 6E & 10A to 10D (Residential)	J00113	16 December 2020
Hitchen Block Stages 10E & 10F (Residential)	J00113	11 March 2021
Hitchen Block Stage 12D (Residential)	J00113	20 April 2021
Hitchen Block Stage 13A (Residential	J00113	16 August 2021

## **3 EARTHWORKS OPERATIONS**

### 3.1 Plant

The main items of plant used by the Contractor, Kerry Dines Limited were:

- 7 x bulldozers with scoops;
- 1 x elevating motorscraper;
- 3 x articulated dump trucks;
- 2 x 4WD sheepsfoot compactors;
- 4 x 20T hydraulic excavators;
- 1 x Tractor with disc ploughs.

### 3.2 Construction Programme

Earthworks operations for this Stage commenced in late October 2021 with topsoil stripping and bulk cut operations of the majority of Stage 13B, including the muckout of the gully passing through the drainage reserve. Underfill drainage (comprising of 160mm perforated drain coils, covered with drainage aggregate and fully wrapped with geotextile cloth) was then installed along the gully invert and along a branch from the main gully where water seepage was observed.

Following this, a toe key (undercut beneath fill batter edge) was installed along the south-western boundary of the drainage reserve and a 3m high fill batter was formed here. The purpose pf the toe key was to 'lock in' the filling into the underlying competent ash soils. Placement of bulk engineered clay fills (Fills J and K as shown on the compaction control test plans included in Appendix 2) progressed until May 2019, however, most work completed during this time was on the surrounding stages.

In December 2020, stripping operations progressed through Stage 13B2 where dense welded tuff materials were encountered and approximately 1m of engineered clay fill was placed over these lots.

By November 2021, all public services and roading was essentially complete and all lots were topsoiled.



## 4 QUALITY ASSURANCE AND CONTROLS

### 4.1 Inspections

During earthworks construction, engineering observations were undertaken on a near regular basis to assess compliance with NZS 4431 and our project specific recommendations and specifications. Project specific inspections were required on this stage of the development for:

- Topsoil stripping of earthworks areas;
- Removal of soft sediments in the gully inverts;
- Placement of the underfill drainage;
- Fill placement and plant performance upon the subgrade periodically throughout the bulk filling works.

### 4.2 Quality Control Criteria

Due to the varying soil types being used as filling, the compaction control criteria of minimum allowable shear strength and maximum allowable air voids were mainly used for quality assurance purposes.

Specification details were as follows for general fills:

Minimum Shear Strength and Maximum Air Voids	Method
--	--------

(a)	Air Voids Percentage	
(4)	(As defined in NZS 4402)	
	Average value less than	10%
	Maximum single value	12%
(b)	Undrained Shear Strength	
	(Measured by Pilcon shear vane – calibrated using NZGS 2001 method)	
	Average value not less than	140 kPa
	Minimum single value	120 kPa
Note:	The average value shall be determined over any ten consecutive tests	

In addition to the above, a higher specification was required for the fill embankments for the stormwater pond within the drainage reserve in Stage 13B1. This specification was:

- Maximum air voids of 6%
- Minimum undrained shear strength of 140kPa.

Where hardfill was used on site as part of bulk filling operations (within the toe key in the drainage reserve), a minimum Clegg Impact Value (CIV) of 20 was specified.

### 4.2.1 Compaction Assurance Testing

Regular insitu density, strength and water content tests were carried out on all areas of the filling at or in excess of the frequency recommended by NZS 4431, and a series of hand auger boreholes were also drilled at selected locations as an added check on quality control. The results of this testing (including testing some testing undertaken on adjacent stages of the subdivision) are appended in Appendix 2.



Control tests carried out on the filling showed that several occasions the required compaction standards were not being achieved. Results of the test failures were relayed to the site foreman and/or his staff, and the affected areas of fill were re-worked as necessary. In each case, further testing was carried out until compliance with the standards was achieved.

## 5 PROJECT EVALUATION

## 5.1 Bearing Capacity and Settlement of Building Foundations

Following the completion of earthworks operations, we returned to the site in October 2021 and drilled a series of hand auger boreholes in order to determine representative finished ground conditions and hence evaluate likely foundation options for future building development.

At current subgrade levels all filled and undisturbed natural ground has a geotechnical ultimate bearing capacity of 300 kPa within the influence of conventional shallow residential building foundation loads.

Where any building platforms have been rutted by heavy machinery subsequent to this report, or softened due to ponded rainwater, engineering advice should be sought with a view affected areas be trimmed back to competent ground and reinstated with compacted hardfill to design subgrade level prior to the commencement of building construction.

It should be noted that NZS 3604 only allows a maximum backfill depth of 600mm over the building platform of a dwelling unless an Engineering design solution or endorsement is proposed, on account of the risk of induced consolidation of the subsoils caused by the weight of the backfill.

## 5.2 Expansive Soils

Two sets of Atterberg Limit expansive soil tests and two Shrink-Swell Index tests were carried out on samples selected from around the site and within the zone of likely influence of shallow building foundations to inform the expansive Site Class for this stage of the subdivision.

The Atterberg Limit tests were carried out in accordance with NZS 4402, "Methods of Testing Soils for Civil Engineering Purposes" test section 2 and the Shrink-Swell Index tests were carried out in accordance with AS 1289, "Methods of Testing Soils for Engineering Purposes" test method 7.1.1. These tests were primarily intended to assess the Expansive Classes of the site materials as defined in AS 2870, "Residential Slabs and Footings – Construction" and MBIE Acceptable Solutions and Verification Methods amendment 19<sup>1</sup>.

Based on the laboratory testing and visual tactile assessments of the soils observed in our postconstruction boreholes, the assessed expansive site class for all residential lots when assessed in accordance with MBIE (Acceptable Solution and Verification Methods Amendment 19) and AS2870:2011 is presented in Table 3 below.

<sup>&</sup>lt;sup>1</sup> Ministry of Business, Innovation and Employment. Verification Methods and Acceptable Solutions Amendment 19 for NZ Building Code B1/AS1, Section 3 (as relevant to expansive soils and good ground). Effective 28 November 2019.



Table 3. Expansive Site Class Summary					
Lot No.	MBIE <sup>2</sup> Expansive Site Class	AS2870 Expansive Site Class			
752-801, 804 and 807-809	H (High); Characteristic ground movement up to 78mm.	H1 (High); Characteristic ground movement up to 60mm.			
802, 803, 805 and 806	S (Slight); Characteristic ground movement up to 22mm.	S (Slight); Characteristic ground movement up to 20mm.			

### Table 3. Expansive Site Class Summary

\*Note: The AS2870:2011 assessment is based on the scaling factor of the site being adjusted to a 1/500yr event to meet the recommendations of MBIE.

Specific design alternatives for these Site Classes are presented in the Suitability Statement. These classifications may be re-addressed by end users during building consent if site specific laboratory shrink-swell testing is undertaken by end users, as recommended in the MBIE document attached.

For Class H soils, if slab on-grade floor slab construction takes place during a long dry summer, exposed building platform soils may dry put and become highly desiccated. Over time the rehydration of the soils below the floor slab can cause swelling and floor slab uplift. Floor slab uplift can cause distress of tile floors and in garages where cracks are more apparent. It may also rack upper storeys and/or rooflines if non-load bearing ground floor walls are lifted and act as struts. It is prudent to place hardfill immediately upon completion of subgrade trimming, followed by thorough soaking of the hardfill prior to concrete placement (e.g. for slab on-grade construction), all of which can help to limit the problem.

## 5.3 Lot Gradients

The appended as built contours plan shows areas having gradients steeper than 1(v) in 4(h) or being immediately adjacent to land having such gradients. The extent of these areas has been determined by the surveyed site gradients and our final walkover inspection, but there may be localised areas having such gradients that have not been shown on the plans.

A site-specific computer slope stability analysis was completed for the stormwater pond in Lot 11 (drainage reserve in adjacent Stage 13B1) as part of our GIR and global factors of safety were found to be satisfactory. Additionally, our observations during construction found that ground conditions were consistent with the design assumptions and the as-builts show no departure from the design.

We are satisfied that these lots are <u>not</u> subject to the hazards described in section 71(3) of the Building Act.

Details of resulting building and earthworks restrictions within the vicinity of these lots are presented in the Suitability Statement.

### 5.4 Fill Induced Settlement

As a result of our pre-fill inspections, the installation of subsoil drainage, quality control testing and the elapsed time since the placement of the majority of the filling (i.e. in excess of 12 months), we are of the

<sup>&</sup>lt;sup>2</sup> Ministry of Business, Innovation and Employment. Verification Methods and Acceptable Solutions Amendment 19 for NZ Building Code B1/AS1, Section 3 (as relevant to expansive soils and good ground). Effective 28 November 2019.



opinion that induced differential settlements beneath or within the certified filling due to its imposed weight should be insignificant with respect to conventional NZS 3604 residential building development.

## 5.5 Vegetation Cover

Wherever practical on sloping land beyond building platform areas, all existing bush and grass cover should be maintained. Any vegetation or bush cleared beyond the immediate area of building platforms for temporary construction purposes should be replaced as soon as possible.

The contribution of appropriate vegetation cover to overall site stability and erosion control should not be underestimated.

## 5.6 Stormwater Controls

It is important on all sloping lots that due care is paid to the design and construction of appropriate stormwater disposal systems. These systems should serve to collect all runoff from roofs, decks and paved areas, together with discharges from retaining wall drains and other subsoil drains and should connect directly into the public stormwater drainage network.

Uncontrolled stormwater discharges onto the ground surface or into soakage pits can cause erosion, scour and/or instability on sloping land and should not be permitted under any circumstances where stability could be compromised.

## 5.7 Service Trenches

As is normal on all subdivisions, building developments involving foundations within a 45° zone of influence from pipe inverts will require Engineering input.

### 5.8 Underfill Drains

The appended fill as-built cut-fill contour plans show the alignments of perforated underfill drains that was placed in a mucked out gully invert prior to filling to tap groundwater seepages. The drains run beneath portions of the drainage reserve within Stage 13B1 as indicated on the As-Built Cut-Fill Contours Plan.

This drain was intended to intercept localised groundwater seepages during earthworks and/or allow engineered fill placement as required by the project specifications. The drain was installed as a precautionary measure, not as remedial works for any existing instability, and it requires no specific maintenance.

Notwithstanding, it is recommended that future site development works preserve this drain. In the event that the drain is compromised by any future development works, then the drain should be reinstated under geotechnical engineering observational guidance.

### 5.9 Stormwater Detention Pond

A new wet stormwater detention pond has been constructed in the vicinity of the drainage reserve in the southern corner of the subdivision (within Stage 13B1). Where fill has been placed to form the pond walls, this was required to meet a higher fill compaction criteria as outlined in Section 4.2.1.

The base of this pond encountered cohesive soils and therefore no clay liner was required. If/where underfill drains passed beneath the base of the pond, a minimum capping layer of 1m of engineer



certified clay was placed over the drain to help eliminate the possibility of future piping erosion and/or seepage from the base of the pond.

Global slope stability of the stormwater pond batters was found to be satisfactory as discussed in Section 5.3.

## 5.10 Topsoil

Topsoil depths in likely building platform areas were checked by the drilling of a borehole in the approximate centre of each of the lots. Our findings, which are indicative only and subject to variation at other locations, show that likely topsoil depths are between 100 mm and 250 mm, however, lots 798 contains no topsoil.

## 5.11 Contractor's Work

We have relied on the Contractor's work practices and assume that the works have been carried out in The approved Contract drawings and design details,

- (i) The approved Contract specifications,
- (ii) Authorised Variations to (i) and (ii) during the execution of the works,
- (iii) The conditions of Resource, Earthworks and Building Consents where applicable,
- (iv)The relevant Lander Geotechnical Consultants Limited reports, recommendations and site instructions,

and that all as-built information and other details provided to the Client and/or :DE Limited are accurate and correct in all respects.

## 6 STATEMENT OF PROFESSIONAL OPINION AS TO THE SUITABILITY OF LAND FOR BUILDING DEVELOPMENT

- I, S.G. Lander, of LDE Limited, Auckland, hereby confirm that:
- 1. I am a Chartered Professional Engineer experienced in the field of geotechnical engineering as defined in section 1.2.3 of NZS 4404 and was retained by the Owner/Developer as the Geotechnical Engineer on Stage 13B of the Hitchen Block residential subdivision.
- The extent of preliminary investigations carried out to date are described in Geotechnical Investigation Report reference J00741, dated 29 August 2018, and the conclusions and recommendations of that document have been re-evaluated in the preparation of this report. The results of all tests carried out under Lander Geotechnical Consultants Limited direction are appended.
- 3. In my professional opinion, not to be construed as a guarantee, I consider that:
  - (a) The earth fills shown on the appended fill as-built plan have been placed in compliance with NZS 4431 and related documents.
  - (b) The completed earthworks give due regard to land slope and foundation stability considerations within the residential lots, however, as shown on the appended contour as-built plan lots 755,



756, 758 to 761, 775, 776, 778 to 781, 795 to 798, and 801 to 809 have gradients steeper than 1(v) in 4(h) or are adjacent to land having such gradients.

Any building development and/or earthworks proposals within the areas shown to be steeper than 1(v) in 4(h) on the as-built contours plan are subject to specific geotechnical investigations and/or foundation design, and/ or adequately designed retaining walls and/ or batter slopes.

- (c) The function of the underfill drain beneath portions of the drainage reserve in Stage 13B1 should not be impaired by any future development or landscaping works.
- (d) A geotechnical ultimate bearing capacity of 300 kPa may be assumed for foundation design on all lots (except where specific geotechnical endorsement is required on account of sloping land greater than 1(v in 4(h)).

Where a geotechnical bearing capacity greater than 300 kPa is required, (i.e. outside the limits of NZS 3604, such as when piling is undertaken), further specific site investigation and design of foundations should be carried out prior to building consent application.

(e) The backfilling and compaction of the stormwater and sanitary sewer trenches on this subdivision has where possible been carried out to appropriate standards having regard for the prevailing ground conditions and associated compaction induced pipe loadings.

Nevertheless, no building development should take place within the 45° zone of influence of drain inverts unless endorsed by specific site investigations, foundation designs and by construction inspections undertaken by a Chartered Professional Engineer experienced in geomechanics to ensure that lateral stability and differential settlement issues are addressed and that building loads are transferred beyond the influence of the pipe and beyond the extent of the trench backfill.

(f) The assessed Expansive Site Class for residential lots 802, 803, 805 and 806 is Class S (slight – with characteristic ground surface movement of up to 22mm) in terms of MBIE Acceptable Solutions and Verification Methods Amendment 19 and Class S (slight – with characteristic ground surface movement of up to 20mm) in terms of AS2870:2011.

The assessed Expansive Site Class for residential lots 752 to 801, 804 and 807 to 809 is Class H (high – with characteristic ground surface movement of up to 78mm) in terms of MBIE Acceptable Solutions and Verification Methods Amendment 19 and Class H1 (high – with characteristic ground surface movement of up to 60mm) in terms of AS2870:2011.

Site specific laboratory Shrink-Swell testing and calculation of specific ys values may be undertaken by end-users to re-assess this during building consent stage.

- (g) Subject to the geotechnical limitations, restrictions, recommendations and expansive soil assessments associated with 3(b) to 3(f) above:
  - (i) The filled and undisturbed original ground within residential lot boundaries is generally suitable for residential buildings constructed in accordance with NZS 3604 and related documents.
  - (ii) On residential lots 802, 803, 805 and 806, foundation design may be carried out in accordance with one of the following methods:



- Class S in terms of MBIE Acceptable Solutions and Verifications Methods for NZ Building Code Clause B1 Structure, effective 28 November 2019;
- Class S in terms of AS2870:2011;
- A specific foundation and structural design may be undertaken by a Chartered Professional Engineer who should allow for expansive soil effects referenced above in the design.
- For buildings having brittle exterior cladding appropriate control joints should also be specifically designed depending on architectural specifications and structural form.

(iii) On residential lots 752 to 801, 804 and 807 to 809, foundation design may be carried out in accordance with one of the following methods:

- Class H in terms of MBIE Acceptable Solutions and Verifications Methods for NZ Building Code Clause B1 Structure, effective 28 November 2019;
- Class H1 in terms of AS2870:2011;
- A specific foundation and structural design may be undertaken by a Chartered Professional Engineer who should allow for expansive soil effects referenced above in the design.
- For buildings having brittle exterior cladding appropriate control joints should also be specifically designed depending on architectural specifications and structural form.
- Road subgrades and lot accessway subgrades have been formed having due regard for slope stability and settlement, although CBR values do vary between natural and filled ground as is to be expected.
- Geotechnical aspects of slope stability and pond permeability within the drainage reserve in stage 13B1 have been appropriately addressed and in these respects the pond is suitable for its intended use.

The professional opinion contained within this report is furnished to the Waikato District Council and DFH Joint Venture Limited for their purposes alone, with respect to the particular brief given to us. It may not be relied upon in any other context of for any other purpose without our prior review and agreement. It does not remove the necessity for the normal inspection of site conditions at the time of erection of any dwelling.

The appended table summarises the status of each residential lot covered by this Suitability Statement.

For and on behalf of LDE Limited

Prepared by:

K.moro

K. Meffan Engineering Geologist MEngNZ

Reviewed By:

**C.J. Edwards** Senior Engineering Geologist CMEngNZ (PEngGeol)

Authorised by:

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**S.G. Lander** Principal Geotechnical Engineer CMEngNZ, CPEng, IntPE(NZ)



Table 4.	Suitability	/ Statement	Summary
	Suitability	Julement	Summary

Lot No.	Comments	Topsoil Depth	Ultimate Bearing
		(mm)	(kPa)
752	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
753	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
754	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	100	300
755	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions.	200	300
	Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	
750	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to $1(v)$ in $4(h)$ gradient restrictions.	100	300
756	Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.		
757	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to $1(v)$ in $4(h)$ gradient restrictions.		
758	Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	150	300



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)
759	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	150	300
760	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
761	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
762	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
763	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
764	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
765	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	250	300
766	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)
767	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
768	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
769	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	300	300
770	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
771	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	150	300
772	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
773	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
774	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
775	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	150	300



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)
776	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
777	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
778	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
779	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
780	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	150	300
781	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)
782	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	250	300
783	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
784	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
785	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	100	300
786	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
787	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
788	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
789	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	150	300
790	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
791	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	100	300
792	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)
793	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	150	300
794	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing depth 900mm or an engineer approved alternative foundation design.	200	300
795	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
796	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
797	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
798	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	0	300
799	Foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
800	Foundation design in accordance with either MBIE B1/AS1 Class H or AS2870:2011 Class H1 or NZS 3604 with minimum footing	200	300



Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)
	depth 900mm or an engineer approved alternative foundation design.		
801	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
802	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 <u>or</u> AS2870:2011 Class S <u>or</u> NZS 3604 with minimum footing depth 450mm <u>or</u> an engineer approved alternative foundation design.	200	300
803	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 or AS2870:2011 Class S or NZS 3604 with minimum footing depth 450mm or an engineer approved alternative foundation design.	200	300
804	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	200	300
805	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 <u>or</u> AS2870:2011 Class S <u>or</u> NZS 3604 with minimum footing depth 450mm <u>or</u> an engineer approved alternative foundation design.	100	300
806	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions.	200	300

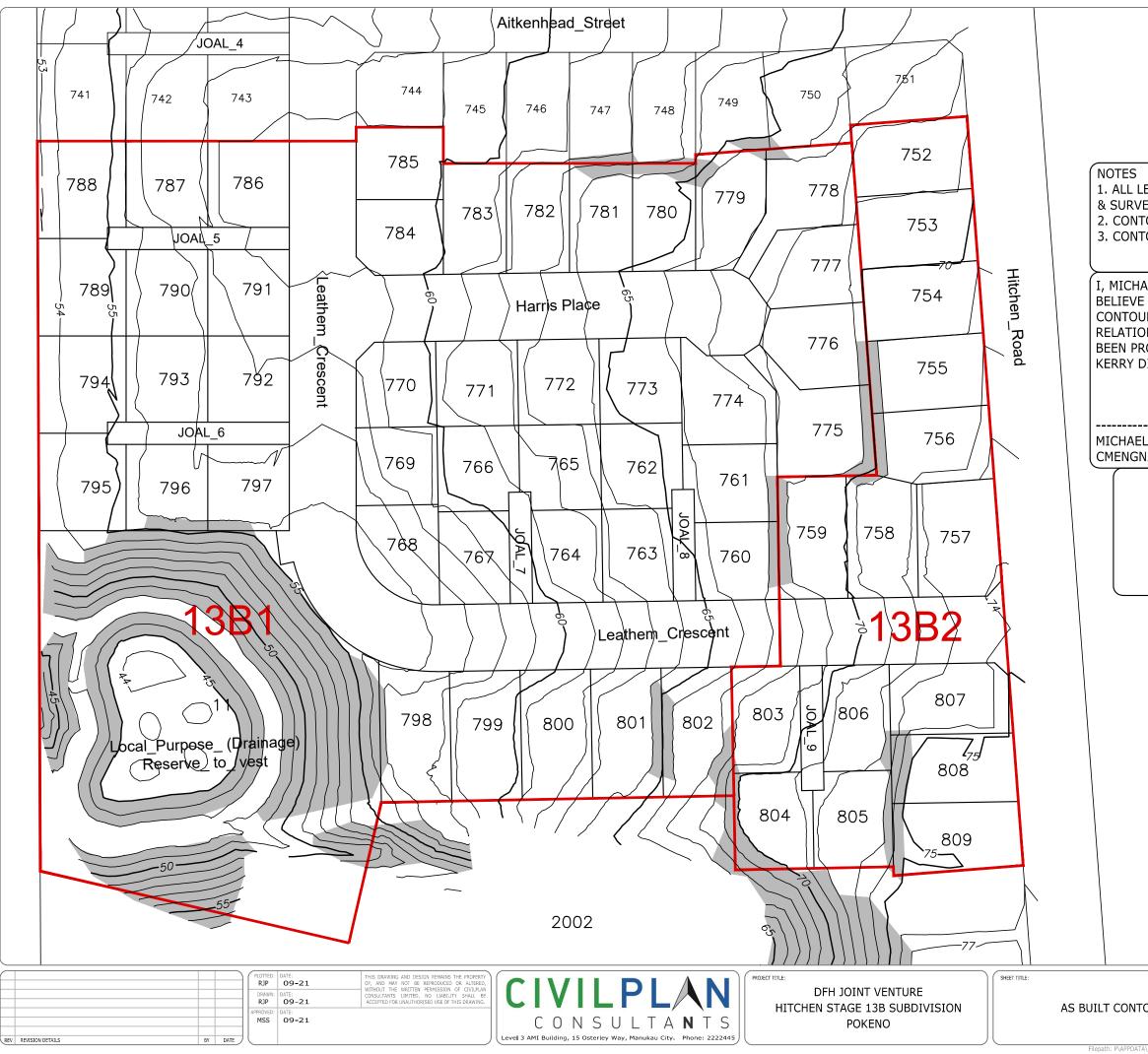


Lot No.	Comments	Topsoil Depth (mm)	Ultimate Bearing (kPa)
	Elsewhere, foundation design in accordance with either MBIE B1/AS1 <u>or</u> AS2870:2011 Class S <u>or</u> NZS 3604 with minimum footing depth 450mm <u>or</u> an engineer approved alternative foundation design.		
807	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	100	300
808	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	300	300
809	Specific site investigation, foundation design and construction inspections required in areas shown hatched on gradient as-built plan due to 1(v) in 4(h) gradient restrictions. Elsewhere, foundation design in accordance with either MBIE B1/AS1 Class H <u>or</u> AS2870:2011 Class H1 <u>or</u> NZS 3604 with minimum footing depth 900mm <u>or</u> an engineer approved alternative foundation design.	300	300



## Appendix 1

CivilPlan Consultants Limited As-Built Plans



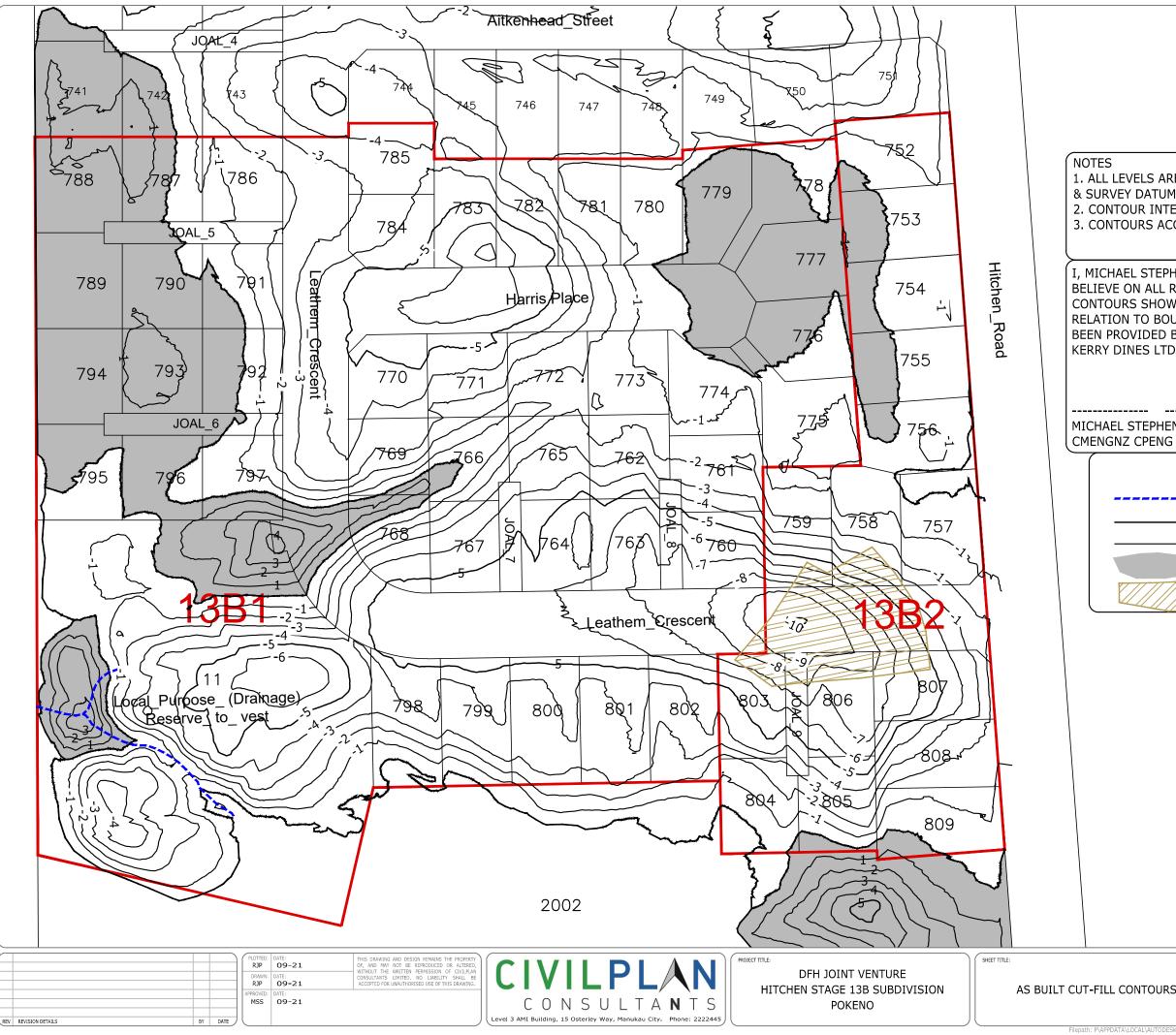


 ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS & SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND 1945.
 CONTOUR INTERVAL IS 1m
 CONTOURS ACCURACY IS + OR - 0.5m

AEL STEPHEN SMITH, CMENGNZ CPENG HEREBY
ON ALL REASONABLE GROUNDS THAT THE
IRS SHOWN ARE ACCURATELY PLOTTED IN
IN TO BOUNDARIES AS THIS INFORMATION HAS
OVIDED BY THE CONTRACTOR OF THE WORKS -
DINES LTD.

_ STEPHEN SMITH IZ CPENG	DATE
	LEGEND
	MAJOR 5m CONTOUR MINOR 1m CONTOUR LAND STEEPER THAN 1:4

	ISSUE STATUS:				AS B	UILT
OURS	SCALE: (A1/A3) SCALE BAR 0 1:1000@A3	1:500 10	/ 1:100 20	00 30	40	50m
	DRAWING NUMBER	12670	)1-13	B-AB	200rev	Α
\LOCAL\AUTODESK\C3D 2020\EN	J\TEMPLATE C:\DATA	A\POKENO\HIT(	CHEN\STAGE	13\AS BUILTS	S\136701-13B	AB200.DWG





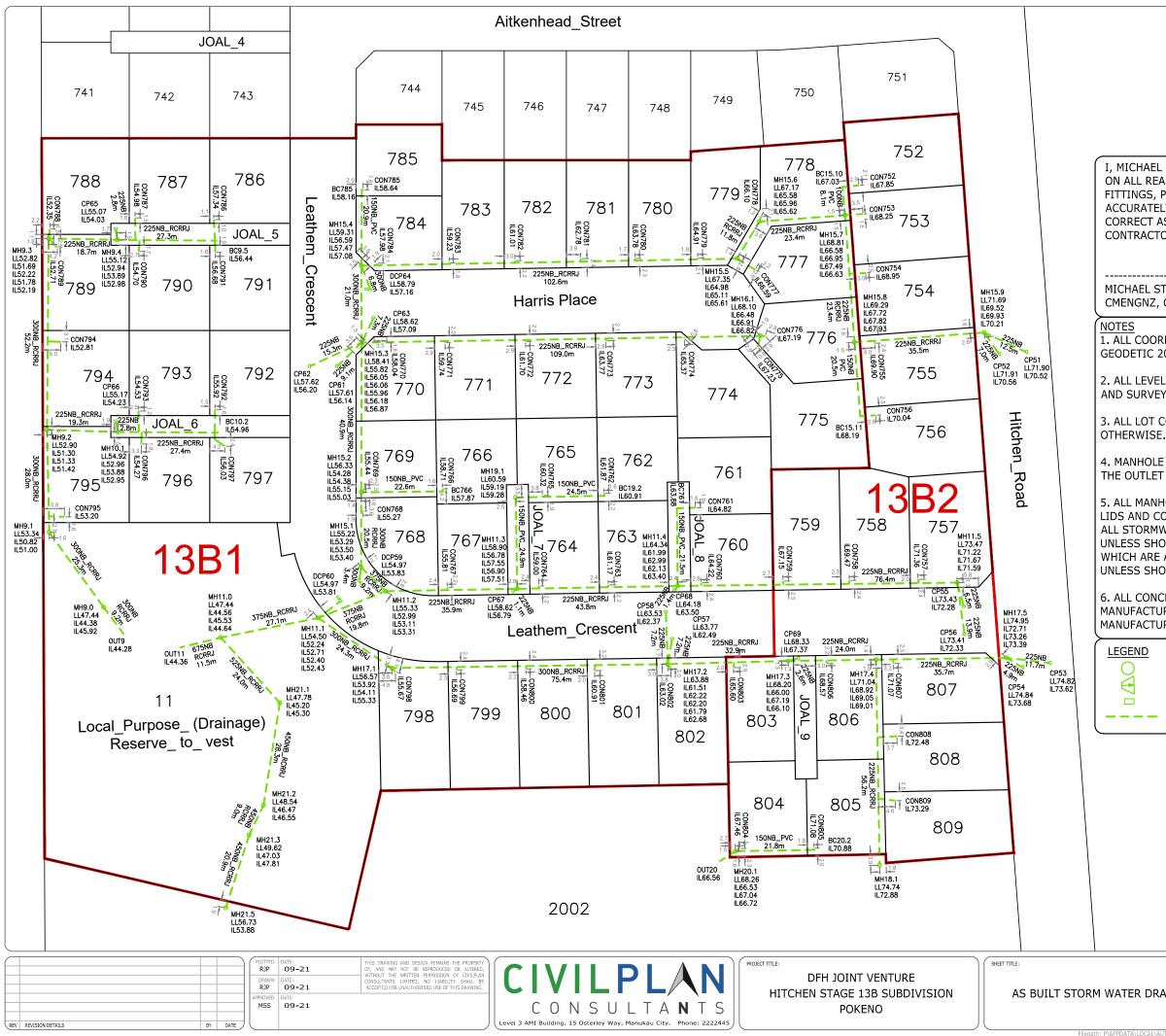
1. ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS & SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND 1945. 2. CONTOUR INTERVAL IS 1m 3. CONTOURS ACCURACY IS + OR - 0.5m

I, MICHAEL STEPHEN SMITH, CMENGNZ CPENG HEREBY BELIEVE ON ALL REASONABLE GROUNDS THAT THE CONTOURS SHOWN ARE ACCURATELY PLOTTED IN RELATION TO BOUNDARIES AS THIS INFORMATION HAS BEEN PROVIDED BY THE CONTRACTOR OF THE WORKS -KERRY DINES LTD.

\_\_\_\_\_ MICHAEL STEPHEN SMITH DATE

LEGEND
 UNDERFILL DRAIN
 MAJOR 5m CONTOUR
MINOR 1m CONTOUR
FILL AREAS
TUFF UNDERCUT AREA
TOTT UNDERCOT AREA

	ISSUE STATUS:				AS B	UILT
ONTOURS	SCALE: (A1/A3) SCALE BAR 0 1:1000@A3	1:500 10	/ 1:100 20	)0 <sub>30</sub>	40	50m
	DRAWING NUMBER					
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I, MICHAEL STEPHEN SMITH, CMENGNZ CPENG HEREBY BELIEVE ON ALL REASONABLE GROUNDS THAT THE PIPE SIZES, FITTINGS, POSITIONS, COORDINATES AND LEVELS SHOWN ARE ACCURATELY PLOTTED IN RELATION TO BOUNDARIES AND IS CORRECT AS THIS INFORMATION HAS BEEN PROVIDED BY THE CONTRACTOR OF THE WORKS - KERRY DINES LTD.

MICHAEL STEPHEN SMITH DATE CMENGNZ, CPENG

1. ALL COORDINATES ARE IN METRES AND IN TERMS OF GEODETIC 2000 MT. EDEN

2. ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS AND SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND 1946

3. ALL LOT CONNECTIONS ARE 100NB PVC UNLESS SHOWN OTHERWISE.

4. MANHOLE INVERT LEVELS ARE LISTED CLOCKWISE FROM THE OUTLET PIPE.

5. ALL MANHOLES ARE 1050NB WITH STANDARD DUTY LIDS AND COVERS UNLESS SHOWN OTHERWISE. ALL STORMWATER PIPES 225NB AND OVER ARE RCRRJ CLASS 2 UNLESS SHOWN OTHERWISE EXCEPT FOR CATCHPIT LEADS WHICH ARE ALL CLASS 4. ALL CATCHPIT LEADS ARE 225NB UNLESS SHOWN OTHERWISE.

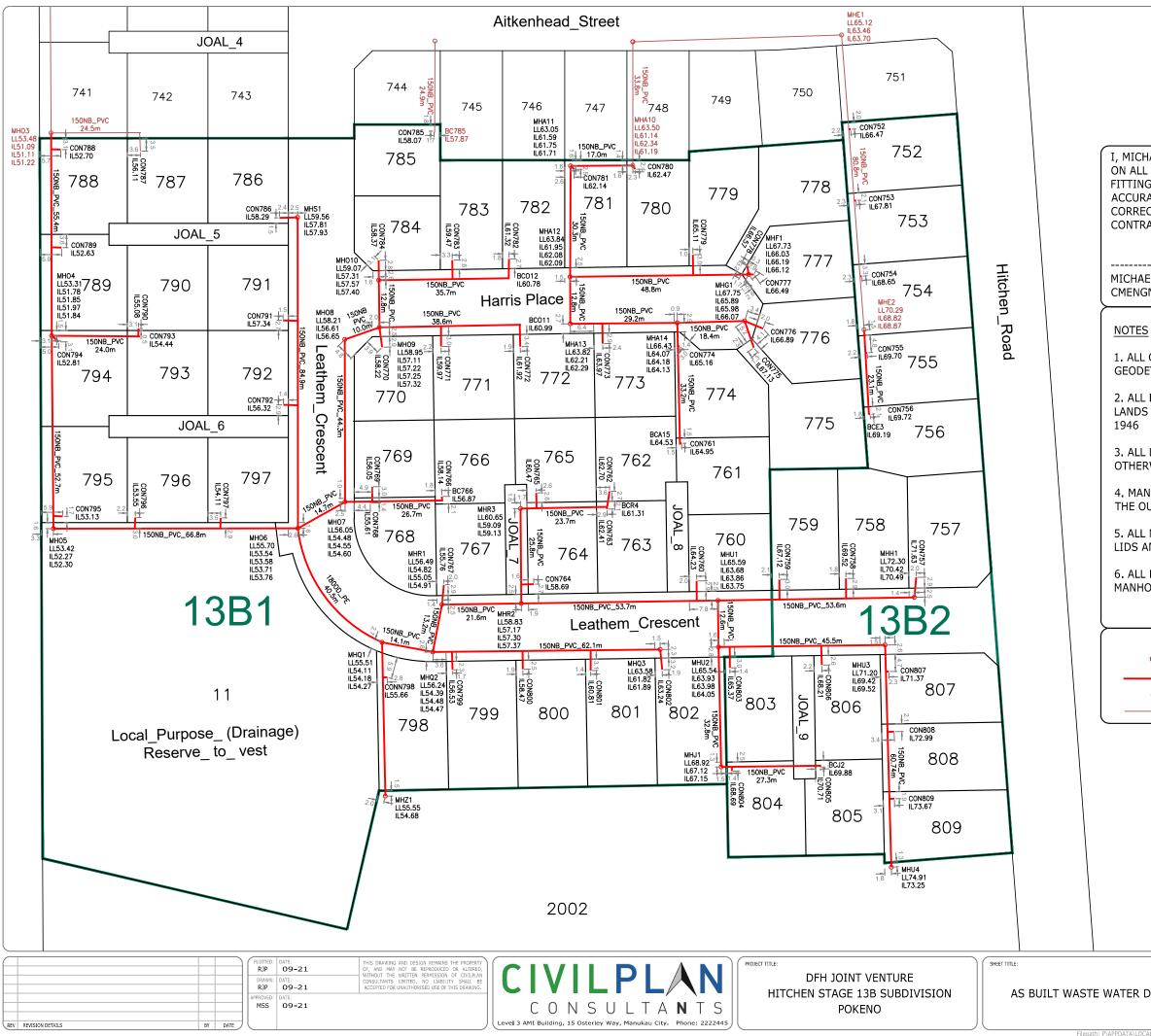
6. ALL CONCRETE PIPES AND STRUCTURES ARE MANUFACTURED BY HYNDS PIPES. ALL PLASTIC PIPES ARE MANUFACTURED BY MARLEY.

 $\bigcirc$ 

)	MANHOLE
1	OUTLET
	CATCHPIT
	SW LINE

EX.MANHOLE
EX.CATCHPIT
 EX.SW LINE

	ISSUE STATUS:				AS B	UILT
R DRAINAGE	SCALE: (A1/A3) SCALE BAR 0	1:500 10	/ 1:100	<b>)0</b> 30	40	50m
	1:1000@A3	.2670	)1-13	B-AB	400rev	A
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I, MICHAEL STEPHEN SMITH, CMENGNZ CPENG HEREBY BELIEVE ON ALL REASONABLE GROUNDS THAT THE PIPE SIZES, FITTINGS, POSITIONS, COORDINATES AND LEVELS SHOWN ARE ACCURATELY PLOTTED IN RELATION TO BOUNDARIES AND IS CORRECT AS THIS INFORMATION HAS BEEN PROVIDED BY THE CONTRACTOR OF THE WORKS - KERRY DINES LTD.

\_\_\_\_\_ \_ DATE MICHAEL STEPHEN SMITH CMENGNZ, CPENG

1. ALL COORDINATES ARE IN METRES AND IN TERMS OF GEODETIC 2000 MT. EDEN

2. ALL LEVELS ARE IN METRES AND IN TERMS OF THE LANDS AND SURVEY DATUM (MEAN SEA LEVEL) AUCKLAND

3. ALL LOT CONNECTIONS ARE 100NB PVC UNLESS SHOWN OTHERWISE.

4. MANHOLE INVERT LEVELS ARE LISTED CLOCKWISE FROM THE OUTLET PIPE.

5. ALL MANHOLES ARE 1050NB WITH STANDARD DUTY LIDS AND COVERS UNLESS SHOWN OTHERWISE.

6. ALL PIPES ARE MANUFACTURED BY MARLEY AND MANHOLES BY HYNDS PIPES.

LEGEND	
0	MANHOLE
	WW LINE
0	EX.MANH

MANHOLL
WW LINE
EX.MANHOLE
EX.WW LINE

	ISSUE STATUS:				AS B	UILT
R DRAINAGE	SCALE: (A1/A3) SCALE BAR 0 1:1000@A3	1:500 10	/ 1:100 20	0 30	40	50m
\LOCAL\AUTODESK\C3D 2020\EN	DRAWING NUMBER					



## Appendix 2

Field Density Test Summary Sheets



Our Ref: 1009213.0657.0.0/3 Customer Ref: J00113 02 August 2021

Lander Geotechnical Itd PO Box 97 385 Manukau Auckland 2241

Attention: Shane Lander

Dear Shane

### Hitchen Block, Pokeno - Stage 5

### **Site Report**

### **Customer's Instructions**

We were instructed to:

Complete nuclear densometer, shear vane and impact hammer testing at the above-mentioned site when requested and report the results.

### **Specifications**

As per email from Mike (Lander Geotechnical) on the 08/01/2019 the specification for Earthwork's testing was as follows;

- Average air voids <10% with maximum air voids 12%.
- Average shear strength >140 kPa with minimum single value 120 kPa.
- Average value to be determined over 10 consecutive tests.

As per email from Mike (Lander Geotechnical) on 21/10/2019 the Earthworks specification for Pond Embankment areas was as follows;

- Average air voids <6% with maximum air voids 8%.
- Average shear strength >140 kPa with minimum single value 120 kPa.

As requested by Kyle (Lander Geotechnical) via email; on the 09/12/2020 the specification for hardfill testing required a minimum single density of 95% of the maximum dry density (MDD) and an impact hammer value of  $\geq 20$ .

As confirmed by Kyle (Lander Geotechnical) via email on the 09/12/2020 an MDD of 2.09 t/m<sup>3</sup> was to be used for the material used on site. The MDD was determined by the Geotechnics plateau test carried out on the 12/02/2020 (Refer to Appendix C).

Material Type	Maximum Dry Density t/m <sup>3</sup>	Optimum Water Content %	Solid Density Assumed t/m <sup>3</sup>	Report Reference Number/Supplier
SPR	2.09	_	-	Plateau Density Test 12/02/2020 (URN 33)

### Laboratory Determined Parameters of Material

## **Dates of Procedures**

Testing was carried out from the 17/10/2019 to 10/05/2021.

### Locations

Testing was carried out as instructed by the contractor on site. Individual test locations were selected on site by the Geotechnics technician on behalf of the customer. No formal testing frequency was provided by the customer.

The attached plans provide indicative locations only and are not to scale. All other information we provide regarding location should be referenced to the asset owner.

### **Samples**

Samples taken for moisture content verification purposes were disposed of 24 hours after testing.

### Methods

NZGS 8:2001 - Test method for determining the vane shear strength of a cohesive soil using a hand held shear vane.

NZS 4407:2015 Test 4.2 - Method using a nuclear surface moisture-density gauge (Direct Transmission Mode) – NDM

NZS 4407:2015 Test 3.1 - Determination of water content

NZS 4407:2015 Test 4.3 Method using a nuclear surface moisture density-gauge (Backscatter mode) - NDM

ASTM D 5874-16 - Standard test method for determination of the impact value (IV) of a soil – Impact Test

## **Material Description**

Material descriptions are provided in the attached results. All descriptions were provided by the customer.

### Results

The following is attached:

Appendix A - Earthworks testing results.

Appendix B - Hardfill testing results.

Appendix C - Plateau density test.

Appendix D- Test location plans.

### **Test Remarks**

### Shear Vane

Shear Vane tests are potentially unsuitable for material described in the Earthworks summary as 'Clay SILT with Gravels, 'Sandy SILT with Clay and Gravels', 'Clay SILT with Sand'. Tests in these materials may not be compliant with the stated test method and results are therefore not covered under the IANZ endorsement of this report. Results are provided for your own interpretation and inference.

### NDM – Direct Transmission

The test method may not be appropriate for materials containing a nominal maximum particle size of >40 mm.

Nuclear densometers are calibrated for a bulk density range of 1,728 kg/m<sup>3</sup> to 2,756 kg/m<sup>3</sup>. Test results outside of these bulk density limits are not covered under the IANZ endorsement of this report.

An assumed solid density value of 2.70 t/m<sup>3</sup> was agreed with the customer. We do not take responsibility for misrepresentation or misinterpretation arising from the use of this assumed value to calculate air voids.

Where oven calculated air voids are negatives, these have been reported as zero.

The calculation of air voids is based on wet density (measured by the nuclear densometer), moisture content (measured by oven drying) and solid density (either assumed or measured by laboratory testing). Negative air voids may be caused by incorrect assumed solid density or due to the variability of onsite material when compared to that tested in a laboratory.

### **Determination of Water Content**

Samples used for the determination of the water content were sampled in conjunction with nuclear densometer testing and disposed of after 24 hours.

### NDM – Backscatter

The test method may not be appropriate for materials containing a nominal maximum particle size of >40 mm.

The wet density and moisture content were measured by the nuclear densometer.

The calculation of percentage compaction is obtained from NDM density values and customer provided maximum dry density (MDD) target.

### Impact Value

Field procedure A was used. The test method is appropriate for materials having a maximum particle size of less than 37.5 mm. Results of tests that were performed on materials containing larger particle sizes, are not covered under the IANZ endorsement of this report.

Calculations of equivalent CBR are based on CBR =  $IV^2x0.07$ . This calculation is not covered under the IANZ endorsement of this report.

## **Pass/Fail Criteria**

We accept no liability for any circumstances that may arise due to the inclusion of the pass/fail criteria or the use of this information by third parties. Pass/fail criteria are based solely on numerical values with no consideration given to uncertainty and are not covered under the IANZ endorsement of these results.

## **General Remarks**

This report has been prepared for the benefit of Lander Geotechnical, with respect to the particular brief given to us and it cannot be relied upon in other contexts or for any other purpose without our prior review and agreement.

The inherent uncertainties of site investigation work, mean the nature and continuity of subsoil away from the test location could vary from the data logged.

Material descriptions are included for information only and are not covered under the IANZ endorsement of this report.

Sample(s) not destroyed during testing will be retained for one month from the date of this report before being discarded.

Please reproduce this report in full when transmitting to others or including in internal reports.

If we can be of any further assistance, feel free to get in touch. Contact details are provided at the bottom of the letterhead page.

GEOTECHNICS LTD

Report prepared by:

David Sayers CMT Field Technician

Authorised for Geotechnics by:

Steven Anderson Project Director Approved Signatory

Report checked by:

Daniel Brasting Project Manager

REDITED

Test results indicated as not accredited are outside the scope of the laboratory's accreditation

3-Aug-21

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Client: Lander Geotechnical Ltd.

Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/SEBA/FRHA
Checked By	JRA/DASA/SEBA
Approved By	SJA

						Test Type		NDM 00			NDM 90 <sup>0</sup>		A	VERAGE ND	м	Solid	_	Final Co	rrected			s	hear Vane	Reading (I	kPa)			PASS / FAIL	
URN	Tech.	Date	Location	Layer	Material		Wet	Moisture	Air Voids	Wet	Moisture	Air Voids	Wet	Moisture	Air Voids	Density (t/m <sup>3</sup> )	Oven Moisture content (%)	Oven Dry		Average Air Voids (10 X Tests)	Readin	g Reading	Reading	Reading	Average	Average	Retest URN	(P) Pass	Comments
						NDM / SV	Density (t/m <sup>3</sup> )	Content (%)	(%)	Density (t/m <sup>3</sup> )	Content (%)	(%)	Density (t/m <sup>3</sup> )	Content (%)	(%)	Assumed	content (70)	Density (t/m <sup>3</sup> )	Air Voids (%)	(10 x 10303)	1	2	3	4	SV (4 x Tests)	SV (10 X Sets)		(F) Fail	1
3.1	JRA	21/10/2019	Fill F - Pond Area	~0.5m Above	Clay SILT	NDM / SV	1.74	45.3	1.6	1.75	42.7	1.9	1.75	44.0	1.8	2.70	41.2	1.24	3.3	-	124	121	121	124	123	-	-	Р	
3.2	5101	11/10/2015	Thirt Fold Area	Underfill Drain	city sici	NDM / SV	1.80	40.6	0.6	1.79	40.1	1.2	1.80	40.4	0.9	2.70	35.8	1.32	3.6	-	135	124	145	161	141	-	-	Р	
4.1	JRA	30/10/2019	Fill F - Pond Area	~1m below FL	Clay SILT	NDM / SV	1.82	37.2	1.5	1.82	36.9	1.7	1.82	37.1	1.6	2.70	36.1	1.34	2.2	-	150	148	150	145	148	-	-	Р	
4.2				~FL		NDM / SV	1.82	32.4	4.5	1.82	32.1	4.7	1.82	32.3	4.6	2.70	32.0	1.38	4.7	-	185	185	174	142	172	-		Р	
5.1	JRA	1/11/2019	Fill E - See Site Plan	~0.5 - 1m placed	Clay SILT	SV	-	-	-	-	-	-	-	-	-	-		-	-	-	87	95	113	106	100	-	6.1	F	
5.2			Pidli			SV	-	-	-	-	-	-	-	-	-	-		-	-	-	103	119	95	79	99	-	6.2	F	
6.1	JRA	2/11/2019	Fill E - See Site Plan	~0.5 - 1m placed	Clay SILT w gravels	NDM / SV	1.89	28.6	3.5	1.90	27.3	4.1	1.89	28.0	3.8	2.70	30.5	1.45	2.0	-	>191	>191	>191	>191	>191	-	-	Р	Retest of URN 5.1
6.2			Fidit			NDM / SV	1.84	32.0	3.9	1.83	32.3	4.0	1.84	32.2	3.9	2.70	34.1	1.37	2.7	•	>191	174	180	>191	>184	-	-	Р	Retest of URN 5.2
7.1	JRA	4/11/2019	Fill E - See Site Plan	~RL 54.70	Clay SILT	NDM / SV	1.88	34.1	0.2	1.89	35.2	0.0	1.88	34.7	0.0	2.70	35.3	1.39	0.0	-	>191	161	166	177	174	-		Р	
7.2			Pidii	~RL 55.20		NDM / SV	1.73	42.1	3.7	1.73	41.3	3.9	1.73	41.7	3.8	2.70	44.5	1.20	2.3	-	147	155	>191	150	>160	149	-	Р	
8.1	JRA	7/11/2019	Fill E - See Site Plan	~RL 55.50	Clay SILT	NDM / SV	1.78	35.9	4.3	1.78	35.5	4.7	1.78	35.7	4.5	2.70	30.9	1.36	7.6	-	>191	122	133	150	>149	151	-	Р	
8.2			r (dl)	~RL 57.00		NDM / SV	1.83	36.0	1.9	1.82	38.6	0.7	1.82	37.3	1.3	2.70	33.0	1.37	4.0	3.2	125	133	>191	139	>147	152.	-	Р	
9.1	JRA	13/11/2019	Fill K - Pond Bund Embankment	~0.5m Below FL of Pond Bund	Clay SILT	NDM / SV	1.76	35.6	5.9	1.76	33.3	7.1	1.76	34.5	6.5	2.70	34.3	1.31	6.6	3.6	>191	>191	>191	>191	>191	157	-	Р	Air Void Average for 2 x tests @ Pond Embankment = 5.6%
9.2			Embankment	or Pond Bund		NDM / SV	1.80	36.3	3.3	1.80	36.5	3.1	1.80	36.4	3.2	2.70	34.1	1.34	4.6	3.7	152	169	161	150	158	155	-	Р	
11.1	JRA	22/11/2019	Fill E - See Site Plan	~1m Below FL	Clay SILT	NDM / SV	1.85	32.2	3.3	1.85	32.3	2.8	1.85	32.3	3.1	2.70	30.4	1.42	4.3	3.9	171	>187	>187	>187	183	164		Р	
11.2			Fidit			NDM / SV	1.87	28.9	4.2	1.87	27.7	5.0	1.87	28.3	4.6	2.70	23.6	1.52	8.1	4.2	128	147	166	144	146	168		Р	
12.1	JRA	25/11/2019	Fill E - See Site Plan	~0.5m Below FL	Clay SILT	NDM / SV	1.84	30.6	4.9	1.84	29.1	5.9	1.84	29.9	5.4	2.70	34.9	1.36	2.0	4.2	>187	>187	>187	>187	>187	168		Р	
12.2		_	Pidli			NDM / SV	1.87	30.8	3.1	1.87	29.7	4.0	1.87	30.3	3.5	2.70	29.8	1.44	3.8	4.3	>187	>187	>187	>187	>187	168	-	Р	
13.1			Fill E - See Site Plan	~FL	Clay SILT	NDM / SV	1.93	31.7	0.0	1.93	32.6	0.0	1.93	32.2	0.0	2.70	31.3	1.47	0.0	4.3	>187	>187	>187	>187	>187	170		Р	
13.2	JRA	27/11/2019	Fidit			NDM / SV	1.88	32.5	1.5	1.88	32.9	1.2	1.88	32.7	1.3	2.70	28.4	1.46	4.3	4.5	>187	>187	>187	>187	>187	172		Р	
13.3			Fill O - Pond Bund Embankment	~1m Below FL	Clay SILT w Gravels	NDM / SV	1.86	31.4	3.0	1.86	31.6	3.1	1.86	31.5	3.1	2.70	25.4	1.48	7.4	4.5	131	176	158	160	156	173	•	Р	
13.4		_	embankment			NDM / SV	1.78	37.4	3.5	1.78	34.6	5.1	1.78	36.0	4.3	2.70	32.8	1.34	6.3	4.7	176	>187	150	142	164	175		Р	
14.1	JRA	29/11/2019	Fill O - Pond Bund Embankment	~FL	Clay SILT	NDM / SV	1.72	34.6	8.6	1.81	34.6	3.7	1.76	34.6	6.1	2.70	33.9	1.32	6.6	4.7	>187	>187	>187	>187	>187	174	-	Р	
14.2		_	Embankment			NDM / SV	1.82	34.9	2.8	1.82	34.7	3.1	1.82	34.8	2.9	2.70	33.5	1.36	3.7	4.7	>187	171	182	>187	182	177	-	Р	
15.1	-		Fill H - See Site Plan			NDM / SV	1.76	40.7	2.7	1.76	39.6	3.3	1.76	40.2	3.0	2.70	43.2	1.23	1.3	4.4	>187	>187	>187	>187	>187	177	-	Р	
15.2	JRA	2/12/2019	Pidli	~FL	Clay SILT	NDM / SV	1.76	39.7	3.2	1.75	40.3	3.5	1.76	40.0	3.4	2.70	39.8	1.26	3.5	3.9	>187	>187	>187	>187	>187	181	-	Р	
15.3	4		Fill O - Pond Floor Area			NDM / SV	1.81	35.0	3.4	1.81	34.6	3.5	1.81	34.8	3.5	2.70	36.1	1.33	2.7	4.0	123	147	163	139	143	177	•	Р	
15.4			Area			NDM / SV	1.79	34.5	4.8	1.79	35.5	4.3	1.79	35.0	4.6	2.70	33.7	1.34	5.4	4.1	150	136	139	131	139	172	•	Р	
16.1	FRHA	13/12/2019	Fill A - See site plan	~3m placed	Clay SILT w gravels	NDM / SV	1.87	32.3	2.2	1.87	31.9	2.2	1.87	32.1	2.2	2.70	32.1	1.41	2.2	4.3	147	147	184	>187	166	170	-	Р	
16.2	-		Pidli			NDM / SV	1.82	34.3	3.1	1.84	32.6	3.4	1.83	33.5	3.2	2.70	33.5	1.37	3.2	4.2	158	176	>187	>187	177	169	-	Р	
17.1	4					NDM / SV	1.77	37.9	3.7	1.78	35.6	4.8	1.78	36.8	4.2	2.70	42.4	1.25	1.0	3.6	>187	>187	UTP	UTP	>187	172	-	Р	
17.2	FRHA	6/01/2020	Fill A - See site plan	~4m placed	Clay SILT	NDM / SV	1.83	34.5	2.5	1.84	35.3	1.5	1.84	34.9	2.0	2.70	33.2	1.38	3.1	3.3	>187	>187	>187	UTP	>187	174	-	Р	
17.3	4		pian			NDM / SV	1.82	35.1	2.6	1.83	35.2	2.5	1.82	35.2	2.5	2.70	32.6	1.38	4.2	3.0	>187	>187	>187	>187	>187	174	-	Р	
17.4						NDM / SV	1.85	33.9	2.0	1.85	35.7	0.8	1.85	34.8	1.4	2.70	33.0	1.39	2.6	2.9	158	147	>187	>187	170	173	-	Р	
18.1	FRHA	8/01/2020	Fill A - See site	~4.5m placed	Clay SILT w gravels	NDM / SV	1.80	36.8	2.8	1.81	37.1	2.2	1.80	37.0	2.5	2.70	33.8	1.35	4.5	3.2	147	>191	169	>191	175	172	-	Р	
18.2			plan			NDM / SV	1.82	37.1	1.8	1.83	34.7	2.7	1.82	35.9	2.2	2.70	37.3	1.33	1.4	3.0	139	147	150	>191	157	169	-	Р	
19.1	FRHA	10/01/2020	Fill A - See site	~5m placed	Clay SILT w gravels	NDM / SV	1.90	26.5	4.8	1.89	25.6	5.9	1.89	26.1	5.3	2.70	24.4	1.52	6.6	3.4	>191	>191	UTP	>191	>191	174	-	Р	
19.2			plan			NDM / SV	1.88	26.2	5.8	1.89	25.8	5.8	1.88	26.0	5.8	2.70	26.4	1.49	5.5	3.4	'>191	122	>191	>191	168	176		Р	
20.1	FRHA	13/01/2020	Fill A - See site	~5.5m placed	Clay SILT w gravels	NDM / SV	1.84	28.1	6.7	1.83	30.0	5.6	1.83	29.1	6.1	2.70	35.8	1.35	1.7	3.4	180	>191	>191	>191	188	179	-	Р	
20.2			plan			NDM / SV	1.79	22.6	12.9	1.80	22.4	12.8	1.79	22.5	12.9	2.70	27.0	1.41	9.6	4.0	>191	>191	>191	>191	>191	180	21.1	F	Dylan informed of fail and a retest was scheduled after he has put some damper fill in and reworked.



Client: Lander Geotechnical Ltd.

Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/SEBA/FRHA
Checked By	JRA/DASA/SEBA
Approved By	SJA

						Test Type		NDM 0 <sup>0</sup>			NDM 90 <sup>0</sup>		A	VERAGE ND	м	Solid	Oven	Final Co	orrected	A		Sh	iear Vane R	teading (H	kPa)			PASS / FAIL	
URN	Tech.	Date	Location	Layer	Material	10010	Wet Density	Moisture	Air Voids	Wet Density	Moisture	Air Voids	Wet Density	Moisture	Air Voids	Density (t/m <sup>3</sup> )	Moisture content (%)	Oven Dry Density		Voids (10 X Tests)	Reading	Reading	Reading R	Reading	Average	Average	Retest URN	(P) Pass	Comments
						NDM / SV	(t/m <sup>3</sup> )	Content (%)	(%)	(t/m <sup>3</sup> )	Content (%)	(%)	(t/m <sup>3</sup> )	Content (%)	(%)	Assumed	content (10)	(t/m <sup>3</sup> )	Air Voids (%)	(10 x 10303)	1	2	3	4	SV (4 x Tests)	SV (10 X Sets)		(F) Fail	
21.1	FRHA	14/01/2020	Fill A - See site	~5.5m placed	Clay SILT	NDM / SV	1.73	40.5	4.5	1.73	39.6	4.8	1.73	40.1	4.7	2.70	37.4	1.26	6.2	4.5	150	'>191	'>191	'>191	150	176	-	Р	Retest of URN 20.2
21.2	FKHA	14/01/2020	plan	"5.5m placed	Clay SILT	NDM / SV	1.71	37.6	7.1	1.71	38.5	6.6	1.71	38.1	6.8	2.70	39.8	1.23	5.9	4.8	180	147	'>191	'>191	164	174	-	Р	
22.1	FRHA	20/01/2020	Fill A - See site	~0.5m Placed	Clay SILT	NDM / SV	1.81	34.1	3.9	1.80	36.2	3.4	1.80	35.2	3.7	2.70	39.4	1.29	1.1	4.5	180	>191	177	166	179	173	-	Ρ	
22.2	Turr	20/01/2020	plan	0.5mm lacca	city sici	NDM / SV	1.82	35.0	3.1	1.82	35.4	2.6	1.82	35.2	2.9	2.70	37.0	1.33	1.8	4.4	>191	>191	>191	>191	>191	175	-	Р	
23.1	FRHA	22/01/2020	Fill A - See site	~1m Placed	Clay SILT	NDM / SV	1.79	32.5	5.9	1.81	30.8	6.3	1.80	31.7	6.1	2.70	31.8	1.37	6.0	4.6	150	>191	>191	>191	181	176	-	Р	
23.2	FKHA	22/01/2020	plan	TW Maced	Clay SIL1	NDM / SV	1.81	36.4	2.5	1.82	36.6	1.9	1.82	36.5	2.2	2.70	35.8	1.34	2.6	4.7	163	174	>191	>191	180	178	-	Р	
24.1	50114	24/04/2020	Fill A - See site	and Car Diaco d	dia su z	NDM / SV	1.76	31.0	8.5	1.77	32.7	6.8	1.77	31.9	7.6	2.70	35.6	1.30	5.3	4.6	>191	>191	>191	>191	>191	178	-	Р	
24.2	FRHA	24/01/2020	plan	~1.5m Placed	Clay SILT	NDM / SV	1.73	33.6	8.4	1.74	35.1	7.1	1.74	34.4	7.8	2.70	32.2	1.31	9.1	4.9	>191	>191	>191	>191	>191	180	-	Р	
25.1	FRHA	20/04/2020	Fill A - See site	RL 57.8	dia cita	NDM / SV	1.79	29.4	8.0	1.79	30.3	7.4	1.79	29.9	7.7	2.70	29.8	1.38	7.7	5.5	>191	>191	>191	>191	>191	181	-	Р	
25.2	FKHA	29/01/2020	plan	RL 50.04	Clay SILT	NDM / SV	1.81	35.2	3.1	1.81	36.6	2.2	1.81	35.9	2.6	2.70	34.0	1.35	3.8	5.0	>191	>191	>191	>191	>191	181	-	Р	
26.1	FRHA	31/01/2020	Fill A - See site	~RL 50.54	Clay SILT	NDM / SV	1.93	29.4	0.8	1.93	24.4	4.7	1.93	26.9	2.7	2.70	23.2	1.57	5.6	4.9	>191	>191	>191	>191	>191	185	-	Р	
26.2	INDA	31/01/2020	plan	~6m Placed	Ciay SIL1	NDM / SV	1.88	26.0	6.0	1.88	25.5	6.6	1.88	25.8	6.3	2.70	25.5	1.5	6.5	5.0	>191	>191	>191	>191	>191	187	-	Р	
27.1	FRHA	3/02/2020	Fill A - See site	~RL 51.04	Clay SILT	NDM / SV	1.78	32.8	6.2	1.78	33.8	5.9	1.78	33.3	6.0	2.70	35.4	1.3	4.7	5.3	>191	>191	>191	>191	>191	189	-	Р	
27.2			plan	~RL 58.3		NDM / SV	1.77	28.7	9.4	1.77	31.1	8.0	1.77	29.9	8.7	2.70	38.3	1.3	3.5	5.5	>191	>191	>191	>191	>191	189	-	Р	
28.1	FRHA	5/02/2020	Fill A - See site	RL 56.10	Clay SILT	NDM / SV	1.76	30.6	8.8	1.76	29.8	9.4	1.76	30.2	9.1	2.70	29.3	1.4	9.7	5.9	>191	>191	>191	>191	>191	190	-	Р	
28.2			pian	RL 51.85		NDM / SV		24.6	5.8	1.89	29.0	3.3	1.90	26.8	4.5	2.70	24.8	1.5	6.0	6.2	>191	>191		>191	>191	191	-	Р	
29.1	FRHA	10/02/2020	Fill A - See site plan	RL 50.17	Clay SILT	NDM / SV		31.9	4.0	1.84	32.4	3.6	1.84	32.2	3.8	2.70	28.6	1.4	6.2	6.3	150	>191		166	174	189	-	Р	
29.2			plan	RL 58.35		NDM / SV	1.82	33.8	3.4	1.84	33.1	2.9	1.83	33.5	3.2	2.70	39.3	1.3	0.0	5.4	>191	>191		>191	>191	189	-	Р	
30.1	FRHA	12/02/2020	Fill A - See site plan	~RL 56.15	Clay SILT	NDM / SV		28.8	3.0	1.9	29.9	1.7	1.90	29.4	2.4	2.70	28.5	1.5	3.0	4.9	120	>191		188	157	186	-	Р	
30.2				~RL 50.22		NDM / SV		29.7	3.4	1.87	30.0	3.5	1.87	29.9	3.4	2.70	32.9	1.4	1.4	4.7	150	>191		163	174	184	-	P	
31.1 31.2	FRHA	14/02/2020	Fill A - See site plan	~RL 58.55 ~RL 50.67	Clay SILT	NDM / SV		29.7	8.2	1.79	28.2	9.1 3.2	1.79	29.0	8.6	2.70	31.8	1.4	6.7	4.8	UTP	UTP 174		>191	>191	184	-	P	
31.2				~RL 50.67		NDM / SV		31.9 33.4	3.3	1.86	31.5 32.4	4.3	1.85	31.7 32.9	3.2 3.8	2.70	31.2 31.7	1.4	4.6	4.5	128 >191	>191		>191	>191	181	-	P	
32.1	FRHA	17/02/2020	Fill A - See site plan	~RL 58.00	Clay SILT	NDM / SV	1.05	21.2	8.6	1.89	22.7	4.5 8.0	1.85	22.0	8.3	2.70	37.2	1.4	0.0	4.5	139	122		>191	151	177		P	
33.1				~RL 59.00		NDM / SV		30.0	7.7	1.77	28.9	9.4	1.78	29.5	8.6	2.70	32.0	1.3	6.9	3.8	>191	>191		>191	>191	177		Р	
33.2	FRHA	19/02/2020	Fill A - See site plan	~RL 58.50	Clay SILT	NDM / SV	-	29.0	6.3	1.82	29.1	6.7	1.83	29.1	6.5	2.70	30.4	1.4	5.6	3.8	>191	>191		>191	>191	177	-	P	
35.9			511 A . 6			NDM / SV	1.80	35.9	3.6	1.79	34.6	4.9	1.79	35.3	4.3	2.70	32.8	1.3	5.8	3.8	>191	>191		>191	>191	179	-	Р	
35.10	FRHA	21/02/2020	Fill A - See site plan	~RL 50.85	Clay SILT	NDM / SV	1.89	26.5	4.9	1.90	25.7	5.2	1.90	26.1	5.0	2.70	31.7	1.4	1.0	3.9	133	150	174	152	152	175	-	Р	
39.1			Fill A - See site			NDM / SV	1.81	31.6	5.6	1.80	32.3	5.8	1.80	32.0	5.7	2.70	35.5	1.3	3.4	3.9	136	>191	>191	>191	177	177	-	Р	
39.2	FRHA	26/02/2020	plan	~RL 51.35	Clay SILT	NDM / SV	1.80	36.2	3.3	1.80	37.8	2.2	1.80	37.0	2.7	2.70	38.8	1.3	1.7	3.9	177	>191	>191	>191	>187.5	178	-	Р	
42.9			Fill A - See site	RL 63.65		NDM / SV	1.95	30.6	0.0	1.95	31.4	0.0	1.95	31.0	0.0	2.70	37.3	1.4	0.0	3.3	>191	>191	>191	>191	>191	178	-	Р	
42.10	FRHA	2/03/2020	plan	RL 61.82	Clay SILT	NDM / SV	1.90	35.1	0.0	1.90	34.6	0.0	1.90	34.9	0.0	2.70	34.7	1.4	0.0	2.9	>191	>191	>191	>191	>191	181	-	Р	
43.1		- / /	Fill A - See site	RL 59.16		NDM / SV	1.86	28.7	5.2	1.86	29.2	4.7	1.86	29.0	4.9	2.70	36.4	1.4	0.0	2.4	106	95	95	120	104	173	-	F	Shear Vanes < 120, Kyle and Dylan informed, no retest requested.
43.2	FRHA	9/03/2020	plan	RL 57.56	Clay SILT	NDM / SV	1.87	30.3	3.6	1.86	30.2	4.1	1.86	30.3	3.8	2.70	32.4	1.4	2.4	2.7	>191	188	>191	180	>187.5	181	-	Р	
44.9	FRHA	13/03/2020	Fill A - See site	RL 58.94	Clau SII T	NDM / SV	1.79	26.5	9.9	1.79	27.3	9.6	1.79	26.9	9.8	2.70	32.4	1.4	6.1	2.6	>191	>191	>191	>191	>191	185	-	Р	
44.10	FRHA	13/03/2020	plan	RL 47.90	Clay SILT	NDM / SV	1.83	29.4	6.1	1.81	34.0	4.0	1.82	31.7	5.1	2.70	36.0	1.3	2.3	2.3	>191	>191	>191	>191	>191	185	-	Р	
45.11	FRHA	16/03/2020	Fill A - See site	~RL 48.40	Clay SILT	NDM / SV	1.88	27.4	5.1	1.87	28.6	4.6	1.87	28.0	4.9	2.70	30.5	1.4	3.1	2.0	>191	>191	>191	>191	>191	185	•	Р	
45.12	1010	10/03/2020	plan	~RL 59.44		NDM / SV	1.88	24.5	7.0	1.88	23.7	7.7	1.88	24.1	7.3	2.70	22.1	1.54	8.9	2.8	180	139	128	120	142	180	-	Ρ	
46.1	FRHA	18/03/2020	Fill A - See site	~RL 48.30	Clav SILT	NDM / SV	1.84	31.8	3.9	1.84	31.7	4.2	1.84	31.8	4.0	2.70	32.6	1.39	3.4	2.8	>191	>191	>191	>191	>191	184	-	Ρ	
46.2	THUR	13,03,2020	plan	~RL 48.90	City Sici	NDM / SV	1.86	32.1	2.6	1.86	29.7	4.2	1.86	30.9	3.4	2.70	32.2	1.41	2.5	2.9	>191	>191	>191	161	184	184	-	Р	



Client: Lander Geotechnical Ltd.

Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/SEBA/FRHA
Checked By	JRA/DASA/SEBA
Approved By	SJA

						Test Type		NDM 0 <sup>0</sup>			NDM 90 <sup>0</sup>		,	VERAGE N	м	Solid	Oven	Final Co	rrected			s	hear Vane	Reading (	kPa)			PASS / FAIL	
URN	Tech.	Date	Location	Layer	Material	NDM / SV	Wet Density	Moisture	Air Voids	Wet Density	Moisture	Air Voids	Wet Density	Moisture Content	Air Voids	Density (t/m <sup>3</sup> )	Moisture content (%)	Oven Dry Density	Average Air Voids	Voids (10 X Tests)		Reading	Reading	Reading	Average SV (4 x	Average SV	Retest URN	(P) Pass	Comments
						NDNI/ SV	(t/m <sup>3</sup> )	(%)	(%)	(t/m <sup>3</sup> )	(%)	(%)	(t/m <sup>3</sup> )	(%)	(%)	Assumed		(t/m <sup>3</sup> )	(%)		1	2	3	4	Tests)	(10 X Sets)		(F) Fail	
47.1			Fill A - See site	~RL 59.45		NDM / SV	1.78	33.1	6.1	1.8	36.2	4.9	1.77	34.7	5.5	2.70	41.4	1.26	1.5	3.0	125	122	131	131	127	180	-	Р	
47.2	FRHA	1/05/2020	plan	~RL 49.00	Clay SILT	NDM / SV	1.81	31.9	5.3	1.8	32.9	4.4	1.81	32.4	4.9	2.70	31.0	1.38	5.8	3.6	147	136	174	133	148	176	-	P	
47.3			Fill F - Drainage line	~0.5m to FL		NDM / SV	1.78	32.9	6.4	1.78	29.9	8.4	1.78	31.4	7.4	2.70	28.8	1.38	9.1	4.5	>191	>191	>191	>191	>191	176	-	P Testing re	requested along filled in trench line where drainage has been installed
49.1	FRHA	15/05/2020	Fill E - Storm water Line	~FL	Clay SILT	NDM / SV	1.92	20.3	8.6	1.92	20.0	8.9	1.92	20.2	8.8	2.70	23.4	1.55	6.1	4.9	152	122	161	163	150	165	-	P	
49.2			Fill E - Sewer line			NDM / SV	1.95	26.8	1.9	1.94	28.6	1.0	1.94	27.7	1.5	2.70	20.0	1.62	7.6	5.0	122	120	139	150	133	159	-	р	
52.1	JRA	4/09/2020	Fill A - REB	~1m above Shear	Clay SILT w Gravels	NDM / SV	1.88	35.8	0.0	1.88	35.8	0.0	1.88	35.8	0.0	2.70	38.3	1.36	0.0	4.8	163	176	174	174	172	165	-	P	
52.1		,,		Key Hardfill		NDM / SV	1.92	30.7	0.7	1.92	30.1	0.7	1.92	30.4	0.7	2.70	29.4	1.48	1.4	4.6	>187	163	158	>187	174	159	-	р	
53.1	JRA	22/09/2020	Fill A - REB	~5m above Shear Key Hardfill - 1st	Clay SILT w Gravels	NDM / SV	1.87	29.5	4.1	1.87	31.1	2.6	1.87	30.3	3.4	2.70	33.1	1.40	1.5	3.9	171	123	131	147	143	165	-	Р	
53.2	5101	11/03/2020	THIN THE	Bench	city size workers	NDM / SV	1.84	39.3	0.0	1.85	31.4	3.8	1.84	35.4	1.5	2.70	32.0	1.40	3.7	3.9	126	144	>187	171	157	156	-	P	
54.1	SEBA	5/10/2020	Fill A - REB	~5m above Shear	Clay SILT w Gravels	NDM / SV	1.90	31.3	1.1	1.89	31.6	1.5	1.89	31.5	1.3	2.70	28.9	1.47	3.1	4.3	189	189	189	189	189	158	-	Р	
54.2	SEDM	3/10/2020	FIII A - NEB	Key Hardfill - 1st Bench	Ciay SILT W GraVels	NDM / SV	1.91	32.2	0.0	1.90	31.5	1.1	1.90	31.9	0.5	2.70	28.9	1.48	2.6	3.9	189	189	189	189	189	164	-	Р	
55.1	STFB	7/10/2020	Fill A - REB	Lower Shear Key RL 43.00	Clay SILT w Gravels	NDM / SV	1.84	32.8	3.0	1.85	28.4	5.8	1.85	30.6	4.4	2.70	33.1	1.39	2.7	2.7	156	184	170	173	171	167	-	Р	
55.2	316	//10/2020	FILLA - NED	Upper Shear Key RL 56.37	Clay SILT W Gravers	NDM / SV	1.86	25.4	7.5	1.86	27.2	5.9	1.86	26.3	6.7	2.70	27.8	1.46	5.6	4.2	178	161	156	167	166	164	-	Р	
56.1	PEFE	9/10/2020	Fill A - REB	Lower Shear Key	Clay SILT w Gravels	NDM / SV	1.83	33.3	3.3	1.80	32.8	5.1	1.82	33.1	4.2	2.70	30.2	1.40	6.1	3.4	172	164	160	156	163	166	-	Р	
56.2	PEFE	9/10/2020	FILLA - NED	RL 43.8	Clay SILT W Gravels	NDM / SV	1.85	34.3	1.6	1.84	35.4	1.6	1.85	34.9	1.6	2.70	30.6	1.41	4.4	3.1	164	174	200	170	177	170	-	Р	
57.1	JRA	23/10/2020	Fill A - REB	RL 45.40	Clay SILT w Gravels	NDM / SV	1.83	37.3	0.9	1.83	35.6	1.9	1.83	36.5	1.4	2.70	34.1	1.36	2.9	3.4	144	126	160	131	140	167	-	Р	
57.2	2101	23/10/2020	THIN NED	112 43.40	city size workers	NDM / SV	1.92	29.8	1.3	1.92	28.6	2.0	1.92	29.2	1.6	2.70	27.9	1.50	2.6	3.5	>187	>187	>187	>187	>187	168	-	Р	
58.1	IRA	28/10/2020	Fill A - REB	RL 46.20	Clay SILT w Gravels	NDM / SV	1.81	32.6	4.9	1.81	32.4	5.1	1.81	32.5	5.0	2.70	38.2	1.31	1.4	3.5	136	134	147	144	140	168	-	Р	
58.2						NDM / SV	1.90	29.3	2.4	1.91	28.9	2.2	1.91	29.1	2.3	2.70	27.8	1.49	3.2	3.5	>187	>187	>187	>187	>187	171	-	Р	
59.1	IRA	30/10/2020	Fill A - REB	RL 47.00	Clay SILT w Gravels	NDM / SV	1.84	31.9	3.7	1.85	35.3	1.3	1.84	33.6	2.5	2.70	34.7	1.37	1.8	3.3	>187	>187	>187	>187	>187	171	-	Р	
59.2						NDM / SV	1.73	36.4	7.0	1.74	36.4	6.2	1.73	36.4	6.6	2.70	34.2	1.29	7.9	3.9	144	163	158	136	150	167	-	Р	
60.1	JRA	4/11/2020	Fill A - REB	RL 48.20	Sandy SILT w clay and	NDM / SV	1.75	31.9	8.6	1.75	31.9	8.7	1.75	31.9	8.6	2.70	39.1	1.26	4.3	4.0	>187	>187	131	128	158	166	-	р	
60.2				RL 48.50	gravels	NDM / SV	1.75	28.7	10.8	1.77	29.6	8.8	1.76	29.2	9.8	2.70	44.5	1.22	0.7	3.5	174	123	126	126	137	163	-	р	
61.1	JRA	16/11/2020	Fill A - REB	RL 49.20	Sandy SILT w clay and	NDM / SV	1.80	37.1	2.9	1.81	39.2	0.9	1.80	38.2	1.9	2.70	44.6	1.25	0.0	2.9	170	126	151	151	150	161	-	р	
61.2					gravels	NDM / SV	1.76	42.1	2.0	1.76	43.9	0.9	1.76	43.0	1.4	2.70	45.8	1.21	0.0	2.5	188	>216	166	182	188	162	-	P	
62.1						NDM / SV	1.88	29.9	3.3	1.88	32.1	1.8	1.88	31.0	2.5	2.70	30.9	1.43	2.6	2.5	182	130	139	159	153	164	-	р	
62.2	JRA	4/12/2020	Fill A - REB	RL 52.60	Sandy SILT w clay and	NDM / SV	1.90	28.5	3.0	1.92	27.3	3.1	1.91	27.9	3.0	2.70	31.7	1.45	0.3	2.2	139	139	159	153	148	160	-	P	
62.3					gravels	NDM / SV	1.90	27.8	3.6	1.90	27.3	3.9	1.90	27.6	3.7	2.70	29.0	1.47	2.7	2.4	133	179	184	170	167	162	-	Р	
62.4						NDM / SV	1.89	28.0	3.9	1.89	27.1	4.6	1.89	27.6	4.3	2.70	31.2	1.44	1.7	2.2	>199	>199	>199	>199	>199	164	-	Р	
63.1				RL 50.90		NDM / SV	1.95	24.3	3.8	1.95	23.6	4.4	1.95	24.0	4.1	2.70	20.9	1.61	6.6	2.7	>199	>199	>199	>199	>199	165	-	Р	
63.2				RL 51.90		NDM / SV	1.86	34.7	1.2	1.86	34.6	0.8	1.86	34.7	1.0	2.70	30.6	1.42	3.7	2.3	176	>199	153	156	171	167	-	Р	
63.3	JRA	7/12/2020	Fill A - REB	RL 52.00	Sandy SILT w clay and	NDM / SV	1.91	28.7	2.6	1.91	28.3	2.9	1.91	28.5	2.7	2.70	25.4	1.52	5.0	2.3	156	122	125	139	136	165	-		Dug by Dines to test missed layers as requested by Kyle (Lander) / Trevor (Dines) - Testing
63.4				RL 51.00	gravels	NDM / SV	1.90	29.7	2.4	1.89	30.4	2.4	1.89	30.1	2.4	2.70	28.9	1.47	3.2	2.6	128	153	>199	133	153	166	-	P	ed behind area of Geo Grid.
63.5				RL 53.40		NDM / SV		24.4	8.2	1.87	25.7	6.9	1.86	25.1	7.5	2.70	23.8	1.50	8.5	3.4	>199	>199	153	>199	>187	170	-	Р	
63.6				RL 52.90		NDM / SV	1.86	23.1	9.1	1.86	23.9	8.6	1.86	23.5	8.8	2.70	27.5	1.46	5.9	4.0	>199	>187	184	>187	189	170	-	Р	
64.1			Fill A - REB	RL 53.0		NDM / SV	1.90	26.7	4.6	1.90	28.4	3.3	1.90	27.6	3.9	2.70	25.2	1.52	5.7	4.3	>199	>199	UTP	UTP	>199	175	-	Р	
64.2	JRA / DASA	9/12/2020			Clay SILT w gravels	NDM / SV		31.9	2.6	1.80	29.9	7.0	1.83	30.9	4.8	2.70	36.7	1.34	1.1	4.4	136	139	>199	156	158	176	-	Р	
64.3	DAJA		Fill Area - 415/422	~1.5m below FL		NDM / SV		35.2	1.3	1.85	34.1	1.6	1.85	34.7	1.5	2.70	37.5	1.35	0.0	4.1	139	136	153	159	147	174	-		ovided Area tested Reference of 415/422 - See Site plan for approximate testing location.
64.4						NDM / SV	1.83	28.3	7.0	1.80	33.7	4.6	1.81	31.0	5.7	2.70	37.3	1.32	1.7	4.1	133	139	145	139	139	168	-	Р	



Client: Lander Geotechnical Ltd.

Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/SEBA/FRHA
Checked By	JRA/DASA/SEBA
Approved By	SJA

						Test Type		NDM 0 <sup>0</sup>			NDM 90 <sup>0</sup>		A	VERAGE NI	м	Solid	Oven	Final Co	orrected	Avorago Air		SI	iear Vane Re	ading (kPa)	)			PASS / FAIL	
URN	Tech.	Date	Location	Layer	Material	NDM / SV		Moisture Content	Air Voids (%)	Wet Density	Moisture Content	Air Voids	Wet Density	Moisture Content	Air Voids (%)	Density (t/m <sup>3</sup> ) Assumed	Moisture content (%)	Density	Pui voida	Voids (10 X Tests)	Reading 1	Reading 2		ading SV	V (4 x	Average SV	Retest URN	(P) Pass	Comments
							(t/m <sup>3</sup> )	(%)		(t/m <sup>3</sup> )	(%)		(t/m <sup>3</sup> )	(%)				(t/m <sup>3</sup> )	(%)							10 X Sets)		(F) Fail	
65.1				RL 54.0		NDM / SV		29.5	6.3	1.81	30.6	6.1	1.82	30.1	6.2	2.70	37.8	1.32	1.2	3.6	156	133			161	164	-	Р	
65.2	JRA /		Fill A - REB			NDM / SV		26.8	1.4	1.98	24.8	1.9	1.97	25.8	1.7	2.70	28.0	1.54	0.0	3.2	UTP	UTP			191.5	166		Р	
65.3	DASA	14/12/2020		RL 53.0	Clay SILT w gravels	NDM / SV		26.4	4.6 7.7	1.90	28.2	3.3	1.90	27.3 21.2	3.9	2.70	31.3	1.45	1.1	2.8	142 UTP	156			170	169	-	P	
65.4	-		Fill Area - 415/422	~1m below FL			1.91	22.0		1.94	20.4	7.3	1.93		7.5	2.70	20.7	1.60	7.9			>199			196	174 175		P	
66.1						NDM / SV	1.99 1.93	24.3 25.4	1.6 4.1	1.99 1.93	23.9 25.0	2.2	1.99 1.93	24.1 25.2	1.9 4.1	2.70 2.70	25.6 31.2	1.59	0.7	2.5	UTP >199	>199 >199			>199 >199	175	-	P	
66.2			Fill Area - 415/422	~0.5m below FL		NDM / SV	1.93	23.4	4.1	1.95	23.0	4.1 0.8	1.95	23.2	4.1	2.70	29.9	1.47	0.0	1.9	>199	>199			199	176		P	
66.3	JRA	16/12/2020		RL 53.20	Clay SILT w gravels	NDM / SV		25.6	5.0	1.94	28.0	3.9	1.94	26.4	4.4	2.70	32.9	1.49	0.0	1.4	182	>199			199	178		P	
66.4			Fill A - REB	RL 53.20	-	NDM / SV		27.8	0.0	1.90	29.0	0.0	1.99	28.4	0.0	2.70	28.8	1.45	0.0	1.3	>199	>199			>199	185		P	
68.1				~RL 65.00		NDM / SV		32.9	4.3	1.90	33.2	5.3	1.81	33.1	4.8	2.70	37.5	1.34	2.0	1.3	151	>216			172	185		P	
68.2	1		_	~RL 60.00	+	NDM / SV		36.2	4.3 6.0	1.80	35.8	6.1	1.01	36.0	4.0 6.0	2.70	34.9	1.31	6.7	1.5	>216	>216			>216	193		P	
68.3	DASA	6/01/2021	Fill A - See Site Plan	~RL 53.00	Clay SILT w gravels	NDM / SV		32.6	0.0	1.75	33.8	0.0	1.75	33.5	0.0	2.70	33.0	1.30	0.0	1.9	>216	>216			197	195		P	
68.4	1			~RL 53.00	ł	NDM / SV		29.6	4.1	1.91	29.0	4.1	1.91	29.3	4.1	2.70	32.5	1.45	1.9	1.9	>216	>216			>216	194		P	
70.5	-			~ RL 63		NDM / SV		25.0	7.5	1.86	25.2	7.5	1.85	25.6	7.5	2.70	22.3	1.41	10.0	2.1	46	65			66	195	74.8	F	
70.6	DASA	11/01/2021	Fill A - See Site Plan	~ RL 63	Sandy SILT w some Clay	NDM / SV		22.4	12.6	1.75	22.9	14.5	1.78	22.7	13.5	2.70	23.8	1.44	12.7	3.3	52	46			47	170	74.9	F	SV < 120 kPa & AV > 12% - Dines informed material would be removed and replaced
70.7	-	,-,	Fill A - REB	~ RL 53	Clay SILT w Gravels	NDM / SV		32.5	7.2	1.77	34.0	6.1	1.77	33.3	6.6	2.70	31.4	1.35	7.8	4.1	170	154			171	167	-	P	
72.1				~ RL 54		NDM / SV	1.79	38.4	2.6	1.80	37.3	2.6	1.79	37.9	2.6	2.70	32.8	1.35	5.7	4.7	170	154			174	165	-	P	
72.2	DASA	12/01/2021	Fill A - REB	~ RL 55	Clay SILT some Sand	NDM / SV		30.5	6.8	1.81	32.4	5.0	1.81	31.5	5.9	2.70	23.3	1.46	11.6	5.8	170	173			168	162		P	
74.8				~0.3m Below FL		NDM / SV		36.0	2.6	1.81	36.0	2.6	1.81	36.0	2.6	2.70	35.0	1.34	3.2	6.2	>199	>199			199	162		Р	Retest of URN 70.5
74.9	JRA	13/01/2021	Fill A - See Site Plan	~FL	Clay SILT some Sand	NDM / SV		23.5	10.4	1.83	23.4	10.3	1.83	23.5	10.3	2.70	25.8	1.45	8.6	6.8	>199	>199			199	165		Р	Retest of URN 70.6
75.5						NDM / SV		34.4	1.3	1.86	34.3	1.1	1.86	34.4	1.2	2.70	32.1	1.41	2.6	6.4	>216	>216			212	165		Р	
75.6	DASA	14/01/2021	Fill A - REB	~RL 54.5	Clay SILT some Sand	NDM / SV		44.3	0.2	1.78	42.3	0.9	1.77	43.3	0.6	2.70	46.7	1.21	0.0	6.4	170	185			185	164		Р	
77.1				RL 54.0		NDM / SV		37.9	1.1	1.83	36.6	1.6	1.82	37.3	1.4	2.70	43.9	1.27	0.0	6.2	154	139			147	157		Р	
77.2	DASA	15/01/2021	Fill A - REB	RL 56.40	Clay SILT w Gravels	NDM / SV	1.82	31.5	5.0	1.82	33.3	4.0	1.82	32.4	4.5	2.70	34.9	1.35	2.9	5.5	>216	>216	>216 >	>216 >	>216	172		Р	
79.10						NDM / SV	1.70	37.0	7.9	1.70	38.4	7.2	1.70	37.7	7.5	2.70	45.7	1.17	3.2	4.6	>199	>199	>199 >	>199 >	>199	187		Р	
79.11	JRA	18/01/2021	Fill A - REB	~RL 56.7	Clay SILT w Gravels	NDM / SV	1.78	36.9	3.8	1.77	36.1	4.6	1.78	36.5	4.2	2.70	41.8	1.25	1.2	3.9	139	159	>199 >	199 1	174	187		Р	
83.1	1					NDM / SV		41.1	0.0	1.82	38.5	0.9	1.82	39.8	0.0	2.70	41.0	1.29	0.0	3.3	>216	>216			>216	191	-	Р	
83.2	DASA	27/01/2021	Fill A - REB	~RL 56	Clay SILT w Gravels	NDM / SV	1.75	41.5	3.1	1.75	41.0	3.1	1.75	41.3	3.1	2.70	35.5	1.29	6.4	2.8	200	185	188	185 1	190	194		Р	
84.1	1					NDM / SV	1.91	28.9	2.2	1.91	30.0	1.3	1.91	29.5	1.7	2.70	25.9	1.52	4.4	2.9	123	126	139	126 1	129	187		Р	
84.2	DASA	29/01/2021	Fill A - REB	~RL 56.5	Clay SILT w Gravels	NDM / SV	1.68	52.2	1.6	1.68	49.4	2.8	1.68	50.8	2.2	2.70	40.2	1.20	7.5	2.8	>216	>216			204	187		Р	
85.1	1			RL 44.6		NDM / SV	1.87	25.4	7.1	1.87	24.0	8.2	1.87	24.7	7.6	2.70	28.6	1.45	4.8	3.0	170	185	173	188 1	179	184		Р	
85.2	DASA	2/02/2021	Fill B	RL 46.6	Clay SILT	NDM / SV	1.80	31.4	6.5	1.80	30.3	7.0	1.80	30.9	6.7	2.70	32.7	1.35	5.5	3.6	185	188	>216 >	>216	201	185		Р	
85.3	1			RL 48.9	1	NDM / SV	1.78	28.3	9.5	1.77	28.4	9.6	1.78	28.4	9.6	2.70	29.5	1.37	8.8	4.5	>216	188	200	200 2	201	191		Р	
86.1	1			~ FL		NDM / SV	1.79	45.7	0.0	1.79	45.7	0.0	1.79	45.7	0.0	2.70	38.1	1.29	2.7	4.5	>216	>216	>216 >	216 >	216	191	-	Р	
86.2	DASA	3/02/2021	Fill C	~ RL 78	Clay SILT	NDM / SV	1.83	30.3	5.3	1.83	29.7	5.8	1.83	30.0	5.5	2.70	31.3	1.40	4.6	4.6	>216	>216	>216 >	>216 >	216	193	-	Р	
88.11					_	NDM / SV	1.79	39.6	1.6	1.80	38.3	2.0	1.80	39.0	1.8	2.70	36.0	1.32	3.6	4.8	>216	>216	>216 >	>216 >	>216	197	-	Р	
88.12	DASA	5/02/2021	Fill C	1m placed	Clay SILT	NDM / SV	1.77	36.6	4.8	1.77	37.0	4.1	1.77	36.8	4.5	2.70	36.1	1.30	4.9	5.3	>216	>216	>216 >	>216 >	216	197	-	Р	
91.1				1m placed		NDM / SV	1.75	32.5	8.4	1.75	31.6	8.7	1.75	32.1	8.6	2.70	30.8	1.34	9.4	5.6	>216	>216	>216	UTP >	>216	199	-	Р	
91.2	DASA	12/02/2021	Fill B	0.5m placed	Clay SILT	NDM / SV	1.61	28.6	17.8	1.61	26.0	19.4	1.61	27.3	18.6	2.70	27.9	1.26	18.2	7.0	>216	>216	>216 >	>216 >	216	208	93.1	F	
L	+		I		1				· · · · · ·													ı – I	I						4



Client: Lander Geotechnical Ltd.

Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/SEBA/FRHA
Checked By	JRA/DASA/SEBA
Approved By	SJA

						Test Type NDM 0 <sup>0</sup>			NDM 90 <sup>0</sup>			AVERAGE NDM			Solid	0.00	Final Co	rrected	.ed		SI	hear Vane F	eading (kP	Pa)			PASS / FAIL		
URN	Tech.	Date	Location	Layer	Material	1014 (5)	Wet	Moisture	Air Voids	Wet Density	Moisture	Air Voids	Wet Density	Moisture	Air Voids	Density (t/m <sup>3</sup> )	Moisture content (%)	Oven Dry	Average	Voids (10 X Tests)	Reading	Reading	Reading F		Average	Average	Retest URN	(P) Pass	Comments
						NDM / SV	Density (t/m <sup>3</sup> )	Content (%)	(%)	(t/m <sup>3</sup> )	Content (%)	(%)	(t/m <sup>3</sup> )	Content (%)	(%)	Assumed	content (70)	Density (t/m <sup>3</sup> )	Air Voids (%)	(10 x 10303)	1	2	3		SV (4 x Tests) (	SV 10 X Sets)		(F) Fail	1
92.1				2m placed		NDM / SV	1.72	34.1	8.8	1.73	32.1	9.5	1.72	33.1	9.2	2.70	35.1	1.28	8.0	7.1	154	170	157	173	164	204	-	Р	
92.2	DASA	17/02/2021	17/02/2021 Fill B	1.5m placed	Clay SILT	NDM / SV	1.57	31.9	17.9	1.57	34.3	16.6	1.57	33.1	17.2	2.70	29.1	1.22	19.5	8.5	>216	>216	>216	>216	>216	208	93.1	F	
92.3	DASA	17/02/2021		1.5m placed	Ciay Sici	NDM / SV	1.70	38.3	7.1	1.70	35.9	8.7	1.70	37.1	7.9	2.70	37.1	1.24	7.9	8.8	170	188	173	176	177	205	-	Р	
92.4				2m placed		NDM / SV	1.79	38.3	2.3	1.80	28.6	8.3	1.80	33.5	5.2	2.70	24.2	1.45	11.5	9.0	170	173	176	173	173	203		Р	
93.1	DASA	18/02/2021	Fill B	2.5m placed	Clay SILT	NDM / SV	1.81	31.9	5.3	1.81	34.6	3.6	1.81	33.3	4.4	2.70	32.0	1.37	5.2	9.3	>216	>216	203	185	205	201		Р	Retest of URN 91.2, 92.2
94.1				~RL 45.2		NDM / SV	1.69	35.7	9.3	1.70	34.7	9.5	1.70	35.2	9.4	2.70	34.1	1.26	10.0	9.8	185	>191	>216	185	194	199		Р	
94.2	DASA	19/02/2021 Fill B	Fill B	~REL 45.2	Clay SILT	NDM / SV	1.79	38.4	2.5	1.78	39.1	2.6	1.78	38.8	2.6	2.70	38.7	1.29	2.6	9.7	170	173	173	182	175	195	-	Р	
94.3				~RL 48		NDM / SV	1.80	38.3	1.7	1.80	37.5	2.3	1.80	37.9	2.0	2.70	48.6	1.21	0.0	9.2	>216	>216	>216	>216	>216	195	-	Р	
94.4				~RL 53		NDM / SV		39.4	3.9	1.76	40.6	2.8	1.76	40.0	3.4	2.70	37.0	1.28	5.1	8.8	170	185		>191	180	191	•	Р	
95.1				RL 54.2		NDM / SV		32.0	6.6	1.79	33.4	5.5	1.79	32.7	6.1	2.70	33.0	1.34	5.9	7.6	154	173	170	154	163	186	-	Р	
95.2	DASA	22/02/2021	Fill B	RL 53.0	Clay SILT	NDM / SV	1.81	38.0	1.7	1.81	37.4	2.0	1.81	37.7	1.9	2.70	51.7	1.19	0.0	6.8	>216	>216	>216		>216	191	-	Р	
95.3				RL 51.0		NDM / SV	1.86	33.1	2.2	1.85	33.2	2.3	1.85	33.2	2.2	2.70	49.2	1.24	0.0	4.8	188	154	154		167	187	-	Р	
96.1	DASA	23/02/2021	REB 1	600mm placed	Clay SILT	NDM / SV	1.74	31.5	9.1	1.76	31.0	8.6	1.75	31.3	8.9	2.70	31.0	1.34	9.0	4.9	>216	>216		_	>216	190	-	Р	
97.1	-			RL 56.6		NDM / SV	1.82	39.3	0.1	1.81	39.5	0.5	1.82	39.4	0.3	2.70	38.1	1.32	1.1	3.9	>216	>216			>216	195	-	P	
97.2 97.3	DASA	24/02/2021	REB 2	RL 54.5	Clay SILT	NDM / SV	1.85	30.0 35.5	4.7	1.86	26.5 37.7	6.5	1.86	28.3	5.6	2.70	37.4	1.35	0.0	3.4	>216 >216	>216 >216	>216	_	>216	196 198	-	Р	
97.3				RL 46.6		NDM / SV	1.83	35.5	2.2	1.83	37.7	0.9 2.5	1.83	36.6 32.4	1.6 2.6	2.70	32.4 36.7	1.38	4.2 0.0	2.8	>216	>216			214 >216	202	-	P	
97.4				RL 46.6		NDM / SV	1.85	32.2	0.0	1.86	32.5	0.3	1.86	32.4	0.0	2.70	36.7	1.36	0.0	2.5	185	170	185		178	198		P	
98.2	DASA	26/02/2021	/2021 REB 2	RL 56.0	Clay SILT	NDM / SV		40.0	0.0	1.83	43.4	0.0	1.84	41.7	0.0	2.70	43.1	1.30	0.0	2.0	>216	>216	185	_	205	201		P	
98.3	- Chart	20/02/2021		RL 46.8		NDM / SV	1.69	39.0	7.6	1.69	40.9	6.3	1.69	40.0	7.0	2.70	38.0	1.23	8.0	2.2	>216	>216	>216		>216	206		Р	
99.1						NDM / SV	1.92	34.9	0.0	1.92	35.3	0.0	1.92	35.1	0.0	2.70	34.6	1.43	0.0	2.2	170	173			194	204	-	P	
99.2				RL 56.5 RL 47.6	Clay SILT	NDM / SV	1.71	38.4	6.7	1.71	37.3	7.3	1.71	37.9	7.0	2.70	40.5	1.22	5.5	2.8	>216	>216			>216	209		р	
99.3	DASA	DASA 1/03/2021 REB 2	REB 2			NDM / SV	1.80	34.2	4.5	1.79	33.0	5.5	1.80	33.6	5.0	2.70	46.9	1.22	0.0	1.9	170	173	>216	>216	194	206		Р	
99.4						NDM / SV	1.79	33.2	5.5	1.77	31.3	8.0	1.78	32.3	6.8	2.70	41.0	1.26	1.5	1.9	>216	>216	>216	>216	>216	206		Р	
100.1				RL 48.4		NDM / SV	1.75	40.1	3.6	1.73	41.7	4.0	1.74	40.9	3.8	2.70	45.2	1.20	1.5	2.1	62	74	92	99	82	193	101.2	F	SV < 120 kPa
100.2				RL 49.20	Clay SILT	NDM / SV	1.66	44.0	6.9	1.64	46.1	6.6	1.65	45.1	6.7	2.70	43.6	1.15	7.5	2.4	>216	>216	>216	>216	>216	193		Р	
100.3	DASA	A 8/03/2021 REB 2	050.2	RL 48.4		NDM / SV	1.66	46.1	5.3	1.67	46.7	4.7	1.67	46.4	5.0	2.70	46.2	1.14	5.1	2.9	170	173	170	182	174	189	-	Р	
100.4	DASA		NED 2	RL 48.4		NDM / SV	1.72	46.3	2.1	1.72	48.0	1.4	1.72	47.2	1.8	2.70	37.3	1.25	7.0	3.6	170	173	170	182	174	189	•	Р	
100.5				RL 58		NDM / SV	1.76	42.0	2.1	1.77	41.8	1.6	1.76	41.9	1.9	2.70	42.2	1.24	1.7	3.8	>216	216	>216	>216	167	185	•	Р	
100.6				RL 58.5		NDM / SV	1.76	37.4	4.6	1.75	38.7	4.5	1.76	38.1	4.5	2.70	40.0	1.25	3.4	3.3	170	166	173	185	174	181	-	Р	
101.1			REB 2	RL 50		NDM / SV	1.77	37.8	3.7	1.78	37.9	3.4	1.78	37.9	3.5	2.70	44.4	1.23	0.0	3.3	173	167	170	196	177	179	-	Р	
101.2	JRA			RL 48.4 Cl	Clay SILT	NDM / SV	1.77	38.1	3.7	1.78	37.4	3.5	1.78	37.8	3.6	2.70	38.1	1.29	3.4	3.1	>199	>199	>199	>199	>199	177	-	Р	Retest of URN 100.1
101.2			Slip" Area above Pond	RL 48.8 (~FL)		NDM / SV	1.74	37.8	5.6	1.72	41.3	4.4	1.73	39.6	5.0	2.70	35.6	1.28	7.3	3.8	156	>199	179	>199	183	176	-	Р	
102.1					chu chi T	NDM / SV	1.79	35.1	4.6	1.78	34.4	5.3	1.78	34.8	4.9	2.70	33.9	1.33	5.5	4.2	>216	>216	>216	>216	>216	176	-	Р	
102.2				RL 50.40		NDM / SV	1.80	31.6	6.1	1.81	30.4	6.4	1.81	31.0	6.2	2.70	39.6	1.29	0.9	4.2	>216	>216	>216	>216	>216	190	-	Р	
102.3	DASA 12/03/2021 REB 2	REB 2	RL 57.20	Clay SILT	NDM / SV	1.84	35.4	1.3	1.82	36.8	2.0	1.83	36.1	1.7	2.70	42.8	1.28	0.0	3.4	>216	>216	>216	>216	>216	190	-	Р		
102.4				RL 59.00		NDM / SV	1.82	38.2	0.9	1.78	37.7	3.4	1.80	38.0	2.1	2.70	37.3	1.31	2.5	3.2	>216	UTP	>216	UTP	>216	194	-	Р	
103.1	DASA	16/03/2021	21 REB 2	RL 51.60	Clay SILT	NDM / SV	1.87	33.8	1.2	1.87	33.8	1.1	1.87	33.8	1.2	2.70	39.1	1.34	0.0	2.5	154	170	166	163	163	193	-	Р	
103.2	DAJA	10/03/2021	NED Z	NE 51.00	Clay SILI	NDM / SV	1.67	47.6	4.4	1.65	47.7	5.2	1.66	47.7	4.8	2.70	52.5	1.09	2.5	2.6	>216	>216	>216	>216	>216	198		Р	
104.1				RL 51.59		NDM / SV	1.78	34.2	5.5	1.78	34.2	5.7	1.78	34.2	5.6	2.70	37.3	1.30	3.7	2.6	170	173	>216	>216	194	199	-	Р	
104.2	DASA	SA 18/03/2021 REB2	RL 51.93	Clay SILT	NDM / SV	1.76	33.4	7.1	1.76	32.1	8.1	1.76	32.8	7.6	2.70	38.9	1.26	3.9	3.0	108	111	99	108	107	193	105.1	F		
104.3			~RL 58.75	city size	NDM / SV	1.80	36.6	2.9	1.79	37.8	2.8	1.80	37.2	2.9	2.70	42.8	1.26	0.0	2.6	108	92	111	114	106	183	105.2	F	4	
104.4					NDM / SV	1.84	28.6	6.3	1.84	30.5	4.9	1.84	29.6	5.6	2.70	33.6	1.38	2.9	2.2	154	>216	>216	188	194	184	-	Р		



## Job: Hitchens Block Stage 5

Client: Lander Geotechnical Ltd.

Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/SEBA/FRHA
Checked By	JRA/DASA/SEBA
Approved By	SJA

						Test Type		NDM 00			NDM 90 <sup>0</sup>		A	VERAGE ND	м	Solid	Oven	Final (	Corrected	Average Air		s	hear Vane	Reading (	(kPa)			PASS / FAIL	
URN	Tech.	Date	Location	Layer	Material	NDM / SV	Wet Density (t/m <sup>3</sup> )	Moisture Content (%)	Air Voids (%)	Wet Density (t/m <sup>3</sup> )	Moisture Content (%)	Air Voids (%)	Wet Density (t/m <sup>3</sup> )	Moisture Content (%)	Air Voids (%)	Density (t/m <sup>3</sup> ) Assumed	Moisture content (%)	Oven Dry Density (t/m <sup>3</sup> )	Average Air Voids (%)		Reading 1	Reading 2	Reading 3	Reading 4	Average SV (4 x Tests)	Average SV (10 X Sets)	Retest URN	(P) Pass (F) Fail	Comments
105.1				RL 51.93		NDM / SV	1.53	23.0	25.3	1.55	23.8	23.9	1.54	23.4	24.6	2.70	24.3	1.24	24.0	-	154	166	157	170	162	179	106.1	F	Retest of URN 104.2
105.2	DASA	19/03/2021	REB2	RL 51.38	Clay SILT	NDM / SV	1.82	37.0	1.5	1.81	35.5	2.9	1.82	36.3	2.2	2.70	38.1	1.32	1.1	4.1	>216	>216	>216	>216	>216	179	-	Р	Retest of URN 105.1
105.3				RL 58.75		NDM / SV	1.74	46.9	0.7	1.74	44.7	1.9	1.74	45.8	1.3	2.70	52.2	1.14	0.0	4.1	170	166	157	160	163	174	-	Р	
106.1	JRA	22/03/2021	REB 2	RL 51.6	Clay SILT	NDM / SV	1.82	35.1	2.6	1.82	36.1	2.2	1.82	35.6	2.4	2.70	33.2	1.37	3.9	4.2	159	>199	173	167	175	169	-	Р	
107.1						NDM / SV	1.78	35.8	4.3	1.77	34.0	6.2	1.78	34.9	5.2	2.70	39.8	1.27	2.3	4.4	185	182	170	>191	182	171	-	Р	
107.2	DASA	24/03/2021	REB2	RL ~53.2	Clay SILT	NDM / SV	1.75	42.7	2.3	1.74	43.2	2.8	1.74	43.0	2.5	2.70	37.8	1.26	5.4	4.7	>216	>216	>216	>216	>216	171	-	Р	
109.1		/ /		RL 54.10		NDM / SV	1.78	36.4	4.0	1.77	36.4	4.7	1.78	36.4	4.3	2.70	33.2	1.33	6.3	5.0	>216	>216	>216	>216	>216	174	-	Р	
109.2	DASA	26/03/2021	REB2	RL 54.00	Clay SILT	NDM / SV	1.80	33.4	5.1	1.80	31.7	6.3	1.80	32.6	5.7	2.70	45.2	1.24	0.0	4.6	166	170	148	163	162	179	-	Р	
110.1						NDM / SV	1.73	35.0	7.5	1.74	36.1	6.3	1.74	35.6	6.9	2.70	37.1	1.27	6.0	5.2	>216	>216	>216	>216	>216	190	-	Р	
110.2		- / /		~RL 54.70		NDM / SV	1.77	31.2	7.7	1.76	31.3	8.4	1.77	31.3	8.1	2.70	39.1	1.27	3.3	5.2	>216	>216	>216	>216	>216	192	-	Р	
110.3	DASA	7/04/2021		-01.57.40	Clay SILT	NDM / SV	1.67	41.0	7.6	1.64	41.4	9.0	1.66	41.2	8.3	2.70	47.3	1.12	5.2	3.4	>216	>216	170	173	194	196	-	Р	
110.4				~RL 57.40		NDM / SV	1.77	38.6	3.4	1.747	37.6	5.2	1.76	38.1	4.3	2.70	42.1	1.24	2.0	3.4	>216	>216	>216	>216	>216	196	-	Р	
111.1						NDM / SV	1.78	29.9	8.2	1.761	29.5	9.5	1.77	29.7	8.8	2.70	42.7	1.24	1.0	3.5	>216	>216	>216	>216	>216	201	-	Р	
111.2			REB2	~1m placed	Clay SILT	NDM / SV	1.87	32.4	2.0	1.861	33.0	2.0	1.87	32.7	2.0	2.70	48.5	1.26	0.0	3.2	>216	>216	>216	>216	>216	205	-	Р	
111.3	DASA	9/04/2021				NDM / SV	1.79	27.6	9.5	1.871	26.0	6.4	1.83	26.8	7.9	2.70	32.2	1.38	4.2	3.3	185	188	200	197	193	206	-	Р	
111.4						NDM / SV	1.80	27.5	8.9	1.81	27.3	8.5	1.80	27.4	8.7	2.70	46.1	1.24	0.0	2.8	>216	>216	>216	>216	>216	206	-	Р	
111.5			Pond Fill	~FL	Clay SILT	NDM / SV	1.86	29.4	4.7	1.822	28.3	7.2	1.84	28.9	6.0	2.70	40.9	1.31	0.0	2.2	>216	>216	>216	>216	>216	206	-	Р	
112.1		40/04/2024	0500	-000	ci	NDM / SV	1.92	30.7	0.4	1.92	31.2	0.1	1.92	31.0	0.3	2.70	33.1	1.44	0.0	2.2	>216	>216	>216	>216	>216	211		Р	
112.2	DASA	19/04/2021	REB2	~800mm from FL	Clay SILT	NDM / SV	1.90	31.4	1.0	1.877	30.4	2.9	1.89	30.9	1.9	2.70	33.0	1.42	0.5	1.6	>216	>216	>216	>216	>216	211	-	Р	
113.1		22/04/2024	0500	~RL 64	ci	NDM / SV	1.80	31.0	6.6	1.80	30.6	7.0	1.80	30.8	6.8	2.70	38.0	1.30	2.3	1.5	185	188	>216	>216	201	210	-	Р	
113.2	DASA	23/04/2021	REB2	~RL 62	Clay SILT	NDM / SV	1.88	31.6	1.8	1.89	31.5	1.6	1.89	31.6	1.7	2.70	35.2	1.39	0.0	1.0	>216	>216	>216	>216	>216	212	-	Р	
114.1	DASA	20/04/2024	Pond	~RL 61.0	ci	NDM / SV	1.76	33.9	6.7	1.76	31.7	8.3	1.76	32.8	7.5	2.70	36.2	1.29	5.4	1.3	185	173	>216	>216	198	210	-	Р	Pond embankment specification provided by Chris (Lander) requires maximum air voids <8%,
114.2	DASA	30/04/2021	Embankment	~RL 56.0	Clay SILT	NDM / SV	1.78	34.3	5.7	1.78	31.6	7.3	1.78	33.0	6.5	2.70	36.8	1.30	4.1	1.7	170	154	173	185	171	206	-	Р	average less than 6%, Shear strength average >140 kPa with a minimum single value of >120 kPa.
115.1		1/05/2024	0500	RL 48.68	ci	NDM / SV	1.72	35.5	7.7	1.74	33.3	8.4	1.73	34.4	8.0	2.70	39.3	1.24	5.2	2.2	170	188	191	200	>187	203	-	Р	
115.2	DASA	4/05/2021	REB2	~ RL 47	Clay SILT	NDM / SV	1.84	32.7	3.4	1.80	31.9	5.8	1.82	32.3	4.6	2.70	45.2	1.25	0.0	1.8	216	216	188	197	204	204	-	Р	



## Job: Hitchens Block Stage 5

Customer: Lander	Geotechnical Ltd
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Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/FRHA
Checked By	SEBA/DASA/JRA
Approved By	SJA

											Nuclear De	nsity (Backscat	ter)					PASS / FAIL	
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m <sup>3</sup> )	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments
1.1						-	-	-	-	-	-	-	-	-	-	10	2.1	F	
1.2						-	-	-	-	-	-	-	-	-	-	11	2.2	F	
1.3	JRA	47/40/2040	600		DI 40.70	-	-	-	-	-	-	-	-	-	-	10	2.3	F	Specification of CIV $\ge$ 20 provided by Kyle (Lander). CIV < 20 - Fail. Hardfill had only been track rolled -
1.4	JRA	17/10/2019	SPR	Fill F Toe Key - Hardfill	RL 40.78	-	-	-	-	-	-	-	-	-	-	12	2.4	F	Kyle Informed & to be retested tomorrow.
1.5						-	-	-	-	-	-	-	-	-	-	10	2.5	F	
1.6						-	-	-	-	-	-	-	-	-	-	10	2.6	F	
2.1						-	-	-	-	-	-	-	-	-	-	22	-	Р	Retest of URN 1.1 once compacted with plate compactor
2.2						-	-	-	-	-	-	-	-	-	-	21	-	Р	Retest of URN 1.2 once compacted with plate compactor
2.3	JRA	18/10/2019	SPR	Fill F Toe Key - Hardfill	RL 40.78	-	-	-	-	-	-	-	-	-	-	22	-	Р	Retest of URN 1.3 once compacted with plate compactor
2.4	JIVA	10/10/2015	JFK	rin r toe key - narunn	NL 40.78	-	-	-	-	-	-	-	-	-	-	20	-	Р	Retest of URN 1.4 once compacted with plate compactor
2.5						-	-	-	-	-	-	-	-	-	-	20	-	Р	Retest of URN 1.5 once compacted with plate compactor
2.6						-	-	-	-	-	-	-	-	-	-	20	-	Р	Retest of URN 1.6 once compacted with plate compactor
10.1						-	-	-	-	-	-	-	-	-	-	31	-	Р	
10.2						-	-	-	-	-	-	-	-	-	-	25	-	Р	
10.3	JRA	21/11/2019	SPR	Fill 0 Pond Toe Key -	Top of Hardfill	-	-	-	-	-	-	-	-	-	-	21	-	Р	
10.4	5101	21, 11, 2015	5.11	Fill 0 Pond Toe Key - Hardfill	rop of Hardini	-	-	-	-	-	-	-	-	-	-	26	-	Р	
10.5						-	-	-	-	-	-	-	-	-	-	29	-	Р	
10.6						-	-	-	-	-	-	-	-	-	-	30	-	Р	
34.1						-	-	2.26	2.14	5.8	2.09	102.2%	-	-	-	55	-	Р	Backscatter specification of 95% MDD provided by Kyle (Lander)
34.2						-	-	2.20	2.12	4.1	2.09	101.2%	-	-	-	48	-	Р	
34.3	FRHA	20/02/2020	SPR	Fill A - Shear Key	0.6m Placed	-	-	2.20	2.09	5.1	2.09	99.9%	-	-	-	28	-	Р	
34.4						-	-	-	-	-	-	-	-	-	-	20	-	Р	
34.5						-	-	-	-	-	-	-	-	-	-	43	-	Р	
34.6						-	-	-	-	-	-	-	-	-	-	37	-	Р	
35.1						-	-	2.21	2.06	7.3	2.09	98.7%	-	-	-	28	-	Р	
35.2						-	-	2.28	2.12	7.1	2.09	101.6%	-	-	-	23	-	Р	
35.3						-	-	2.20	2.10	4.7	2.09	100.5%	-	-	-	45	-	Р	
35.4	FRHA	21/02/2020	SPR Fill A - Shear Key	1.2m Placed	-	-	2.25	2.15	4.9	2.09	102.8%	-	-	-	50	-	Р		
35.5		.,,				-	-	-	-	•	-	-	•	-	-	26	-	Р	
35.6						-	-	-	-		-	-		-	-	37	-	Р	
35.7						-	-	-	-	-	-	-	-	-	-	37	-	Р	
35.8						-	-	-	-	-	-	-	-	-	-	41	-	Р	



 Job : Hitchens Block Stage 5
 Job #
 1009213.0657.0.0/3

 ICS
 Entered By
 DASA/JRA/FRHA

 Customer: Lander Geotechnical Ltd
 Checked By
 SEBA/DASA/JRA

 Approved By
 SJA

											Nuclear De	nsity (Backscat	ter)					PASS / FAIL	
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m <sup>3</sup> )	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments
36.1						-	-	2.12	2.02	5.2	2.09	96.6%	-	-	-	36	-	Р	
36.2						-	-	2.15	2.05	5.0	2.09	98.0%	-	-	-	31	-	Р	
36.3						-	-	2.13	2.02	5.0	2.09	96.9%	-	-	-	48	-	Р	
36.4	FRHA	24/02/2020	SPR	Fill A - Shear Key	1.8m Placed	-	-	2.12	2.03	4.3	2.09	97.3%	-	-	-	28	-	Р	
36.5	_					-	-	-	-	-	-	-	-	-	-	52	-	Р	
36.6						-	-	-	-	-	-	-	-	-	-	61	-	Р	
36.7						-	-	-	-	-	-	-	-	-	-	42	-	Р	
36.8						-	-	-	-	-	-	-	-	-	-	26	-	Р	
37.1						-	-	2.16	2.04	5.7	2.09	97.6%	-	-	-	26	-	Р	
37.2						-	-	2.20	2.06	6.6	2.09	98.6%	-	-	-	30	-	Р	
37.3						-	-	2.19	2.07	5.7	2.09	98.9%	-	-	-	32	-	Р	
37.4	FRHA	25/02/2020	SPR	Fill A - Shear Key	2.4m Placed	-	-	2.20	2.05	7.6	2.09	97.9%	-	-	-	31	-	Р	
37.5						-	-	-	-	-	•	-	-	-	-	32	-	Р	
37.6						-	-	-	-	-	-	-	-	-	-	28	-	Р	
37.7						-	-	-	-	-	-	-	-	-	-	41	-	Р	
37.8						-	-	-	-	-	-	-	-	-	-	38	-	Р	
38.1						-	-	2.24	2.11	6.2	2.09	101.0%	-	-	-	36	-	Р	
38.2						-	-	2.26	2.12	6.9	2.09	101.3%	-	-	-	31	-	Р	
38.3						-	-	2.21	2.10	5.5	2.09	100.3%	-	-	-	40	-	Р	
38.4	FRHA	25/02/2020	SPR	Fill A - Shear Key	3.0m Placed	-	-	2.13	2.02	5.2	2.09	96.7%	-	-	-	28	-	Р	
38.5	_					-	-	-	-	-	-	-	-	-	-	29	-	Р	
38.6						-	-	-	-	-	-	-	-	-	-	33	-	Р	
38.7						-	-	-	-	-	-	-	-	-	-	26	-	Р	
38.8						-	-	-	-	-	-	-	-	-	-	30	-	Р	
40.1						-	-	2.13	2.01	5.6	2.09	96.4%	-	-	-	49	-	Р	
40.2						-	-	2.21	2.08	6.2	2.09	99.4%	-	-	-	57	-	Р	
40.3						-	-	2.09	2.01	4.4	2.09	95.9%	-	-	-	60	-	Р	
40.4	FRHA	26/02/2020	SPR	SPR Fill A - Shear Key	3.6m Placed	-	-	2.16	2.05	5.3	2.09	98.2%	-	-	-	30	-	Р	
40.5						-	-	-	-	-	-	-	-	-	-	36	-	Р	
40.6						-	-	-	-	-	-	-	-	-	-	34	-	Р	
40.7						-	-	-	-	-	-	-	-	-	-	40	-	Р	
40.8						-	-	-	-		-	-	-	-	-	41	-	Р	



## Job : Hitchens Block Stage 5 Job # 1009213.0657.0.0/3 CUstomer: Lander Geotechnical Ltd Entered By DASA/JRA/FRHA Checked By SEBA/DASA/JRA Approved By SJA

											Nuclear De	nsity (Backsca	tter)					PASS / FAIL	
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m³)	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments
41.1	1					-	-	2.19	2.08	5.7	2.09	99.3%	-	-	-	27	-	Р	
41.2						-	-	2.22	2.05	8.5	2.09	98.0%	-	-	-	27	-	Р	
41.3						-	-	2.24	2.13	5.1	2.09	102.0%	-	-	-	51	-	Р	
41.4	FRHA	27/02/2020	SPR	Fill A - Shear Key	4.2m Placed	-	-	2.21	2.08	6.4	2.09	99.4%	-	-	-	38	-	Р	
41.5	4			,		-	-	-	-	-	-	-	-	-	-	40	-	Р	
41.6	_					-	-	-	-	-	-	-	-	-	-	40	-	Р	
41.7	4					-	-	-	-	-	-	-	-	-	-	36	-	Р	
41.8						-	-	-	-	-	-	-	-	-	-	37	-	Р	
42.1						-	-	2.14	2.01	6.2	2.09	96.4%	-	-	-	32	-	Р	
42.2	4					-	-	2.12	2.01	5.5	2.09	96.2%	-	-	-	20	-	Р	
42.3	4					-	-	2.23	2.08	7.1	2.09	99.5%	-	-	-	23	-	Р	
42.4	FRHA	2/03/2020	SPR	Fill A - Shear Key	4.8m Placed	-	-	2.22	2.09	6.2	2.09	99.9%	-	-	-	28	-	Р	
42.5	4					-	-	-	•	-	-	-	-	-	-	28	-	Р	
42.6	4					-	-	-	-	-	-	-	-	-	-	29	-	Р	
42.7	4					-	-	-	•	-	-	-	-	-	-	27	-	Р	
42.8						-	-	-	•	•	-	-	-	-	-	29	-	Ρ	
44.1	4					-	-	2.23	2.07	7.6	2.09	99.3%	-	-	-	26	-	Р	
44.2	4					-	-	2.20	2.06	6.9	2.09	98.3%	-	-	-	29	•	Р	
44.3	-					-	-	2.15	2.02	6.8	2.09	96.5%	-	-	-	21	•	Р	
44.4	FRHA	13/03/2020	SPR	Fill A - Shear Key	3.6m Placed	-	-	2.12	2.00	6.3	2.09	95.6%	-	-	-	20	•	Р	
44.5	4					-	-	-	-	•	-	-	•	-	-	24	-	P	
44.6	-					-	-	-	-	-	-	-	•	-	-	37	-	P	
44.7	-					-	-	-	-	•	-	-	•	-	-	24	-		
44.8						-	-	-	-		-	-	•	-	-	20 32	-	P	
45.1	-							2.16	2.03	6.2	2.09	97.3%			-			P	
45.2 45.3	-					-	-	2.19 2.16	2.07 2.03	5.6 6.3	2.09	99.2% 97.2%	-		-	42 35	-	Р Р	
45.4	-								2.03	6.8	2.09					30		Р Р	
45.4	-					-	-	2.18 2.18	2.04	7.5	2.09	97.8% 96.8%	-	-	-	43	-	Р Р	
45.5	FRHA	16/03/2020	020 SPR	Fill A - Shear Key	5.4m Placed	-	-	2.18	2.02	4.8	2.09	96.8%	-	-	-	43	-	P	
45.7	+					-	-	-	-	4.0	-	-				42 27		P	
45.8	+					-	-					-				35		Р Р	
45.9	1					-	-	-			-	-				33	-	Р Р	
45.9	1					-	-				-	-		-	-	40	-	Р Р	
45.10	1					-		-		-	-		-	-	-	40		r	



	Job : Hitchens Block Stage 5	Job #	1009213.0657.0.0/3
INICS		Entered By	DASA/JRA/FRHA
	Customer: Lander Geotechnical Ltd	Checked By	SEBA/DASA/JRA
		Approved By	SJA

											Nuclear De	nsity (Backsca	tter)					PASS / FAIL		
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m <sup>3</sup> )	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments	
48.1						-	-	2.17	2.04	6.5	2.09	97.5%	-	-	-	27	-	Р		
48.2						-	-	2.34	2.15	8.7	2.09	103.1%	-	-	-	35	-	Р		
48.3						-	-	2.13	2.03	4.7	2.09	97.1%	-	-	-	23	-	Р		
48.4						-	-	2.16	2.00	7.6	2.09	95.9%	-	-	-	25	-	Р		
48.5	FRHA	13/05/2020	SPR	Fill A - Shear Key	6.0m Placed	-	-	2.21	2.04	8.5	2.09	97.4%	-	-	-	20	-	Р		
48.6						-	-	2.31	2.11	9.6	2.09	100.8%	-	-	-	22	-	Р		
48.7						-	-	-	-	-	-	-	-	-	-	23	-	Р		
48.8						-	-	-	-	-	-	-	-	-	-	21	-	Р		
48.9						-	-	-	-	-	-	-	-	-	-	20	-	Р		
50.1						-	-	2.12	2.02	4.8	2.09	96.8%	-	-	-	28	-	Р		
50.2						-	-	2.28	2.16	5.5	2.09	103.5%	-	-	-	28	-	Р		
50.3						-	-	2.21	2.13	4.0	2.09	101.7%	-	-	-	38	-	Р		
50.4						-	-	2.15	2.06	4.0	2.09	98.8%	-	-	-	36	-	Р		
50.5						-	-	2.09	2.00	4.2	2.09	95.9%	-	-	-	24	-	Р		
50.6						-	-	2.19	2.08	5.0	2.09	99.6%	-	-	-	28	-	Р		
50.7						-	-	-	-	-	-	-	-	-	-	25	-	Р		
50.8	FRHA	18/05/2020	SPR	Fill A - Shear Key	6.4m placed	-	-	-	-	-	-	-	-	-	-	43	-	Р		
50.9						-	-	-	-	-	-	-	-	-	-	36	-	Р		
50.1						-	-	-	-	-	-	-	-	-	-	32	-	Р		
50.11						-	-	-	-	-	-	-	-	-	-	24	-	Р		
50.12						-	-	-	-	-	-	-	-	-	-	36	-	Р		
50.13						-	-	-	-	-	-	-	-	-	-	33	-	Р		
50.14						-	-	-	-	-	-	-	-	-	-	33	-	Р		
50.15						-	-	-	-	-	-	-	-	-	-	30	-	Р		
51.1						-	-	-	-	-	-	-	-	-	-	34	-	Р		
51.2						-	-	-	-	-	-	-	-	-	-	38	-	Р		
51.3	FRHA	17/06/2020	GAP65	P65 Fill A - Shear Key	~7.4m placed	-	-	-	-	-	-	-	-	-		26	-	Р	CIV Value of >20 requested by Kyle (Lander) on the 16/06/2020	
51.4	1					-	-	-	-	-	-	-	-			30	-	Р		
67.1						-	-	2.23	2.07	7.9	2.09	99.0%	-	-	-	23	-	Р		
67.2	1				~1m placed	-	-	-	-	-	-	-	-	-	-	20	-	Р		
67.3	JRA	17/12/2020	SPR	Fill B - REB Shear Key		-	-	2.18	2.02	7.7	2.09	96.8%	-	-	-	26	-	Р		
67.4	1			FIII B - KEB Snear Key		~2m placed	-	-	2.24	2.10	6.7	2.09	100.5%	-	-	-	34	-	Р	
67.5						-	-	2.24	2.07	8.5	2.09	98.9%	-	-	-	31	-	Р		



	Job : Hitchens Block Stage 5	Job #	1009213.0657.0.0/3
INICS		Entered By	DASA/JRA/FRHA
	Customer: Lander Geotechnical Ltd	Checked By	SEBA/DASA/JRA
		Approved By	SJA

											Nuclear De	nsity (Backsca	ter)					PASS / FAIL	
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m <sup>3</sup> )	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments
69.1						-	-	2.30	2.16	6.4	2.09	103.3%	-	-	-	21	-	Р	
69.2						-	-	2.23	2.05	8.6	2.09	98.2%	-	-	-	23	-	Р	
69.3						-	-	-	-	-	-	-	-	-	-	27	-	Р	
69.4						-	-	-	-	-	-	-	-	-	-	18		F	
69.5	DASA	11/01/2021	SPR	Fill B - REB Shear Key	~3m placed	-	-	-	-	-	-	-	-	-	-	16	70.1 - 70.4	F	
69.6		,,				-	-	-	-	-	-	-	-	-	-	14		F	
69.7						-	-	-	-	-	-	-	-	-	-	14		F	
69.8						-	-	-	-	-	-	-	-	-	-	22	-	Р	
69.9						-	-	-	-	-	-	-	-	-	-	30	-	Р	
69.10						-	-	-	-	-	-	-	-	-	-	28	-	Р	
70.1						-	-	2.24	2.07	8.0	2.09	99.3%	-	-	-	25	-	Р	
70.2	DASA	11/01/2021	SPR	Fill B - REB Shear Key	~3m placed	-	-	2.35	2.17	8.3	2.09	103.9%	-	-	-	30	-	Р	Retest of URN 69.4 - 69.7
70.3				Fill B - REB Shear Key		-	-	-	-	-	-	-	-	-	-	26	-	Р	
70.4						-	-	-	-	-	-	-	-	-	-	28	-	Р	
71.1						-	-	2.09	1.92	8.9	2.09	91.7%	-	-	-	-	71.2	F	
71.2						-	-	2.23	2.05	8.7	2.09	98.1%	-	-	-	-	-	Р	Retest of URN 71.1
71.3	DASA	12/01/2021	SPR	Fill B - REB Shear Key	~2.6 below FL	-	-	2.22	2.03	9.8	2.09	96.9%	-	-	-	-	-	Р	
71.4						-	-	2.13	1.95	9.3	2.09	93.3%	-	-	-	-	71.5	F	
71.5						-	-	2.23	2.06	8.2	2.09	98.7%	-	-	-	-	-	Р	Retest of URN 71.4
72.3						-	-	2.03	1.87	8.8	2.09	89.4%	-	-	-	21		F	
72.4						-	-	2.10	1.91	9.8	2.09	91.5%	-	-	-	22	72.6 - 72.8	F	
72.5						-	-	2.08	1.92	8.0	2.09	91.9%	-	-	-	21		F	
72.6					~0.5m placed	-	-	1.70	1.52	11.8	2.09	72.8%	-	-	-	24		F	
72.7	DASA	12/01/2021	SPR	Fill B - REB Shear Key	(Hardfill extension	-	-	1.79	1.63	10.0	2.09	77.9%	-	-	-	27	73.1 - 73.2	F	
72.8	5, GA	12,01/2021	5. K	The shee shear key	where old sump was)	-	-	1.83	1.68	8.9	2.09	80.3%	-	-	-	21		F	
72.9					,	-	-	-	-	-	-	-	-	-	-	30	-	Р	
72.10						-	-	-	-	-	-	-	-	-	-	26	-	Р	
72.11						-	-	-	-	-	-	-	-	-	-	27	-	Р	
72.12							-	-	-	-	-	-	-	-	-	30	-	Р	



## Job: Hitchens Block Stage 5

Customer: Lander Geotechnical Ltd	
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Job #	1009213.0657.0.0/3
Entered By	DASA/JRA/FRHA
Checked By	SEBA/DASA/JRA
Approved By	SJA

											Nuclear De	nsity (Backscat	ter)					PASS / FAIL	
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m <sup>3</sup> )	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments
73.1					~0.5m placed	-	-	2.17	2.04	6.7	2.09	97.4%	-	-	-	22	-	Р	Retest of URN 72.3 - 72.8
73.2					(Hardfill extension	-	-	2.17	2.04	6.6	2.09	97.4%	-	-	-	24	-	Р	
73.3					where old sump was)	-	-	-	-	-	-	-	-	-	-	22	-	Р	
73.4	DASA	13/01/2021	SPR	Fill B - REB Shear Key	,	-	-	-	-	-	-	-	-	-	-	27	-	Р	
73.5	571571	10/01/2021	5111	The shear key		-	-	2.18	2.05	6.5	2.09	98.1%	-	-	-	30	-	Р	
73.6					~2.1m below FL	-	-	2.32	2.16	7.4	2.09	103.5%	-	-	-	28	-	Р	
73.7						-	-	-	-	-	-	-	-	-	-	26	-	Р	
73.8						-	-	-	-	-	-	-	-	-	-	28	-	Р	
74.1						-	-	2.28	2.10	8.2	2.09	100.7%	-	-	-	34	-	Р	
74.2					~2.1m below FL	-	-	2.40	2.24	7.3	2.09	107.2%	-	-	-	40	-	Р	
74.3						-	-	2.35	2.16	9.1	2.09	103.2%	-	-	-	36	-	Р	
74.4	JRA	13/01/2021	SPR	Fill B - REB Shear Key	~1m placed (lower	-	-	2.35	2.20	7.0	2.09	105.2%	-	-	-	36	-	Р	
74.5					extension area)	-	-	2.23	2.06	8.3	2.09	98.6%	-	-	-	30	-	Р	
74.6					~1.5m placed (lower	-	-	2.31	2.14	7.8	2.09	102.5%	-	-	-	32	-	Р	
74.7					extension area)	-	-	2.36	2.21	7.1	2.09	105.5%	-	-	-	39	-	Р	
75.1						-	-	2.22	2.07	7.4	2.09	99.1%	-	-	-	28	-	Р	
75.2	DASA	14/01/2021	SPR	Fill B - REB Shear Key	~2.6m below FL	-	-	2.24	2.05	9.1	2.09	98.0%	-	-	-	30	-	Р	
75.3	DASA	14/01/2021	JFK	FILD - KED SHEAL KEY	(Extension area)	-	-	-	-	-	-	-	-	-	-	27	-	Р	
75.4						-	-	-	-	-	-	-	-	-	-	26	-	Р	
76.1						-	-	2.31	2.14	7.8	2.09	102.4%	-	-	-	32	-	Р	
76.2	DASA	14/01/2021	SPR	Fill B - REB Shear Key	~2.1m below FL	-	-	2.29	2.10	9.1	2.09	100.6%	-	-	-	28	-	Р	
76.3	DASA	14/01/2021	SPR	FIII B - REB SHear Key	(Extension Area)	-	-	-	-	-	-	-	-	-	-	26	-	Р	
76.4						-	-	-	-	-	-	-	-	-	-	30	-	Р	
78.1						-	-	2.32	2.10	10.5	2.09	100.3%	-	-	-	23	-	Р	
78.2	JRA	18/01/2021	SPR	Fill B - REB Shear Key	~1m Placed (Ex	-	-	2.21	2.02	9.7	2.09	96.5%	-	-	-	20	-	Р	
78.3	JKA	18/01/2021	SPR	FIII B - REB Shear Key	Ramp Extension Area)	-	-	-	-	-	-	-	-	-	-	20	-	Р	
78.4						-	-	-	-	-	-	-	-	-	-	21	-	Р	
79.1						-	-	2.24	2.05	9.0	2.09	98.2%	-	-	-	25	-	Р	
79.2					~1.5m Placed (Ex	-	-	-	-	-	-	-	-	-	-	28	-	Р	
79.3					Ramp Extension Area)	-	-	2.24	2.06	8.6	2.09	98.5%	-	-	-	30	-	Р	
79.4	1			Fill B - REB Shear Key		-	-	-	-	-	-	-	-	-	-	25	-	Р	
79.5	JRA	18/01/2021	SPR		~0.7m below Hardfill	-	-	2.21	2.03	8.8	2.09	97.0%	-	-	-	32	-	Р	
79.6	1				FL	-	-	-	-	-	-	-	-	-	-	32	-	Р	
79.7	1					-	-	2.22	2.05	8.6	2.09	97.9%	-	-	-	36	-	Р	
79.8	1				~0.5m below Hardfill	-	-	-	-	-	-	-	-	-	-	22	-	Р	
79.9	1				FL	-	-	2.23	2.03	9.8	2.09	97.2%	-	-	-	32	-	Р	



 Job : Hitchens Block Stage 5
 Job #
 1009213.0657.0.0/3

 Entered By
 DASA/JRA/FRHA

 Customer: Lander Geotechnical Ltd
 Checked By
 SEBA/DASA/JRA

 Approved By
 SJA

											Nuclear De	nsity (Backsca	ter)					PASS / FAIL	
URN	Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m <sup>3</sup> )	% Solid Density	% Total Voids	Impact Value 1	Retest URN	(P) Pass (F) Fail	Comments
80.1						-	-	2.31	2.09	10.5	2.09	100.1%	-	-	-	21	-	Р	
80.2	JRA	19/01/2021	SPR	Fill B - REB Shear Key	~2m Placed (Ex Ramp Extension	-	-	2.56	2.35	9.0	2.09	112.3%	-	-	-	25	-	Р	
80.3	5101	13/01/2021	5.11	The bolication	Area)	-	-	-	-	-	-	-	-	-	-	26	-	Р	
80.4						-	-	-	-	-	-	-	-	-	-	26	-	Р	
81.1						-	-	2.24	2.06	8.6	2.09	98.6%	-	-	-	24	-	Р	
81.2					0 Far halan haadfill	-	-	2.22	2.05	8.2	2.09	98.2%	-	-	-	25	-	Р	
81.3	DASA	21/01/2021	SPR	Fill B - REB Shear Key	0.5m below hardfill FL	-	-	-	-	-	-	-	-	-	-	27	-	Р	
81.4						-	-	-	-	-	-	-	-	-	-	21	-	Р	
81.5						-	-	-	-	-	-	-	-	-	-	28	-	Р	
82.1						-	-	2.22	2.04	8.6	2.09	97.6%	-	-	-	28	-	Р	
82.2						-	-	2.23	2.04	9.0	2.09	97.7%	-	-	-	30	-	Р	
82.3						-	-	2.24	2.06	8.9	2.09	98.6%	-	-	-	33	-	Р	
82.4						-	-	2.25	2.06	9.0	2.09	98.8%	-	-	-	34	-	Р	
82.5						-	-	2.24	2.05	9.2	2.09	97.9%	-	-	-	27	-	Р	
82.6	DASA	22/01/2021	SPR	Fill B - REB Shear Key	FL	-	-	-	-	-	-	-	-	-	-	31	-	Р	
82.7						-	-	-	-	-	-	-	-	-	-	32	-	Р	
82.8						-	-	-	-	-	-	-	-	-	-	28	-	Р	
82.9						-	-	-	-	-	-	-	-	-	-	24	-	Р	
82.10						-	-	-	-	-	-	-	-	-	-	30	-	Р	
82.11						-	-	-	-	-	-	-	-	-	-	32	-	Р	
82.12						-	-	-	-	-	-	-	-	-	-	34	-	Р	
87.1						-	-	-	-	-	-	-	-	-	-	20	-	Р	
87.2						-	-	-	-	-	-	-	-	-	-	21	-	Р	
87.3						-	-	-	-	-	-	-	-	-	-	24	-	Р	
87.4						-	-	-	-	-	-	-	-	-	-	30	-	Р	
87.5						-	-	-	-	-	-	-	-	-	-	28	-	Р	
87.6						-	-	-	-	-	-	-	-	-	-	26	-	Р	
87.7						-	-	-	-	-	-	-	-	-	-	24	-	Р	
87.8	DASA	4/02/2021	GAP65	Fill B - REB Shear Key	500mm placed	-	-	-	-	-	-	-	-	-	-	28	-	Р	
87.9		, , , , ,		,		-	-	-	-	-	-	-	-	-	-	30	-	Р	
87.10						-	-	-	-	-	-	-	-		-	24	-	Р	
87.11						-	-	-	-	-	-	-	-	-	-	26	-	Р	
87.12						-	-	-	-	-	-	-	-	-	-	25	-	Р	
87.13						-	-	-	-	-	-	-	-	-	-	22	-	Р	
87.14						-	-	-	-	-	-	-	-	-	-	20	-	Р	
87.15						-	-	-	-	-	-	-	-	-	-	21	-	Р	
87.16						-	-	-	-	-	-	-	-	-		24	-	Р	

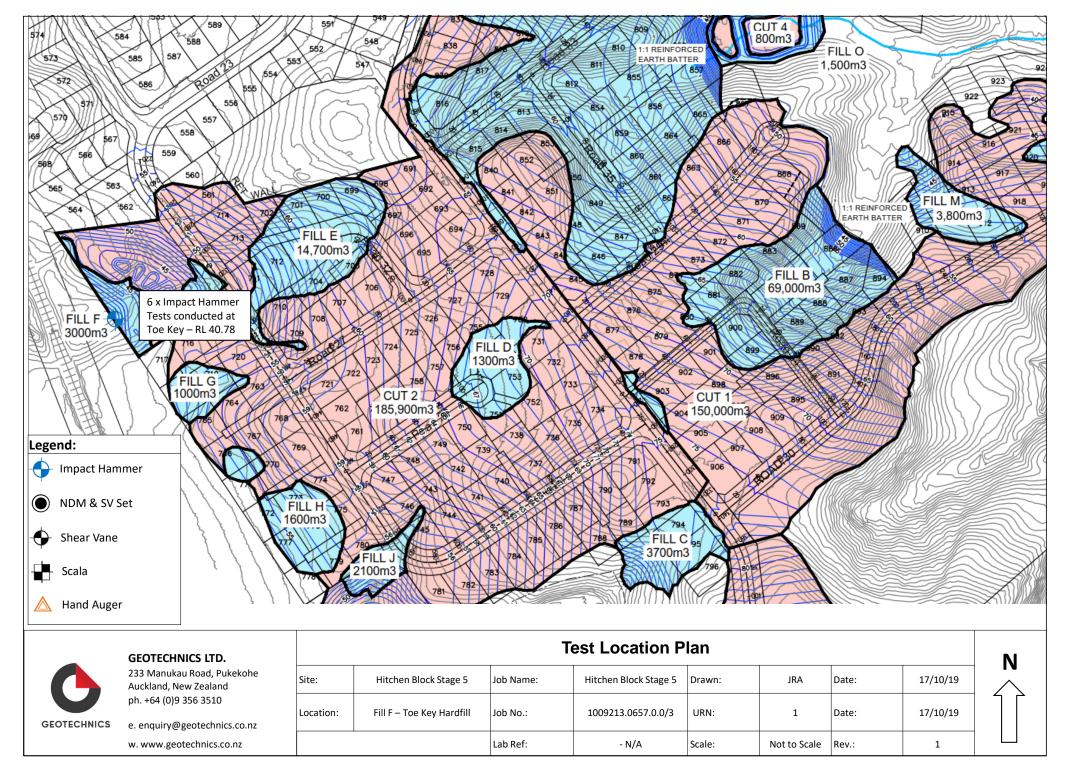


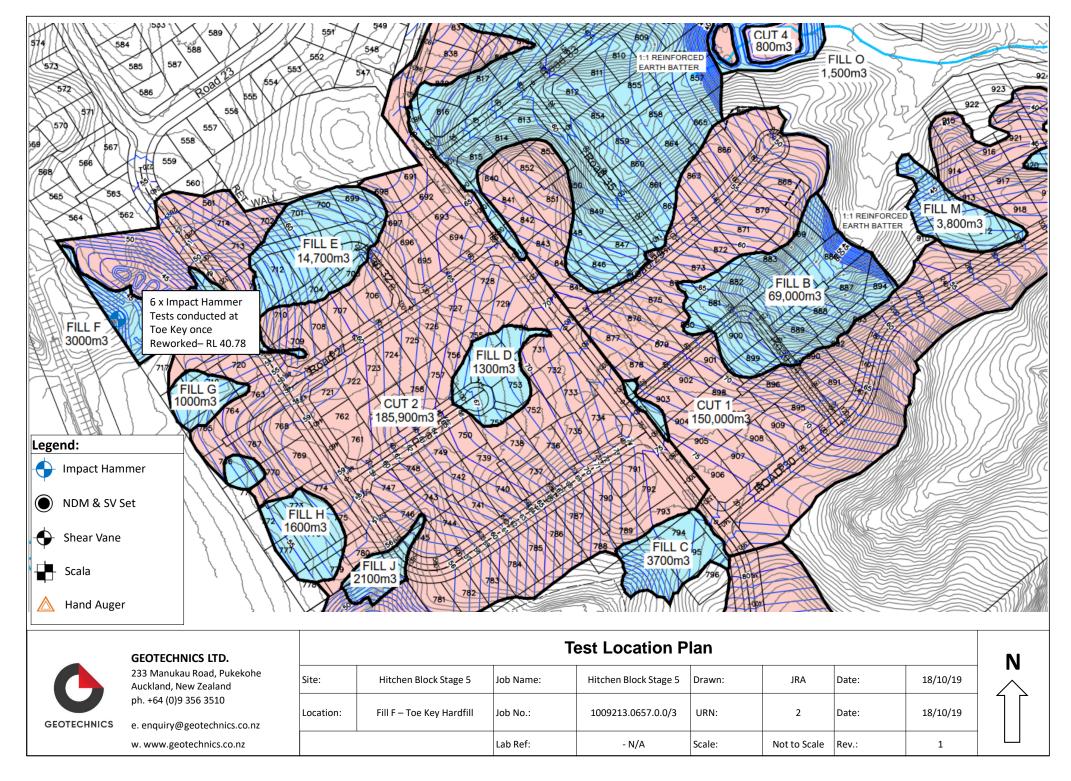
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HNICS		Entered By	DASA/JRA/FRHA
писз	Customer: Lander Geotechnical Ltd	Checked By	SEBA/DASA/JRA
		Approved By	SJA

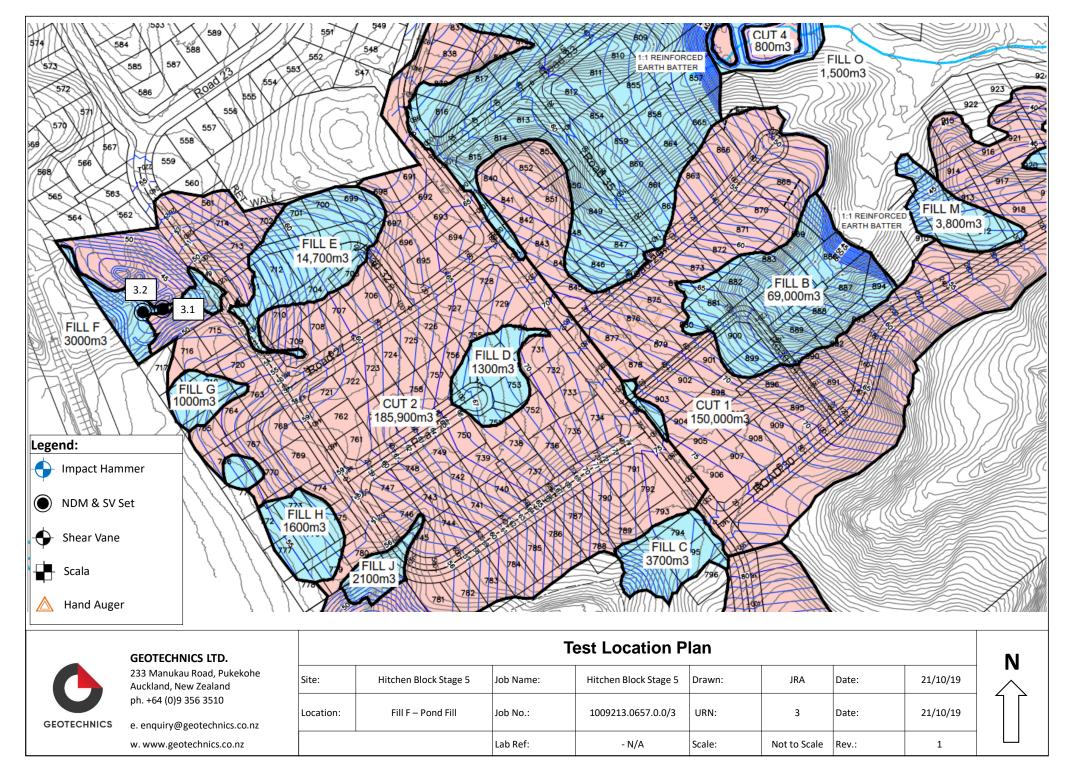
											Nuclear De	nsity (Backsca	tter)					PASS / FAIL	
U	N Tech.	Date	Material Type	Location	Layer	Chainage	Offset	Wet Density	Dry Density (t/m³)	Moisture Content (%)	Maximum Dry Density (t/m <sup>3</sup> )	% Maximum Dry Density	Solid Density (t/m <sup>3</sup> )	% Solid Density	% Total Voids	Impact Value 1	Retest URI	l (P) Pass (F) Fail	Comments
88	8.1					-	-	-	-	-	-	-	-	-	-	22	-	Р	
88	3.2					-	-	-	-	-	-	-	-	-	-	28	-	Р	
88	1.3					-	-	-	-	-	-	-	-	-	-	24	-	Р	
88	3.4					-	-	-	-	-	-	-	-	-	-	27	-	Р	
88	DASA	5/02/2021	GAP65	Fill B - REB Shear Key	500mm placed	-	-	-	-	-	-	-	-	-	-	20	-	Р	
88	.6			,		-	-	-	-	-	-	-	-	-	-	21	-	Р	
88						-	-	-	-	-	-	-	-	-	-	26	-	Р	
88						-	-	-	-	-	-	-	-	-	-	27	-	Р	
88	8.9					-	-	-	-	-	-	-	-	-	-	30	-	Р	
88	.10					-	-	-	-	-	-	-	-	-	-	32	-	Р	
89	0.1					-	-	-	-	-	-	-	-	-	-	22	-	Р	
89						-	-	-	-	-	-	-	-	-	-	24	-	Р	
89	0.3					-	-	-	-	-	-	-	-	-	-	28	-	Р	
89	0.4		GAP65			-	-	-	-	-	-	-	-	-	-	28	-	Р	
89	DASA	9/02/2021		Fill B - REB Shear Key	FL	-	-	-	-	-	-	-	-	-	-	30	-	Р	
89	0.6					-	-	-	-	-	-	-	-	-	-	24	-	Р	
89	.7					-	-	-	-	-	-	-	-	-	-	18	90.1	F	
89	.8					-	-	-	-	-	-	-	-	-	-	16	90.2	F	
89	0.9					-	-	-	-	-	-	-	-	-	-	14	90.3	F	
89	.10					-	-	-	-	-	-	-	-	-	-	14	90.4	F	
90	0.1					-	-	-	-	-	-	-	-	-	-	28	-	Р	Retest of 89.7
90	0.2					-	-	-	-	-	-	-	-	-	-	30	-	Р	Retest of 89.8
90	0.3					-	-	-	-	-	-	-	-	-	-	32	-	Р	Retest of 89.9
90	0.4					-	-	-	-	-	-	-	-	-	-	26	-	Р	Retest of 89.10
90	0.5 DASA	10/02/2021	GAP65	Fill B - REB Shear Key	FL	-	-	-	-	-	-	-	-	-	-	28	-	Р	
90	0.6	10, 02, 2021	0, 05	The billed her		-	-	-	-	-	-	-	-	-	-	30	-	Р	
90	).7					-	-	-	-	-	-	-	-	-	-	32	-	Р	
90	0.8					-	-	-	-	-	-	-	-	-	-	34	-	Р	
90	.9					-	-	-	-	-	-	-	-	-	-	30	-	Р	
90	.10					-	-	-	-	-	-	-	-	-	-	28	-	Р	
10	8.1					-	-	2.21	2.00	10.6	2.09	95.7%	-	-	-	20	-	Р	
10	8.2	25/03/2021	SPR	Pond Fill	51	-	-	-	-	-	-	-	-	-	-	20	-	Р	
10	8.3	25/05/2021	SPR	PUILU FIII	FL	-	-	-	-	-	-	-	-	-	-	29	-	Р	
10	8.4					-	-	-	-	-	-	-	-	-	-	32	-	Р	

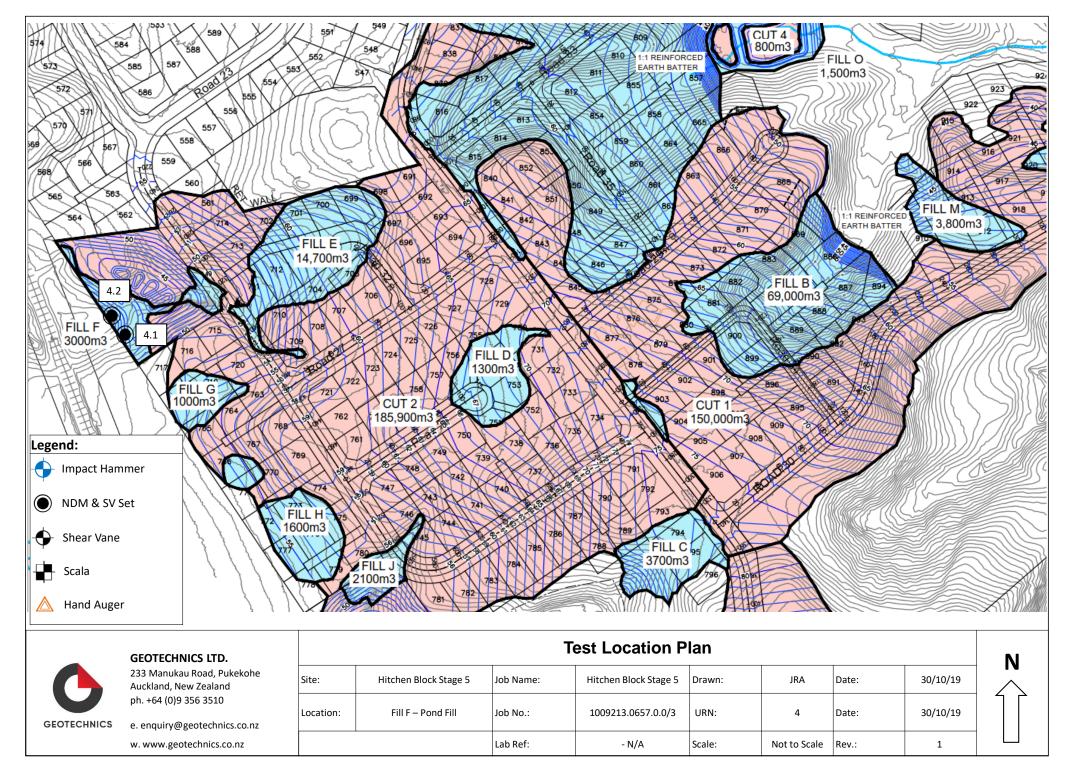
		GE	от	ECHI		S		Lander Ge	lock Stage otechnical ensity Test								Job # Entered By: Checked By: Approved By:	1009213.0657.0.0/3 FRHA SEBA SJA
URN	Tech.	Date	Location	Location (A / B)	Layer	Material Type	Number (Cumu VIBE	of Passes Ilative) STATIC	Nuclear Wet Density (t/m <sup>3</sup> )	Nuclear Dry Density (t/m <sup>3</sup> )	Nuclear water content (%)	Field Maximum Dry Density (MDD) (t/m <sup>3</sup> )	Percentage maximum Dry Density MDD (%)	Percentage of Solid Density (%)	Impact Value 1 (IV)	Pass / Fail	Retest URN	Comments
33.3							1	STATIC	1.77	1.65	7.4	2.09	78.7%	-	20	-	-	
33.5							2		1.89	1.79	5.7	2.09	85.5%	-	13	-	-	
33.7							3		1.78	1.67	6.2	2.09	80.0%	-	14	-	-	
33.9								5	2.09	1.95	7.1	2.09	93.5%	-	23	-	-	
33.1	FRHA	19/02/2020	Shear Key	Location A	~300mm	SPR		7	2.13	1.99	6.7	2.09	95.4%	-	28	-	-	
33.13					placed			9	2.15	2.04	5.5	2.09	97.5%	-	31	-	-	
33.15								11	2.17	2.04	6.4	2.09	97.6%	-	35	-	-	
33.17								13	2.19	2.08	5.2	2.09	99.4%	-	35	-	-	
33.19								15	2.21	2.09	5.7	2.09	100.0%	-	35	-	-	Maximum field dry density achieved of 2.09 t/m <sup>3</sup>
33.21								17	2.21	2.07	7.0	2.09	98.9%	-	34	-	-	
						Density vs. C	umulative N	umber Of Pa	sses							Plateau Te	st Details	
	2.20															Plant		17 Tonne Single Drum Roller
																Material Typ	pe	SPR
(e	2.10								2.08	2.09		2.07				Supplier		Smythe's Quarry
:y (t/m	2.00					.99		2.04			2					Layer Thickr	iess	~300mm
Densit	1.90			1.95	1								- <b>E</b> Vibratio	on 🖛 Static		Subgrade Ty	pe	Unknown
Nuclear Dry Density (t/m³)	1.80 -															Definition o	f # of Passes	1 Pass = 2 passes above the test area with plant (i.e. there & back)
Nuc		/	1.79	/												Comments		
	1.70	1 65	1.67	1.67	1			1		1	1							
	0		2	4	6	8	10	12		14	16	18						
						Cumulative Number	of Passes (#)											

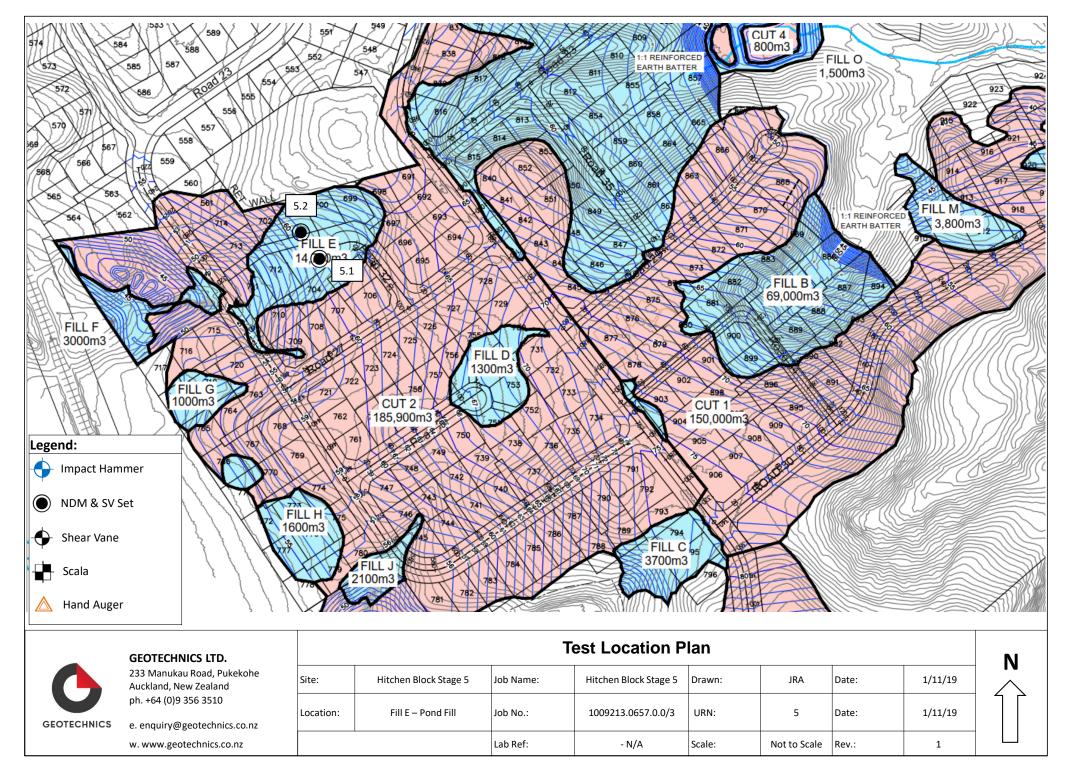
	3	GE	ΟΤΕ	CHN	ICS		Client:	Lander G	Block Stag eotechnical eensity Test							-	Job # Entered By: Checked By: Approved By:	1009213.0657.0.0/3 FRHA SEBA SJA
URN	Tech.	Date	Location	Location (A / B)	Layer	Material Type	Number (Cumu	ilative)	Nuclear Wet Density (t/m <sup>3</sup> )	Nuclear Dry Density (t/m <sup>3</sup> )	Nuclear water content (%)	Field Maximum Dry Density (MDD) (t/m <sup>3</sup> )	Percentage maximum Dry Density MDD (%)	Percentage of Solid Density (%)	Impact Value 1 (IV)	Pass / Fail	Retest URN	Comments
33.4							VIBE 1	STATIC	1.70	1.62	4.9	2.09	77.7%	-	11	-	-	
33.6							2		1.78	1.70	4.9	2.09	81.2%	-	14	-	-	
33.8							3		1.75	1.66	5.4	2.09	79.5%	-	16	-	-	
3.10								5	1.94	1.84	5.5	2.09	87.9%	-	24	-	-	
3.12	FRHA	19/02/2020	Shear Key	Location B	~300mm placed	SPR		7	2.01	1.92	4.6	2.09	91.9%	-	23	-	-	
3.14	FNDA	19/02/2020	Sileal Key	LOCATION B				9	2.03	1.91	6.2	2.09	91.5%	-	24	-	-	
3.16								11	2.06	1.96	4.9	2.09	93.8%	-	27	-	-	
3.18								13	2.10	2.01	4.6	2.09	96.1%	-	28	-	-	
3.20								15	2.07	1.98	4.7	2.09	94.8%	-	27	-	-	
33.22								17	2.07	1.98	4.7	2.09	94.6%	-	27	-	-	
						Density vs. Cu	imulative Nu	mber Of Pa	isses							Plateau Te	st Details	
	2.05															Plant		17 Tonne Single Drum Roller
	2.00								2.01							Material Typ	e	SPR
( <sub>e</sub> u	1.95					$\sim$	1	.96		1.98	1	.98				Supplier		Smythe's Quarry
ity (t/r	1.90				1.	92 1.91										Layer Thickn	ess	~300mm
Nuclear Dry Density (t/m³)	1.85			1.84										on 🖛 Static		Subgrade Ty	ре	Unknown
lear Dr	1.80	1.04														Definition of	# of Passes	1 Pass = 2 passes above the test area with plant (i. there & back)
Nuc	1.70		1.70	/												Comments		
	1.65	1.62	1.66	.66														
	1.60		2	4	6	8	10	12		14	16	18						
						Cumulative Number	of Passes (#)											

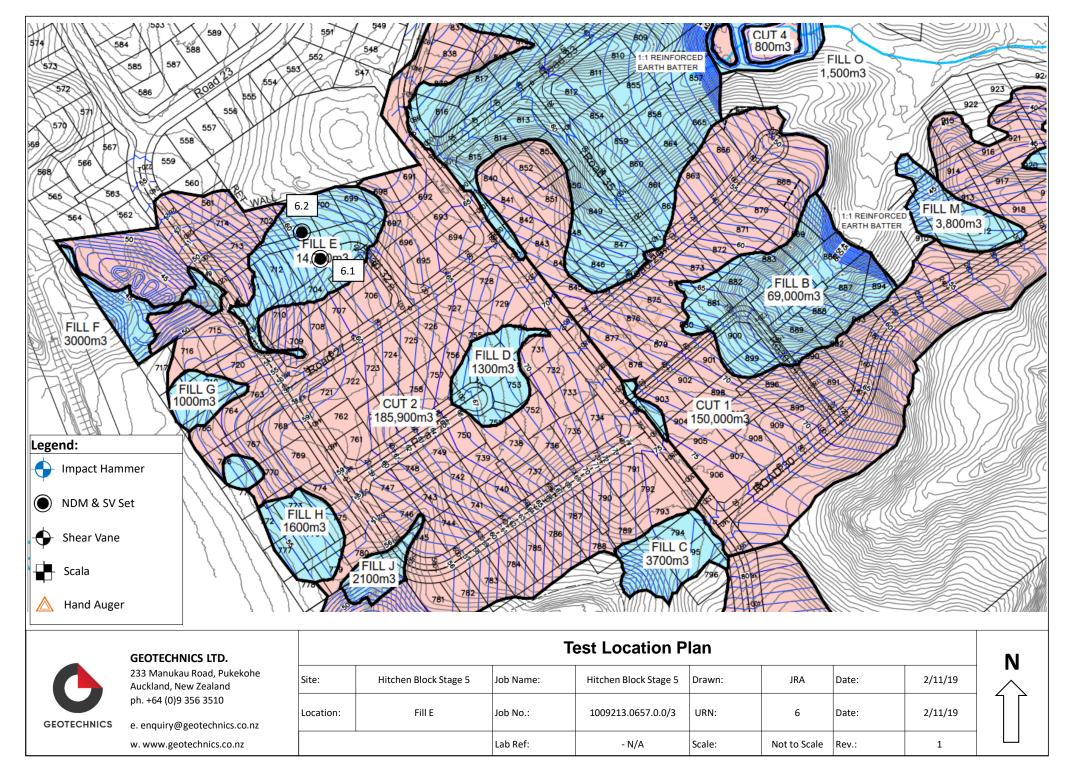


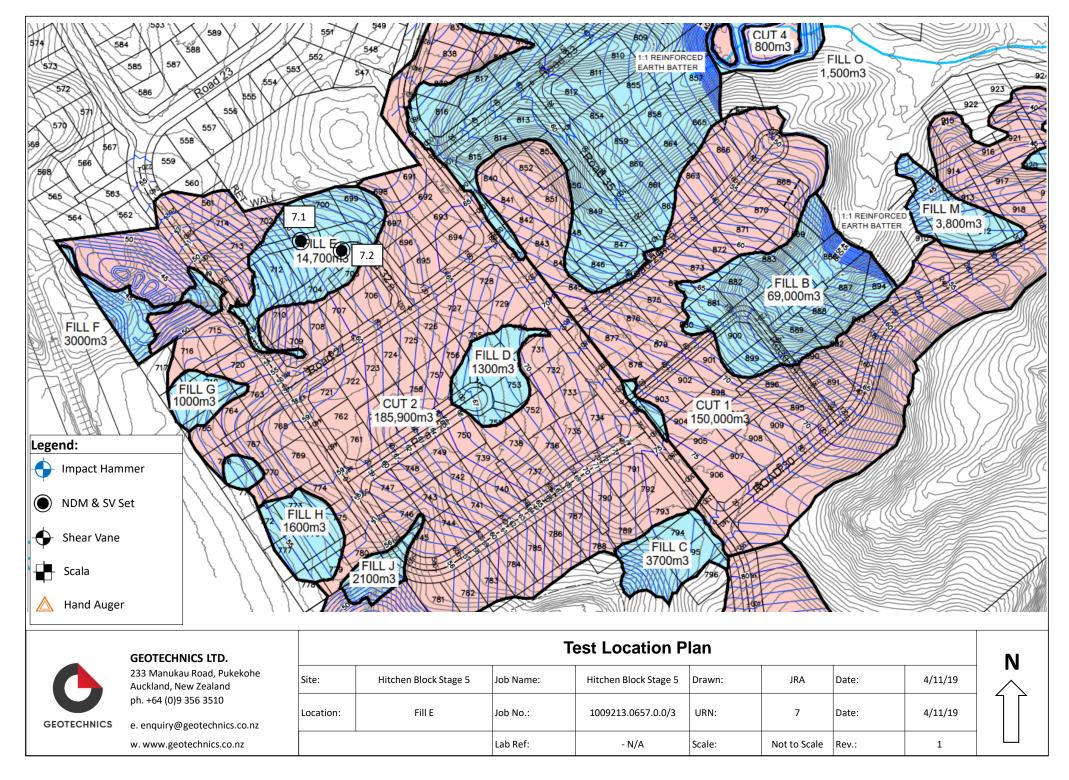


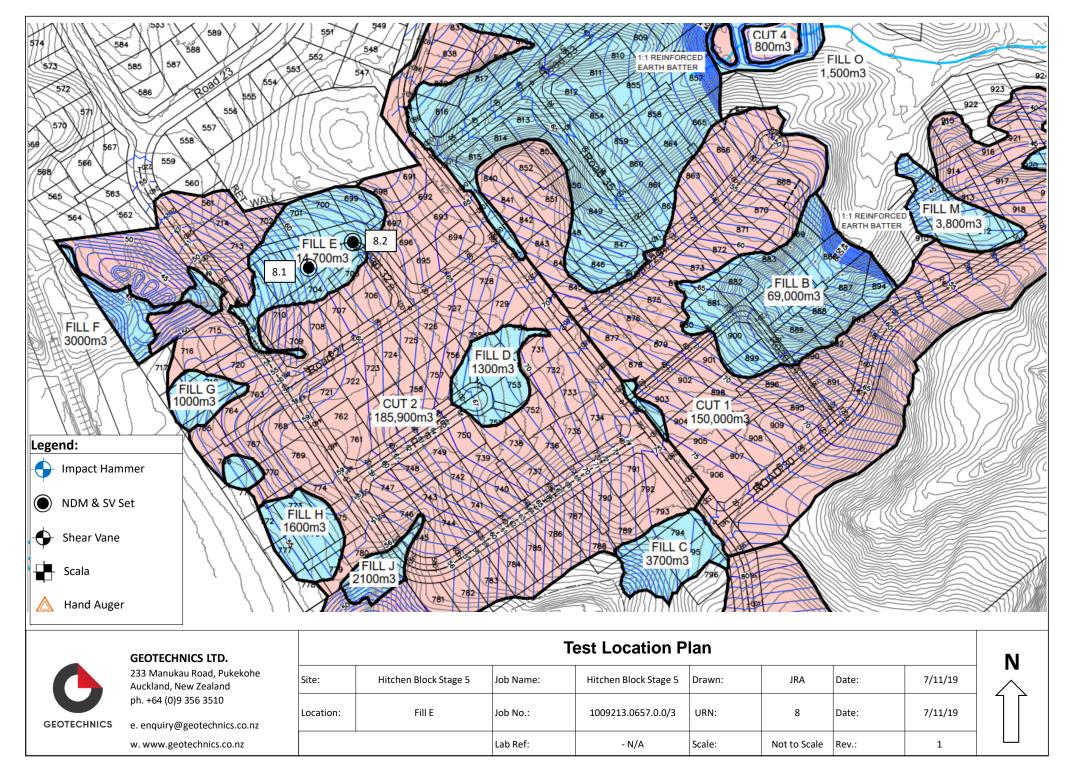


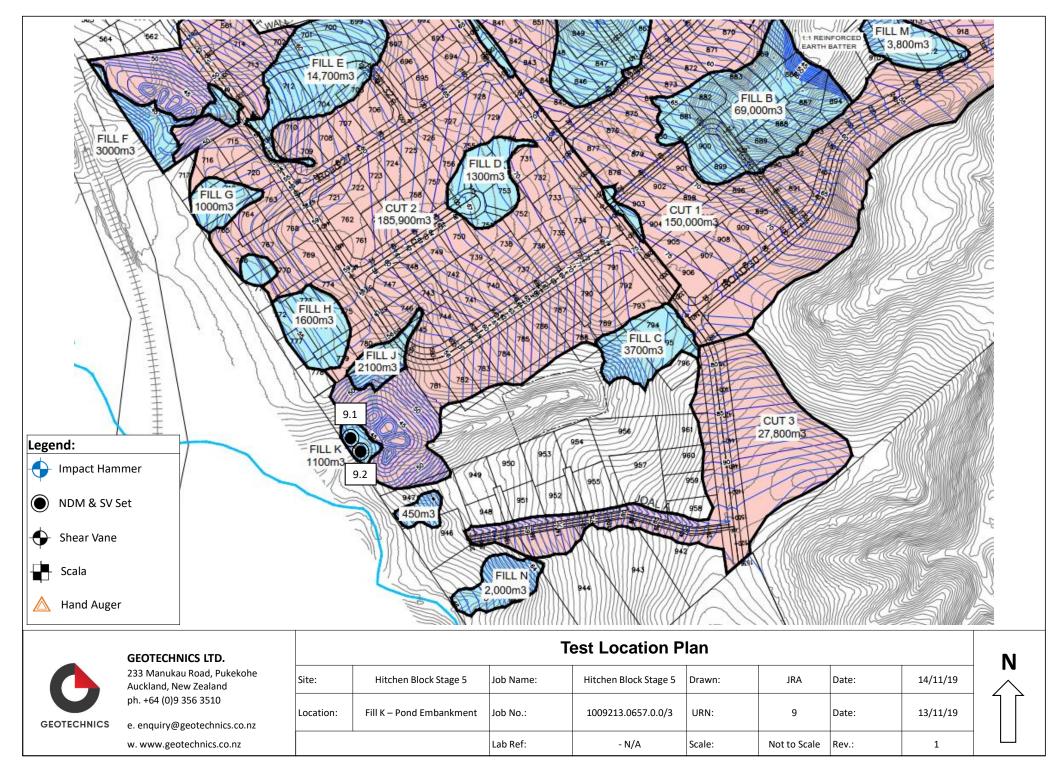


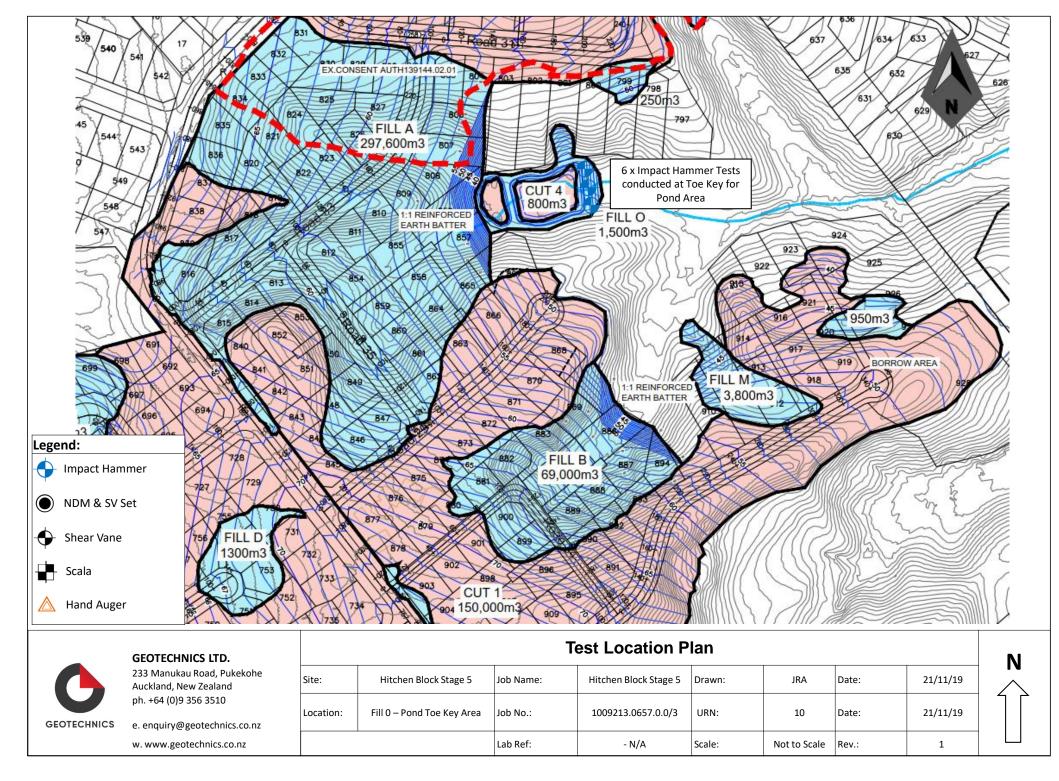


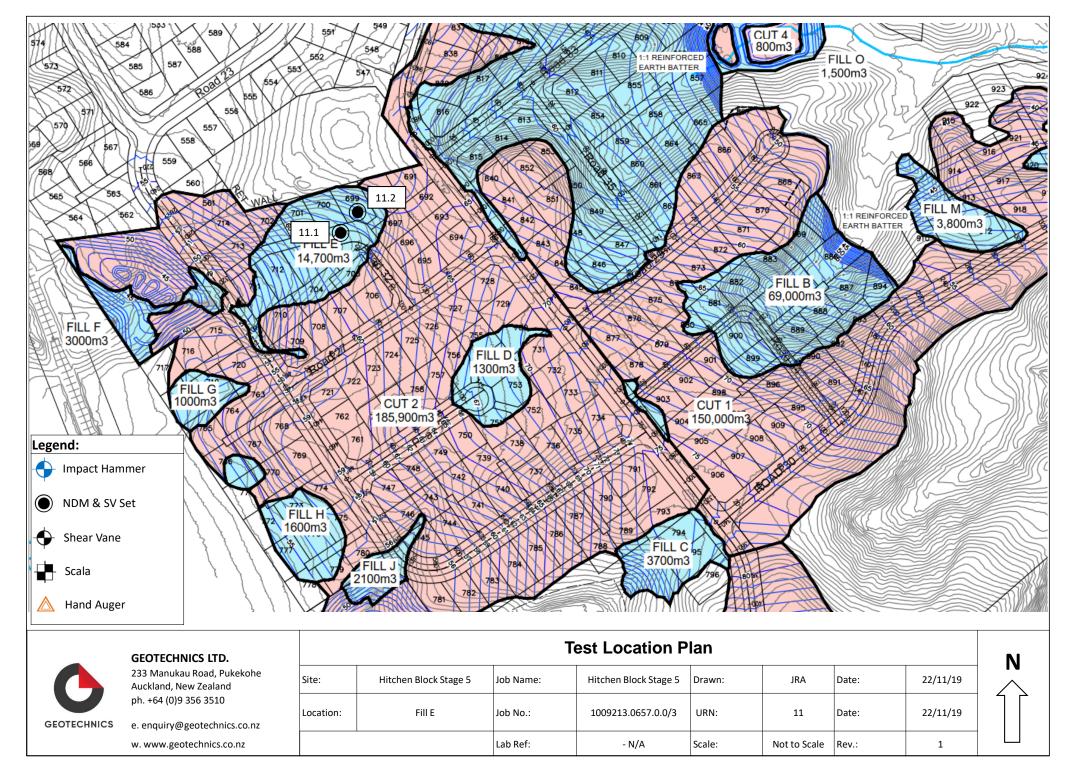


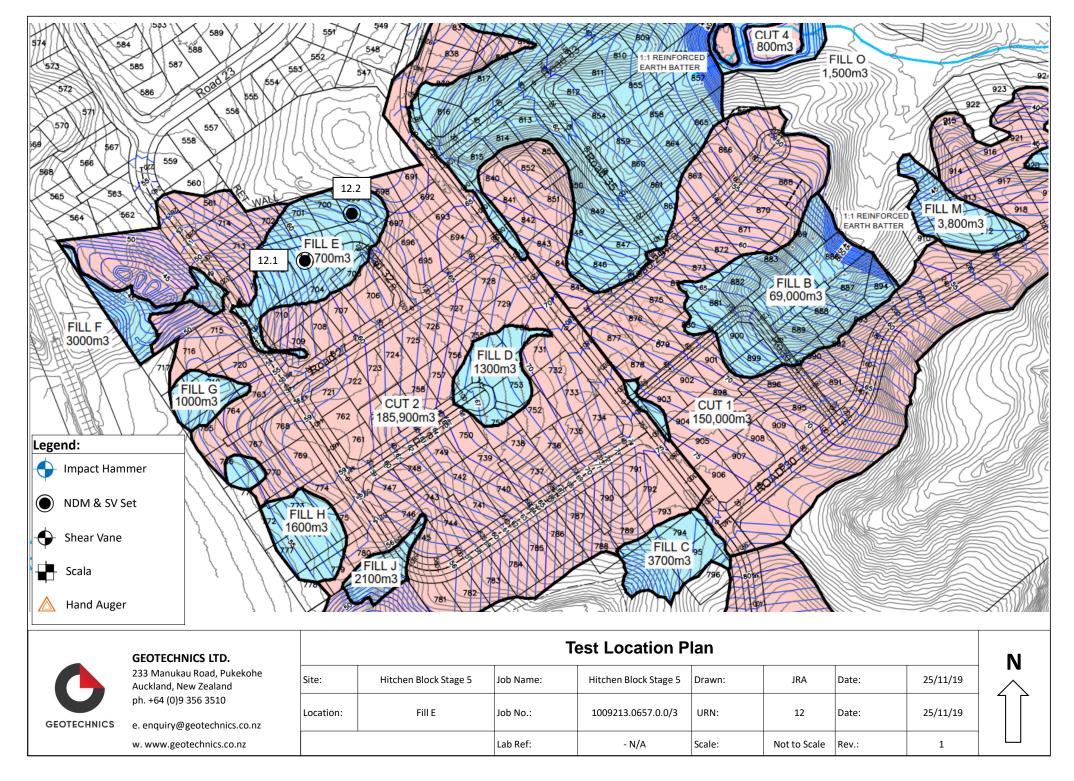


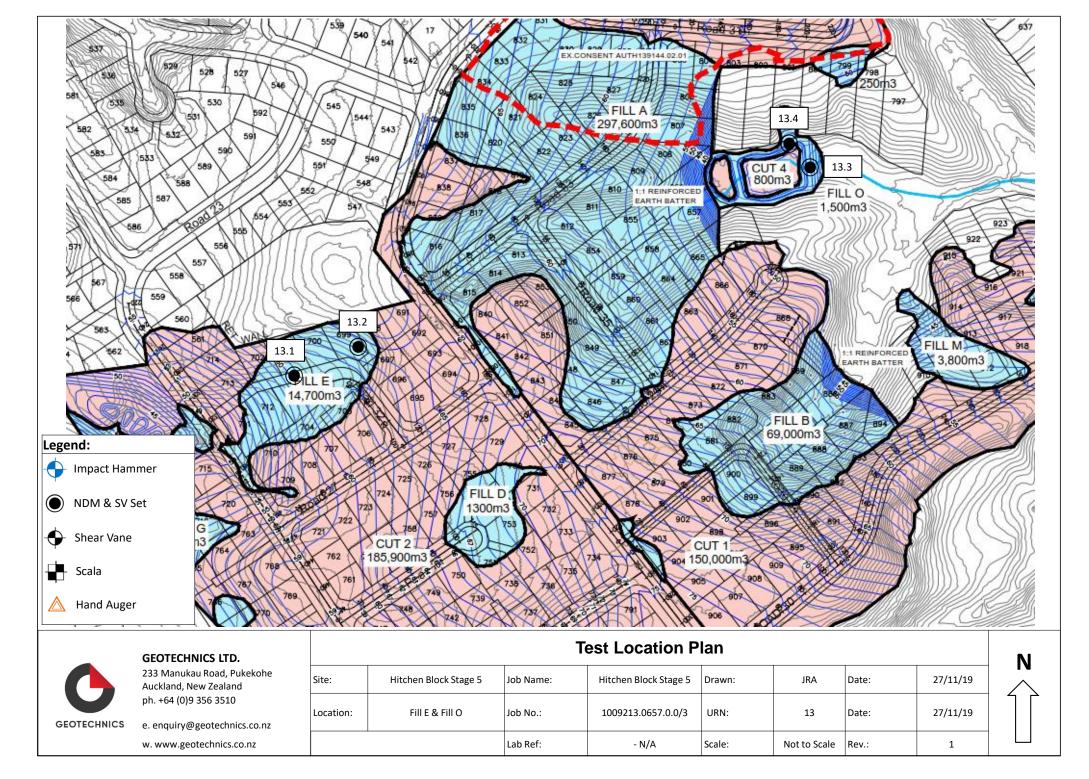


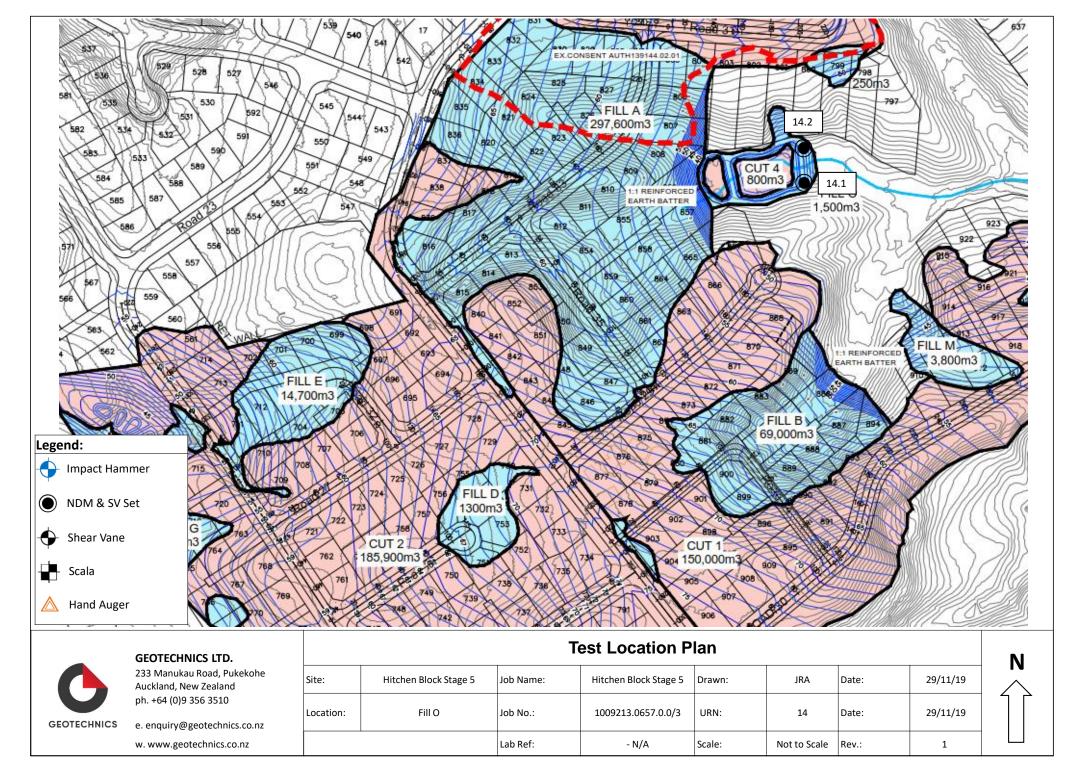


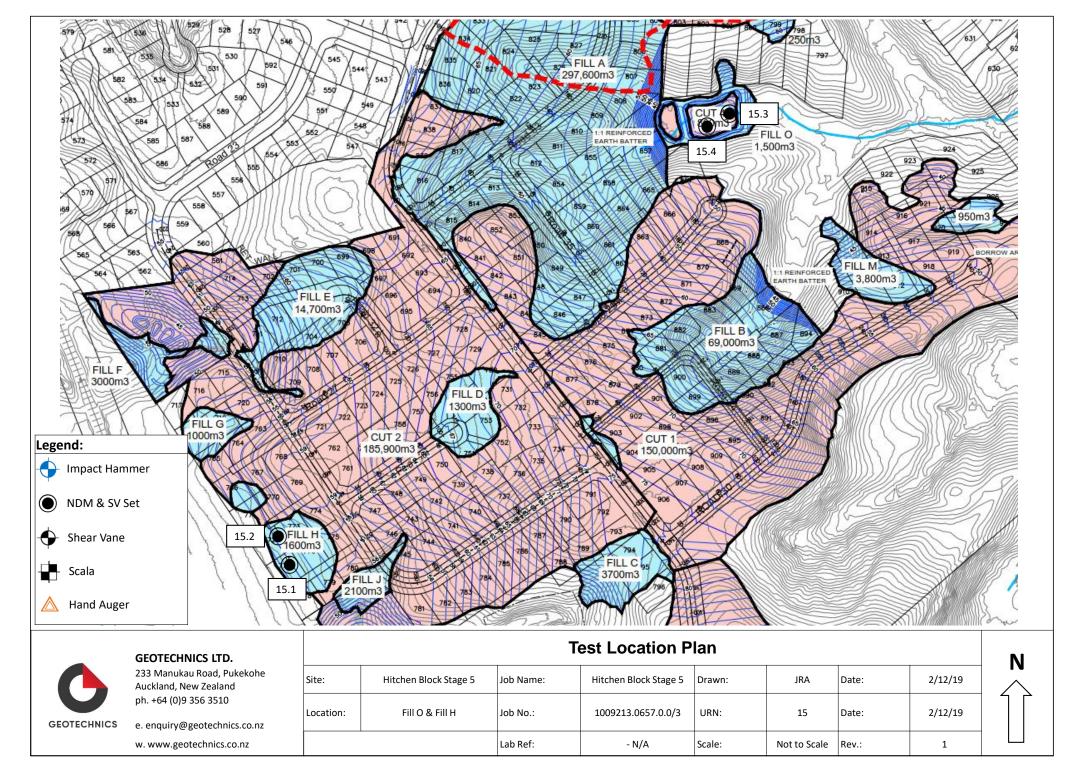


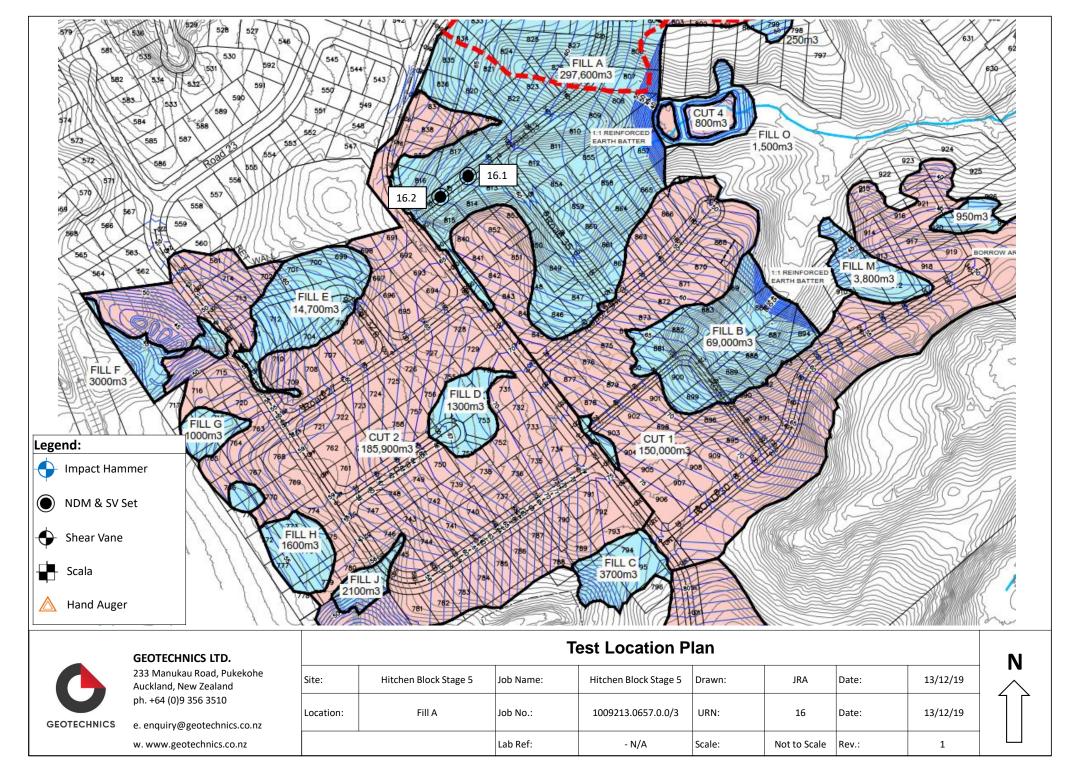


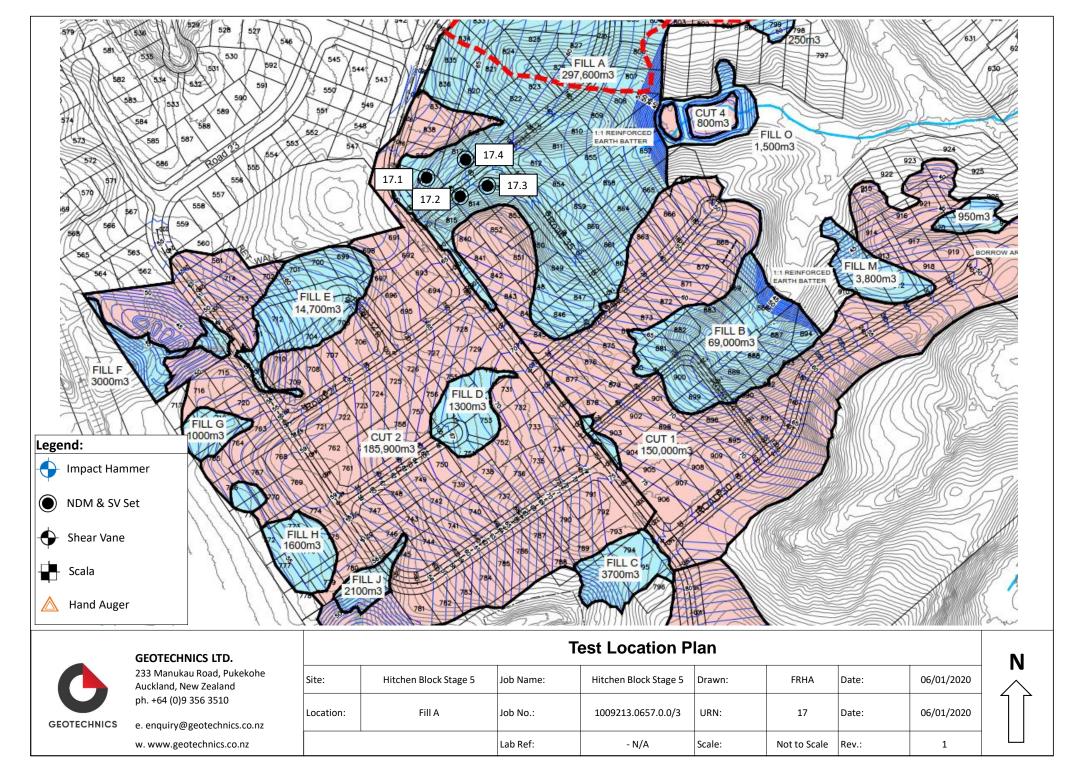


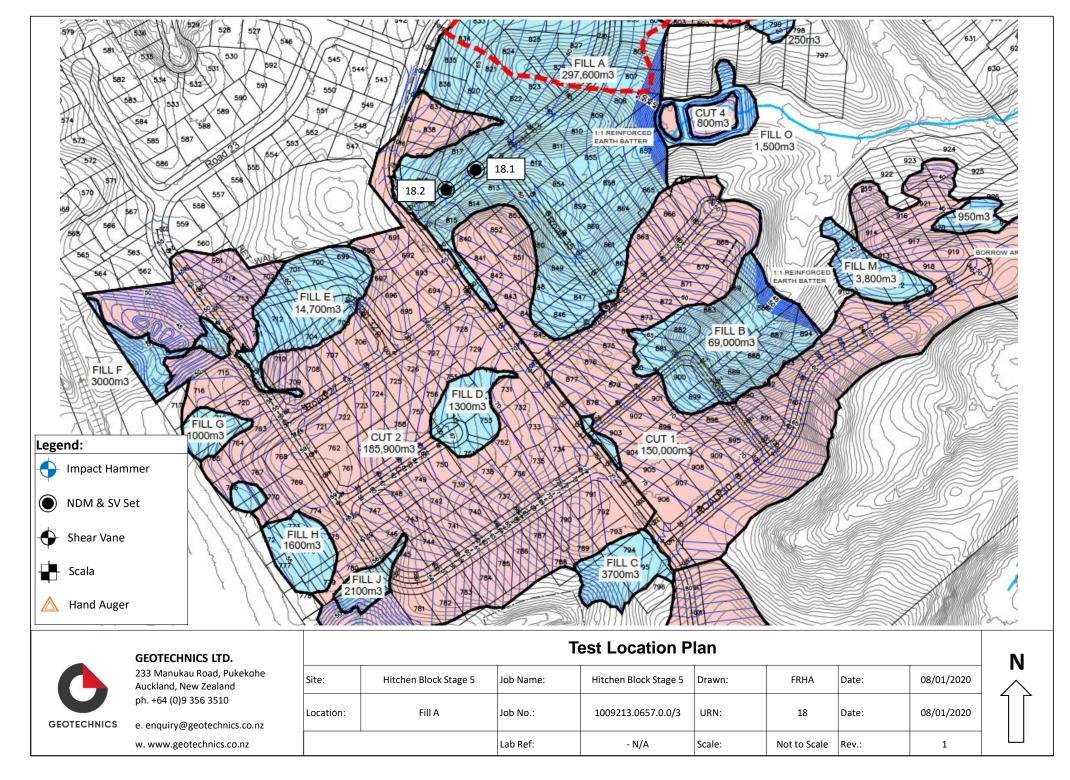


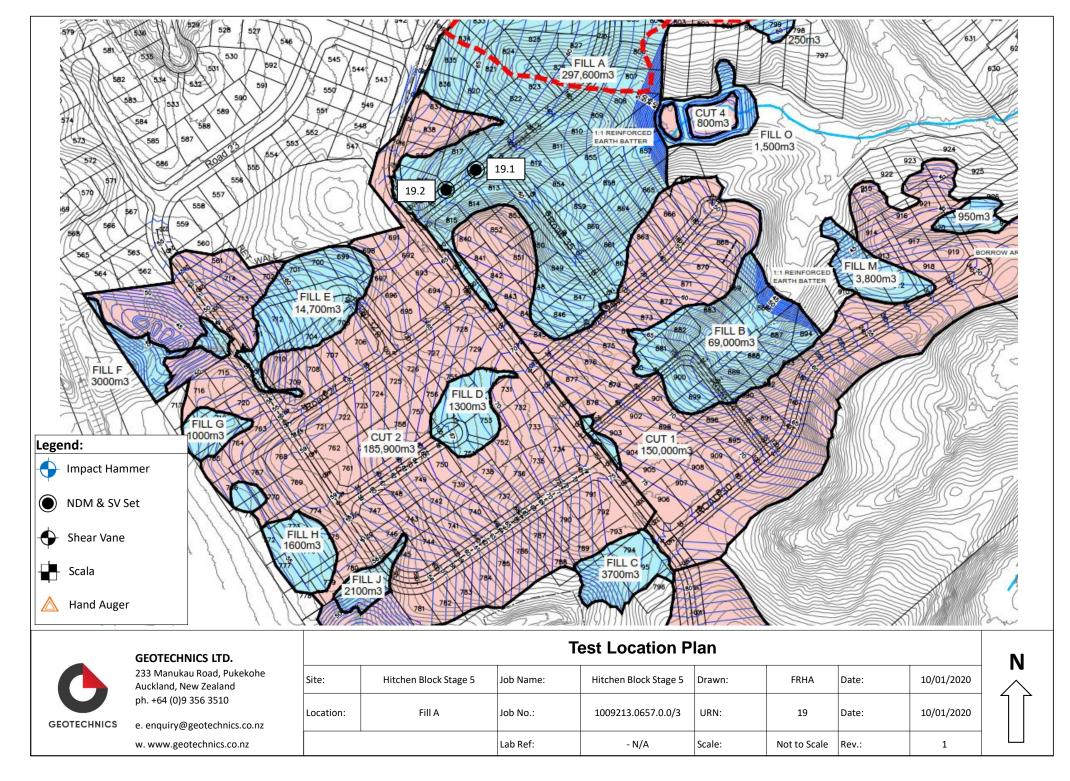


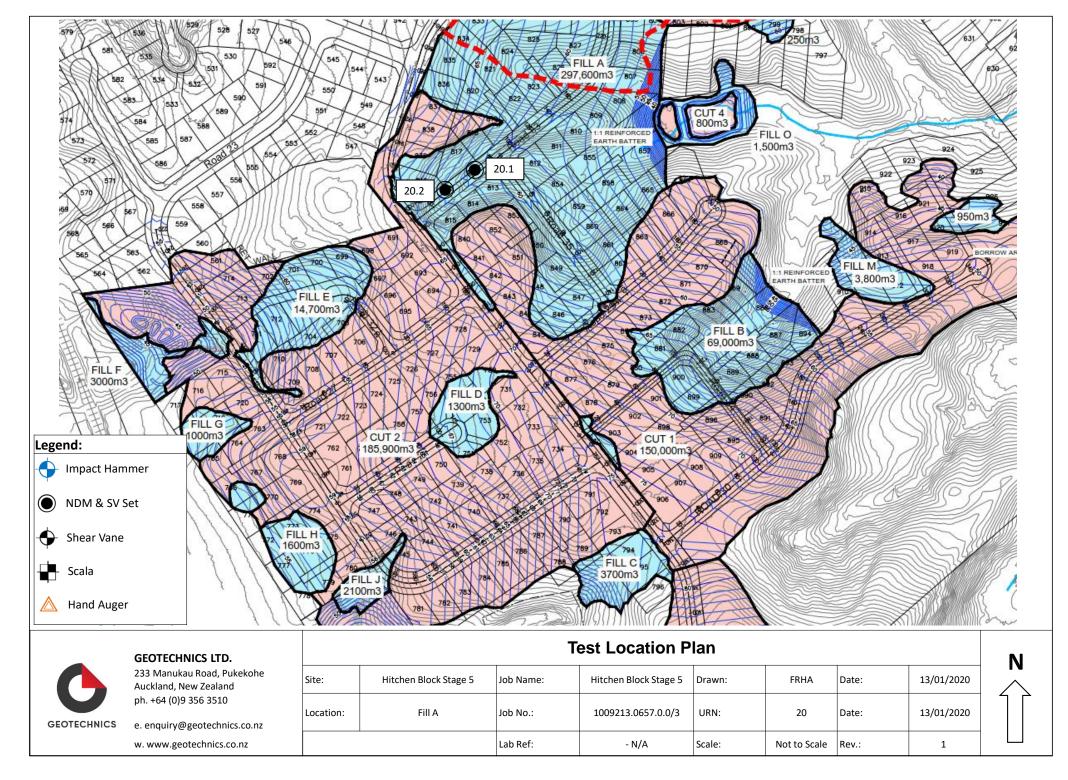


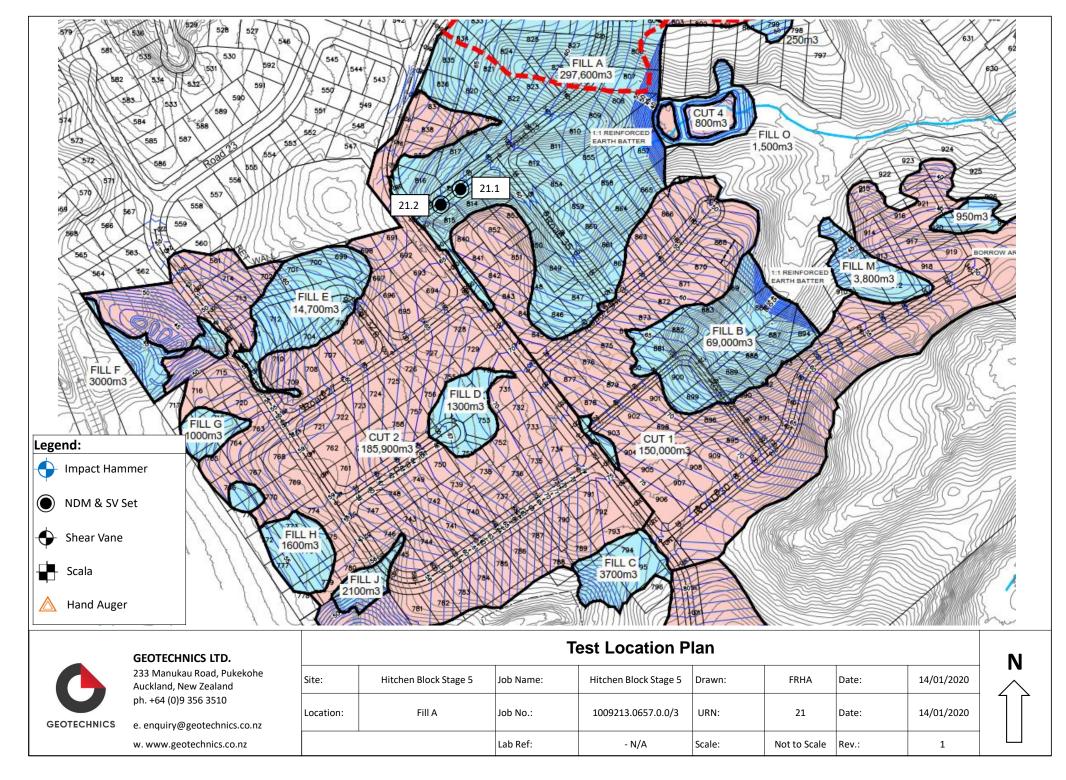


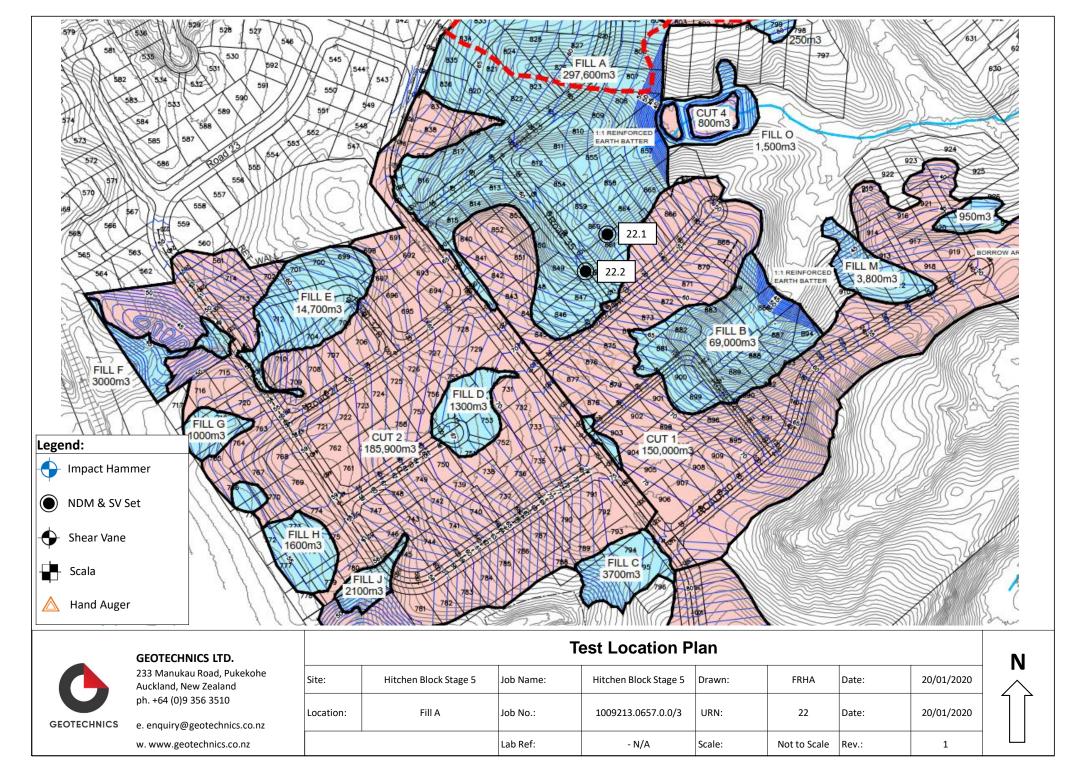


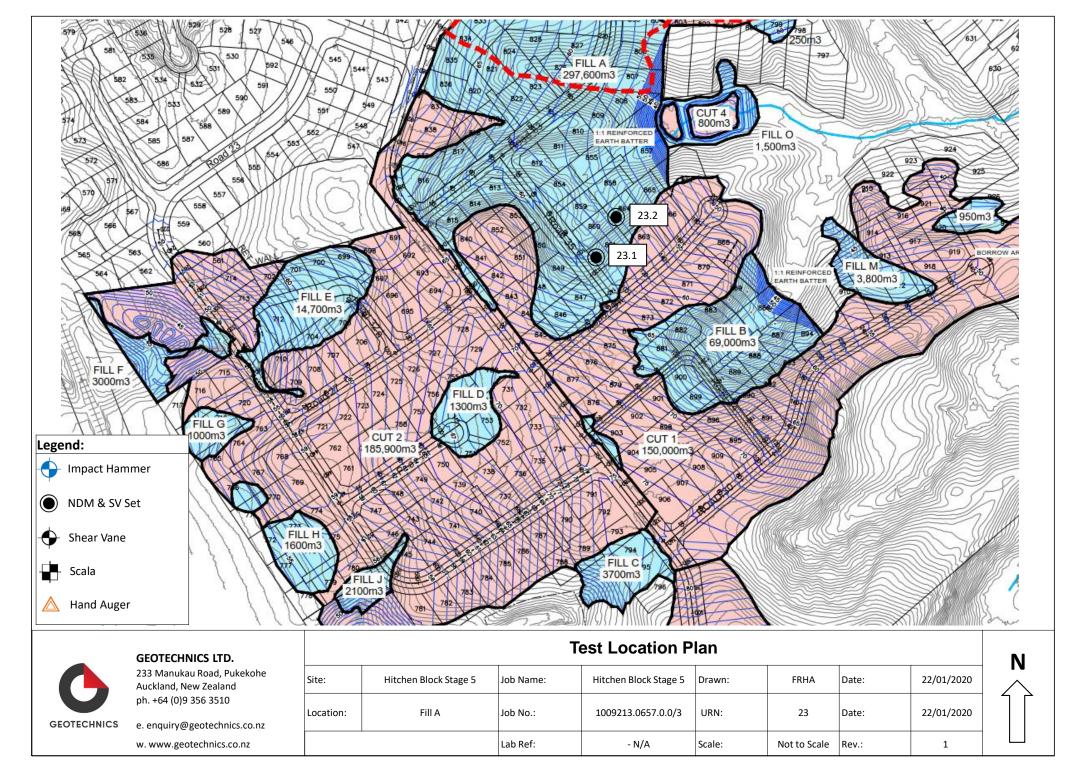


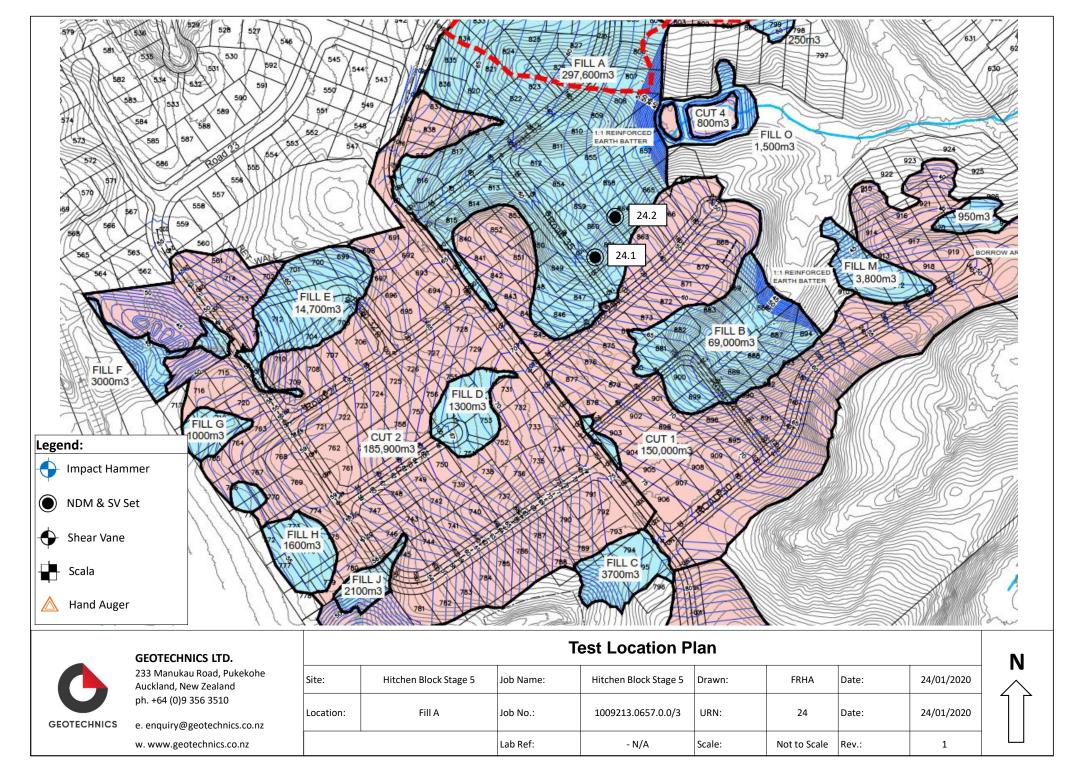


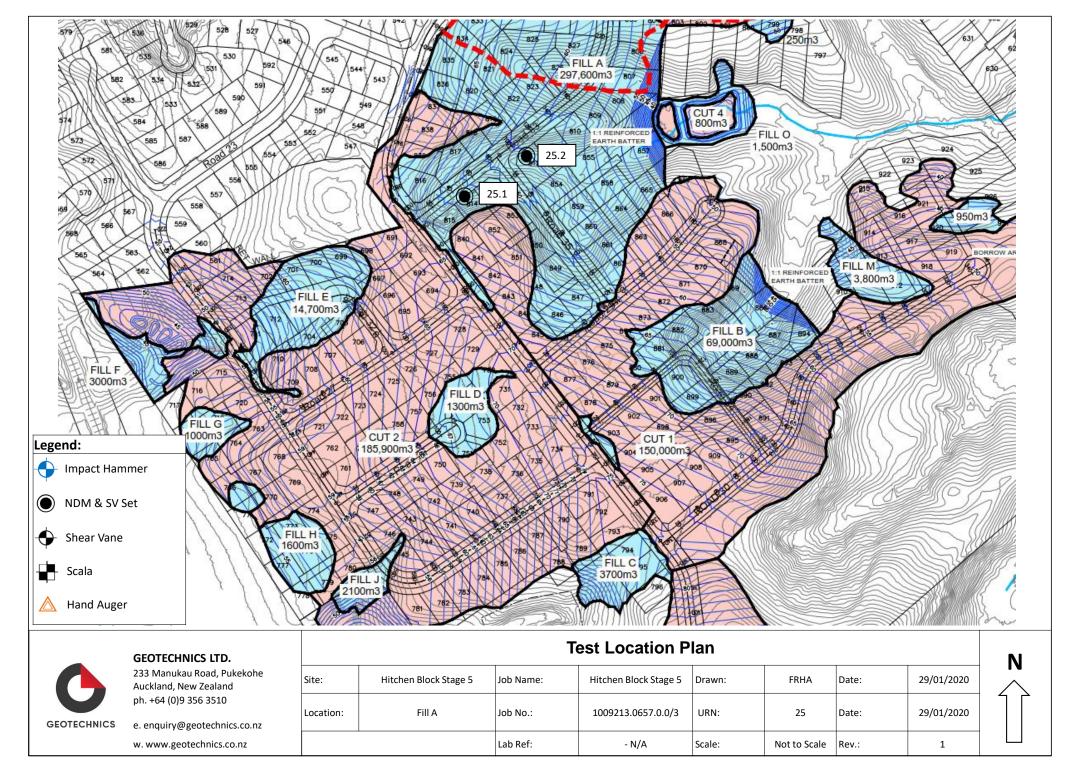


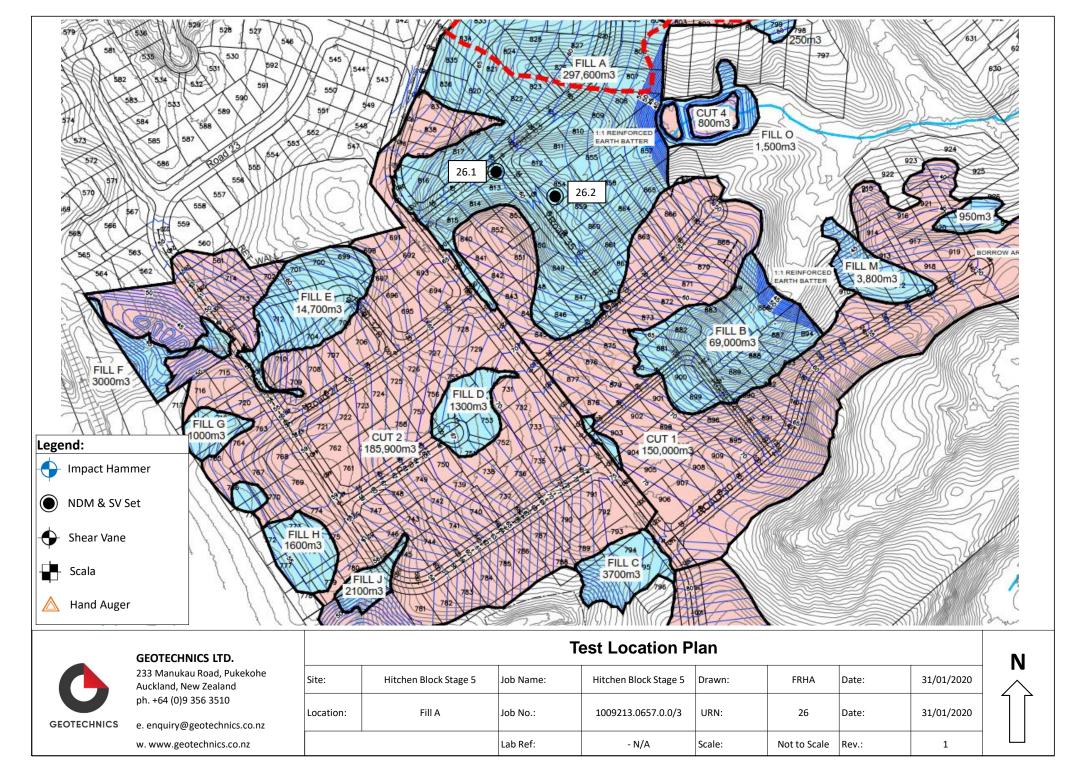


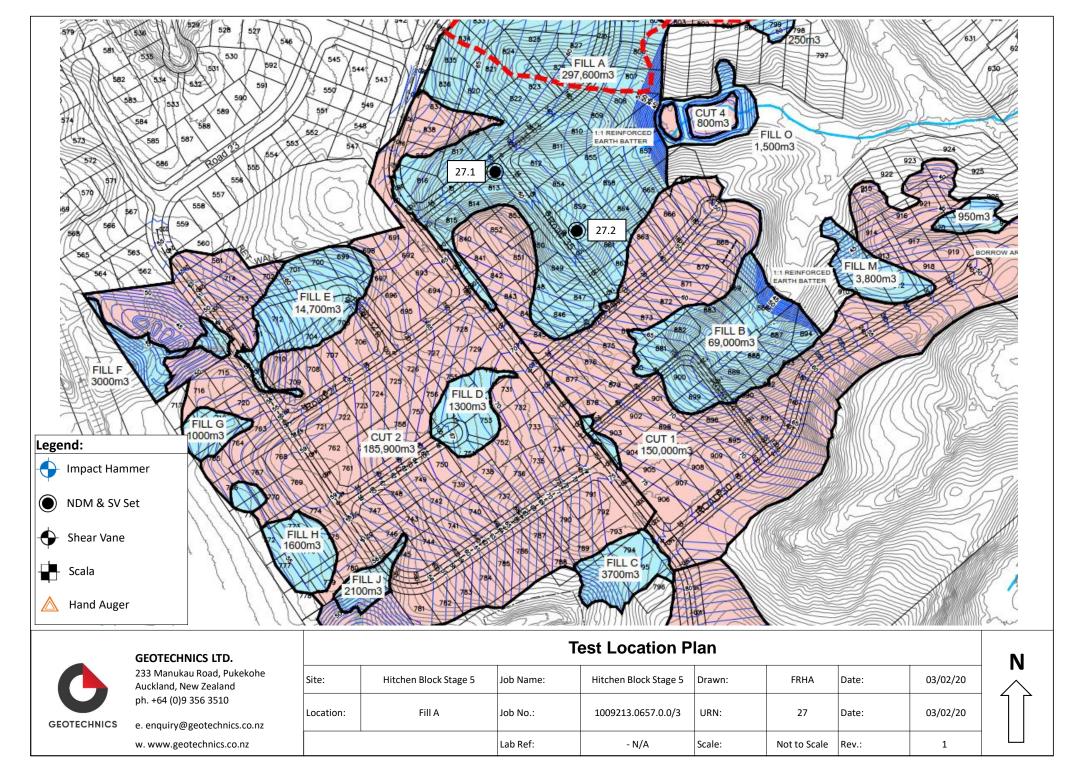


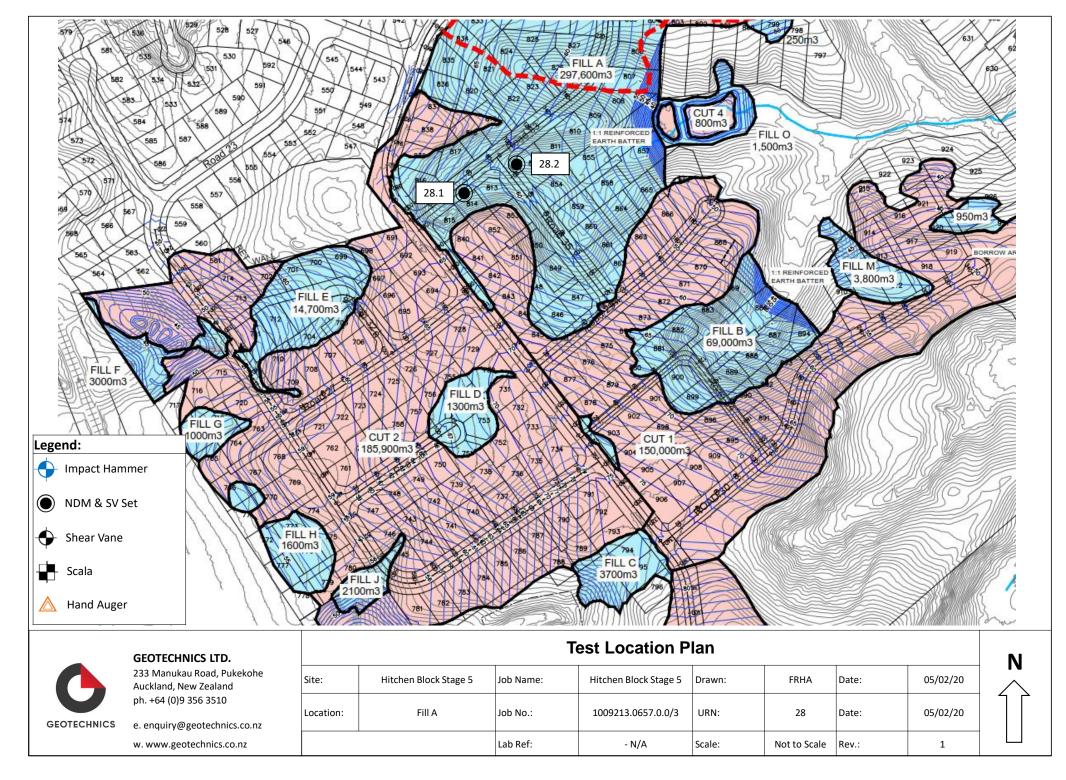


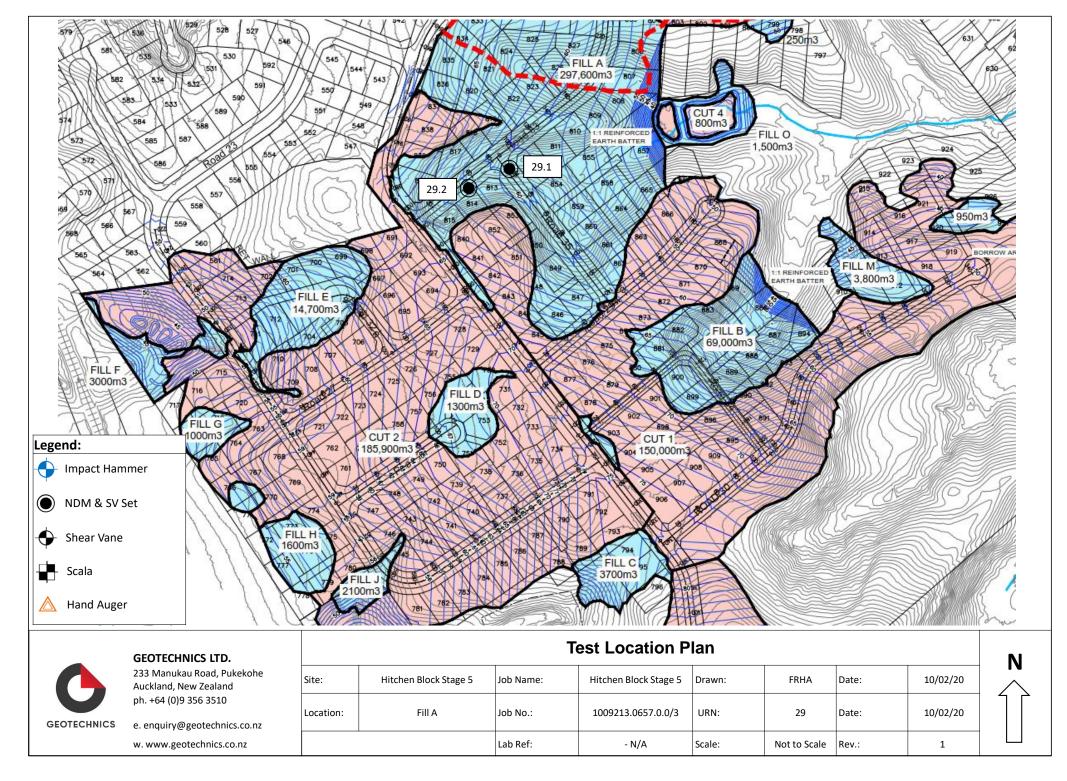


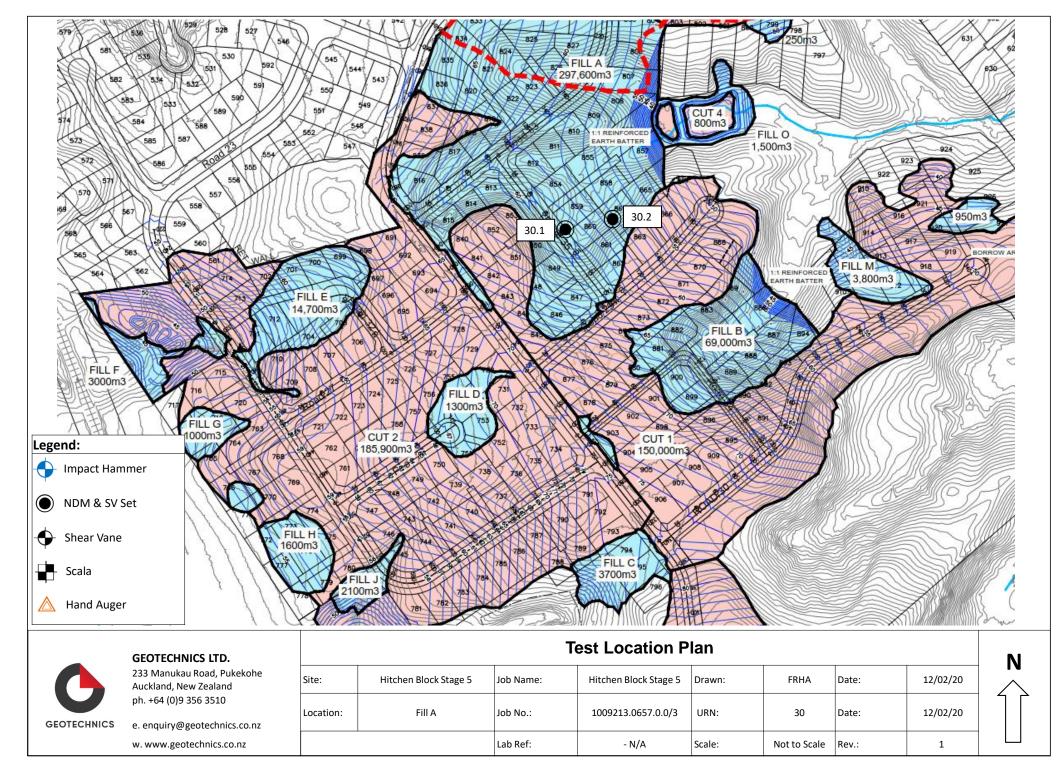


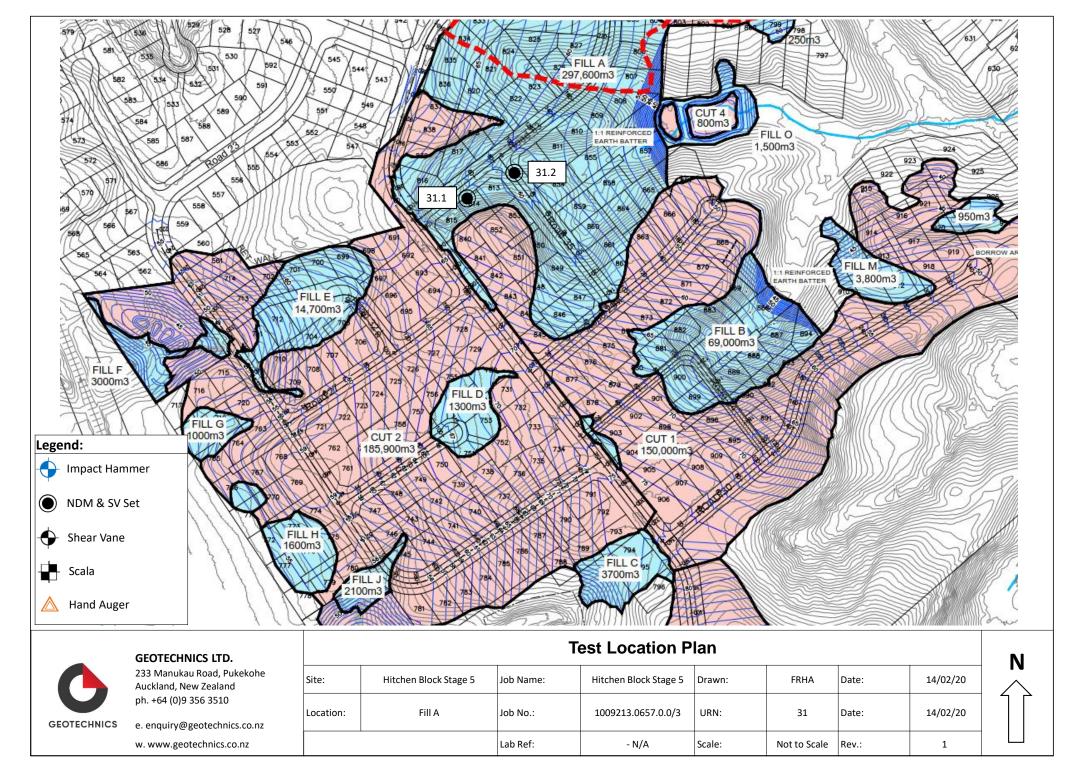


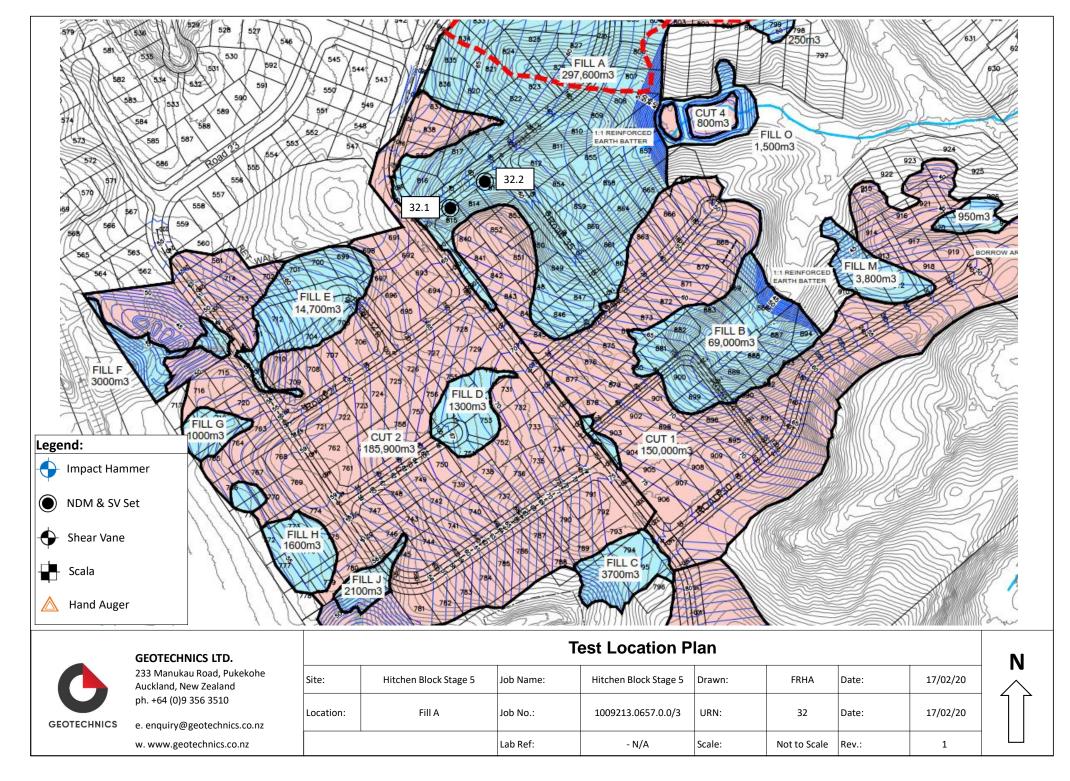


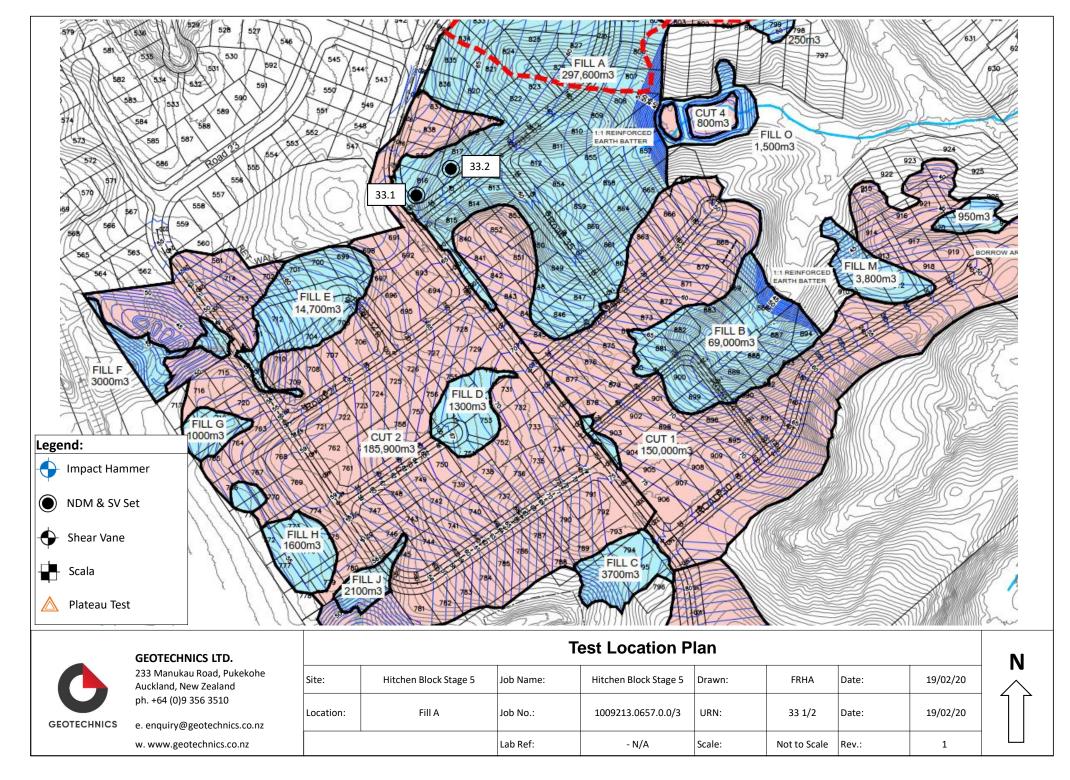


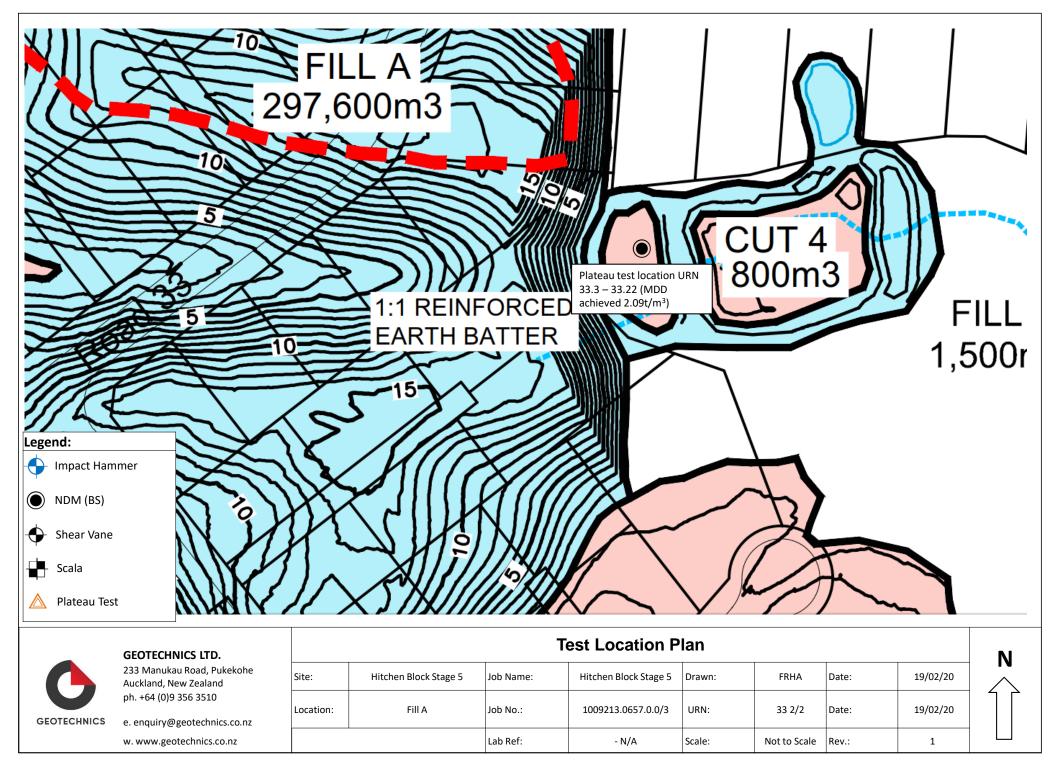


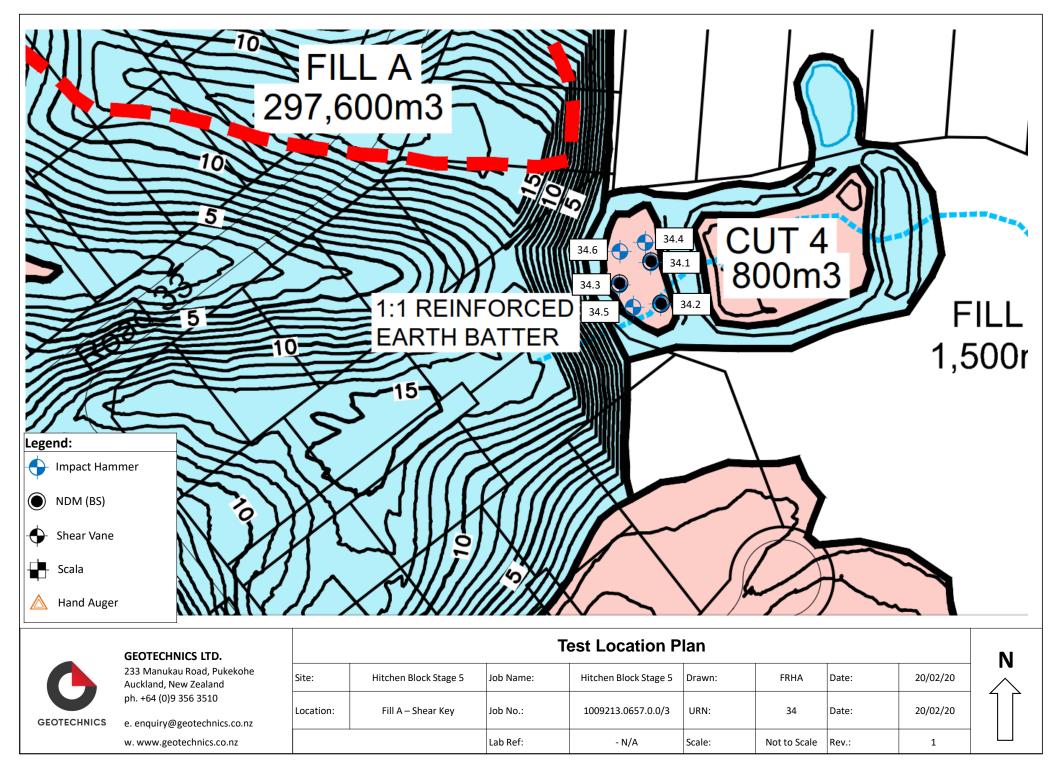


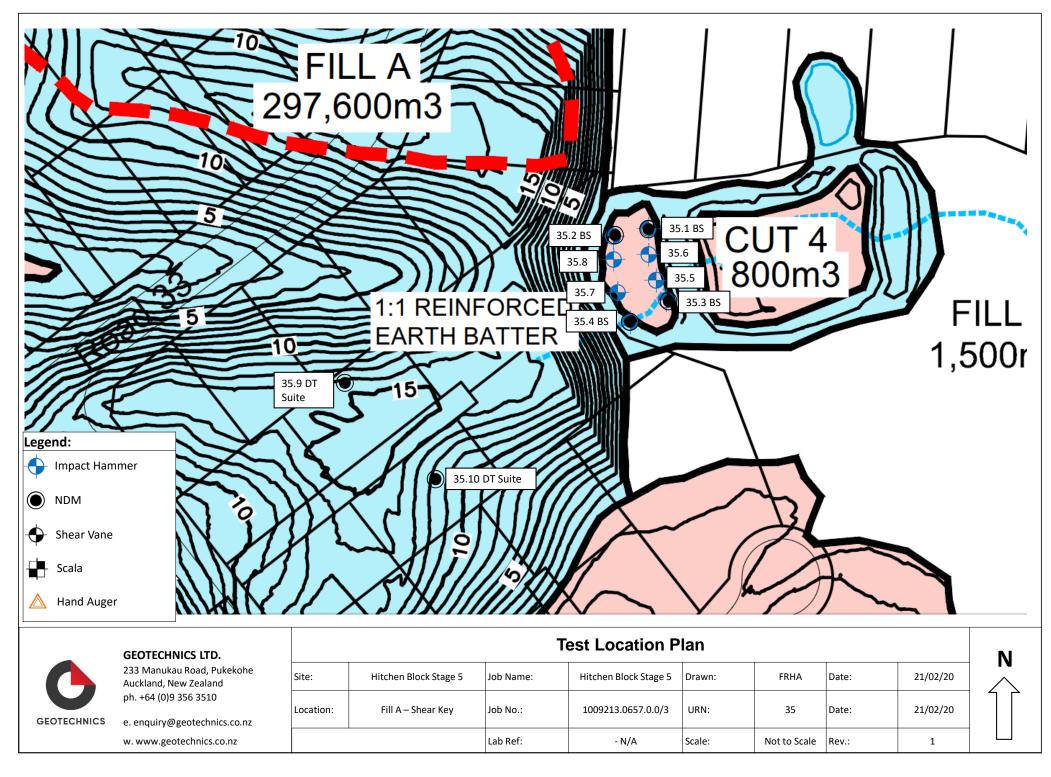


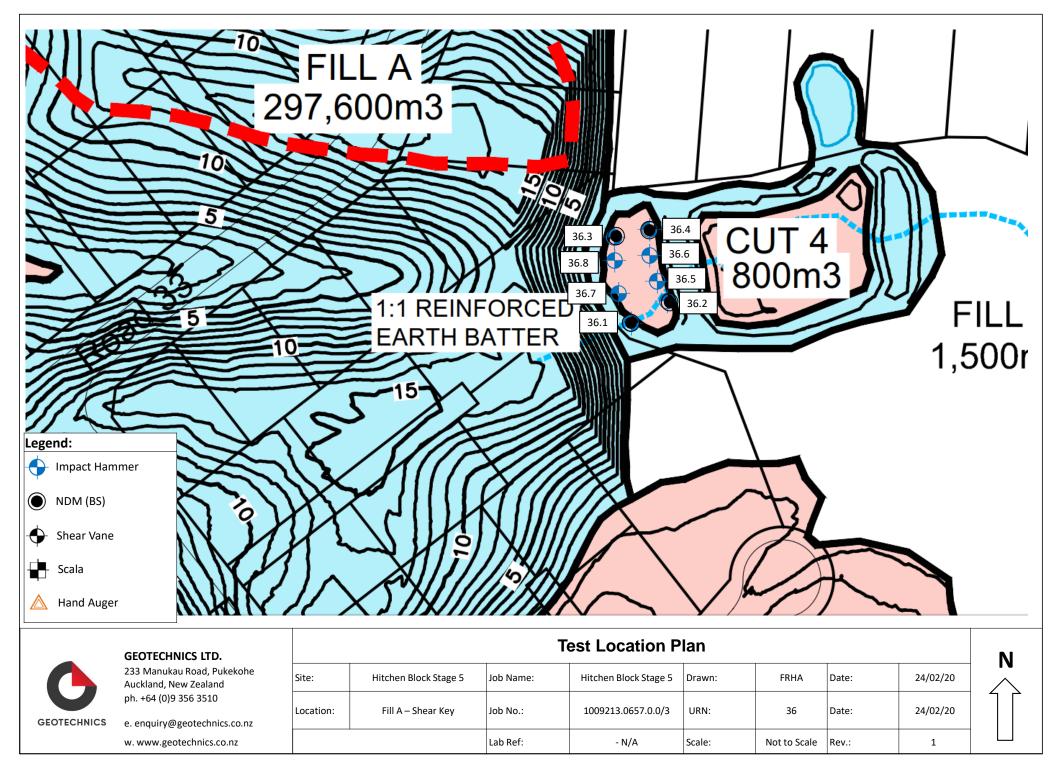


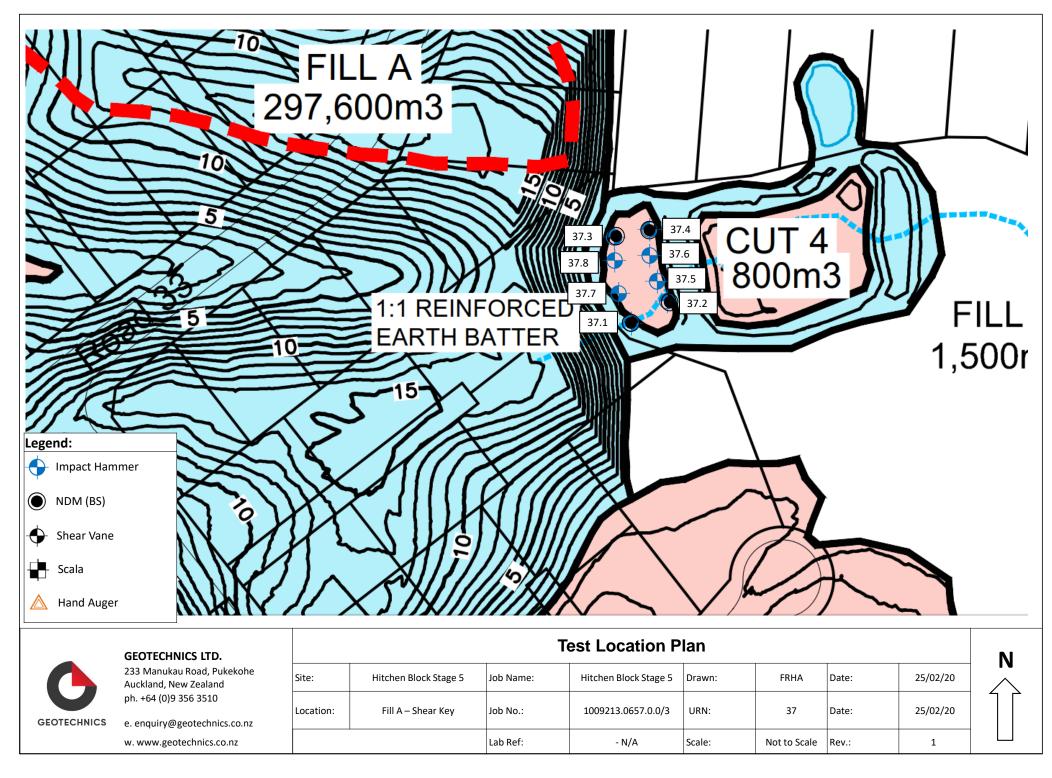


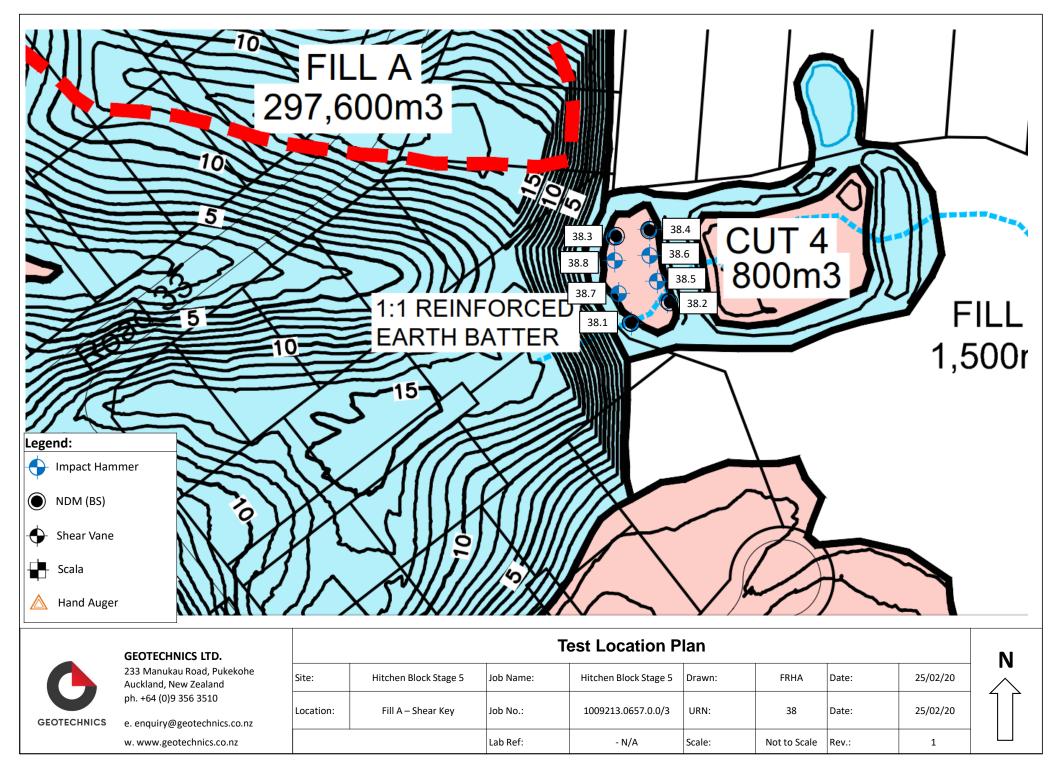


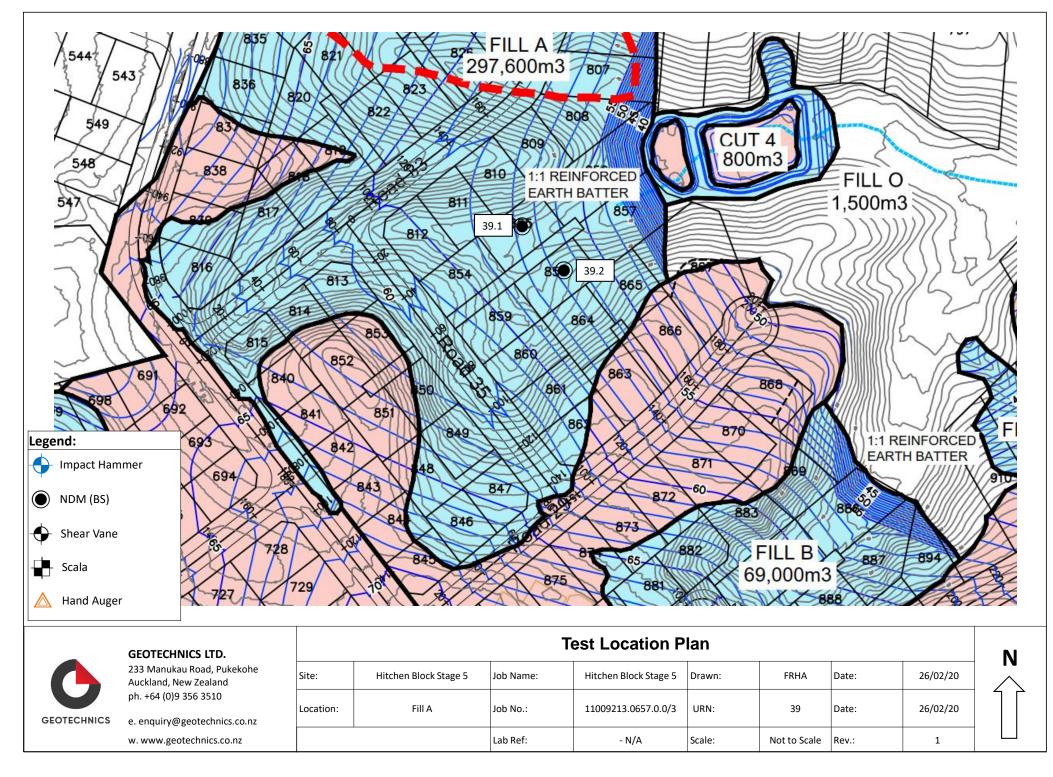


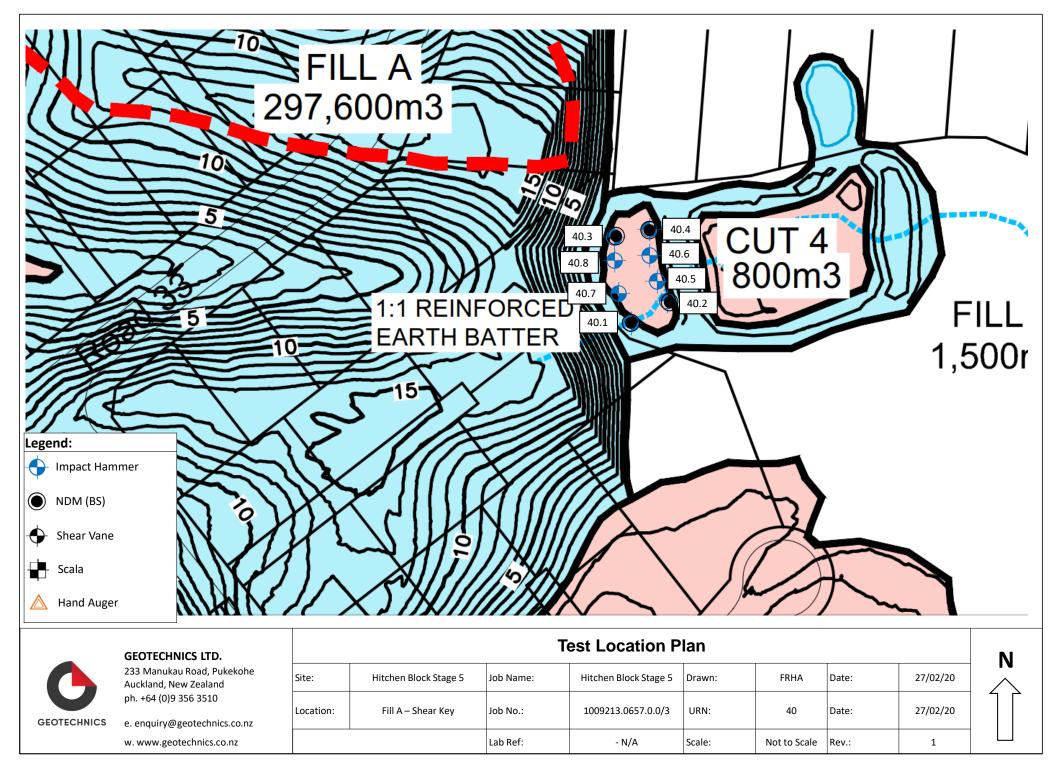


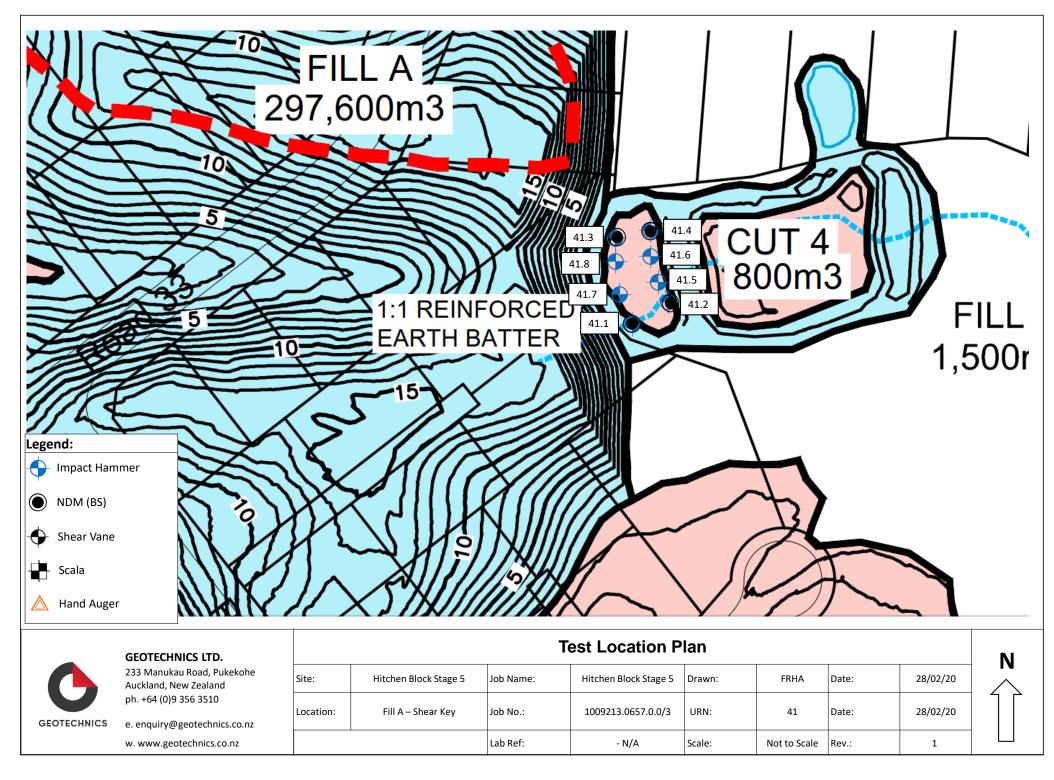


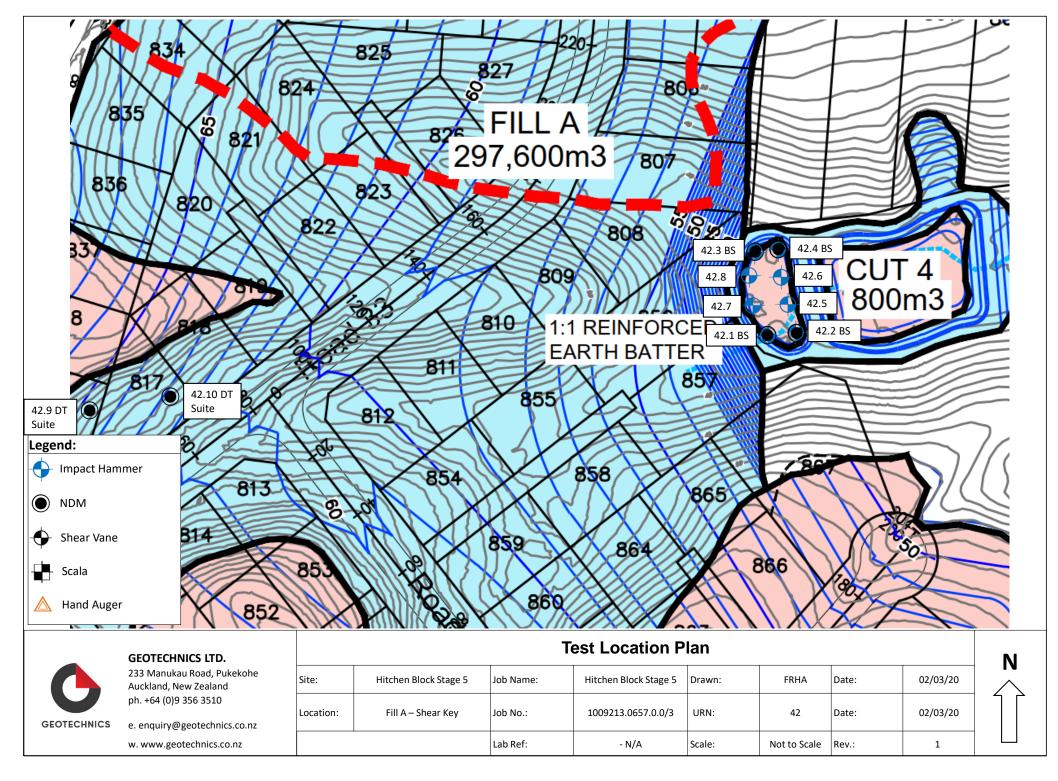


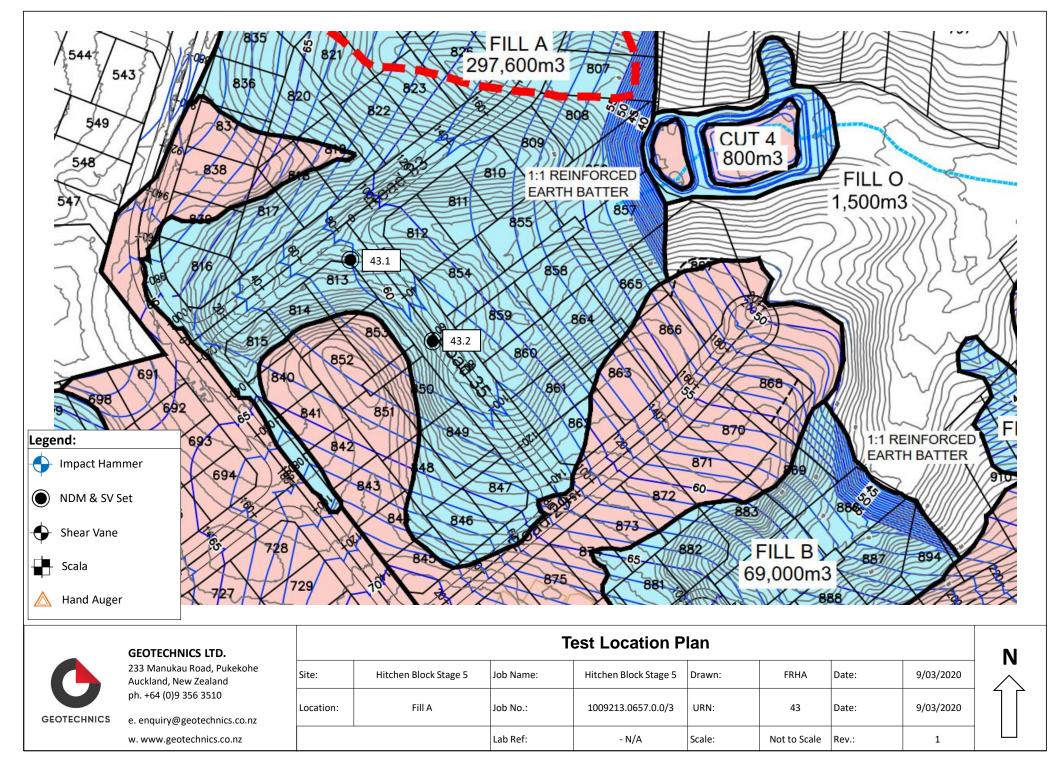


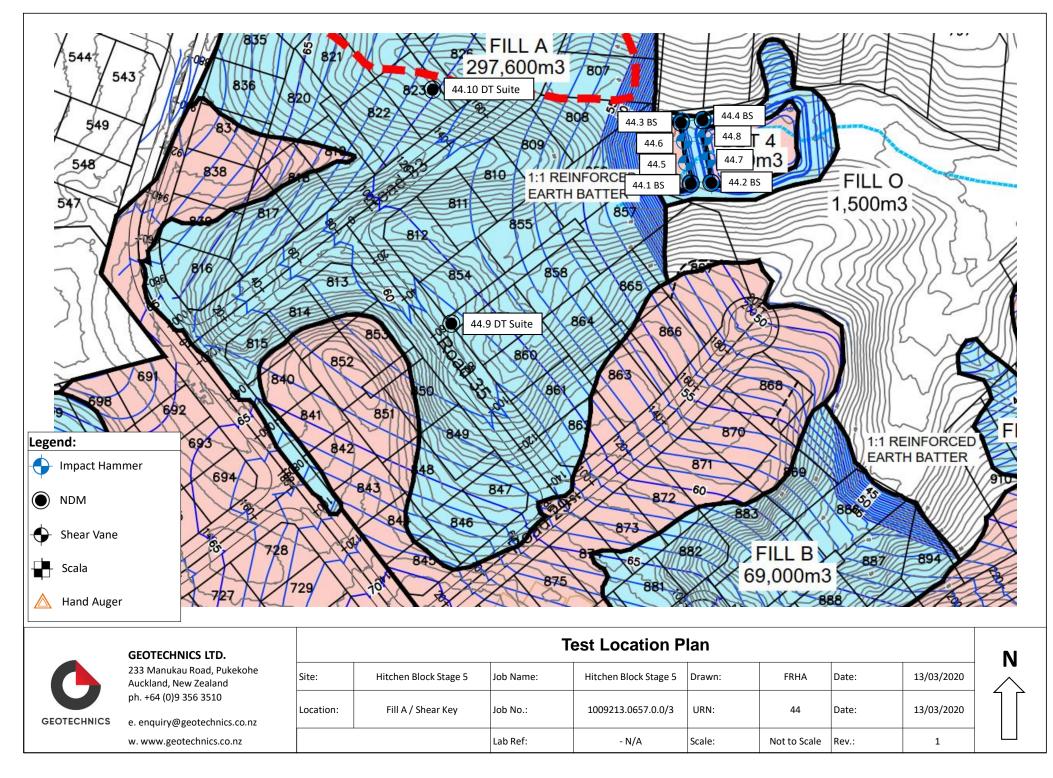


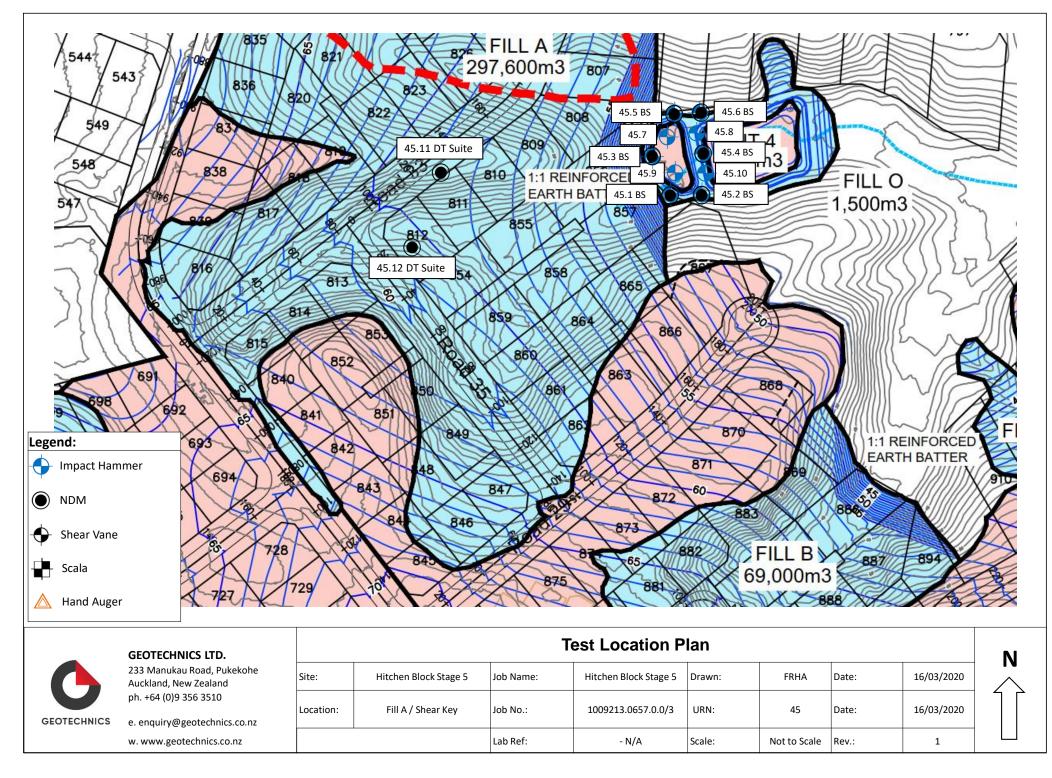


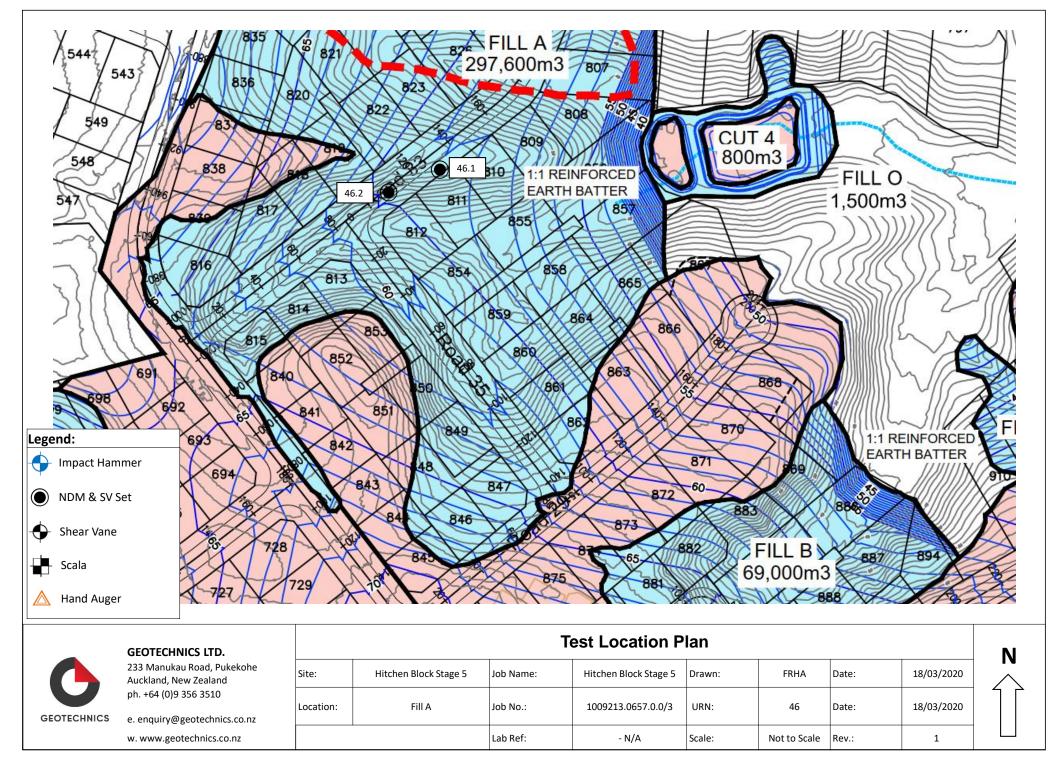


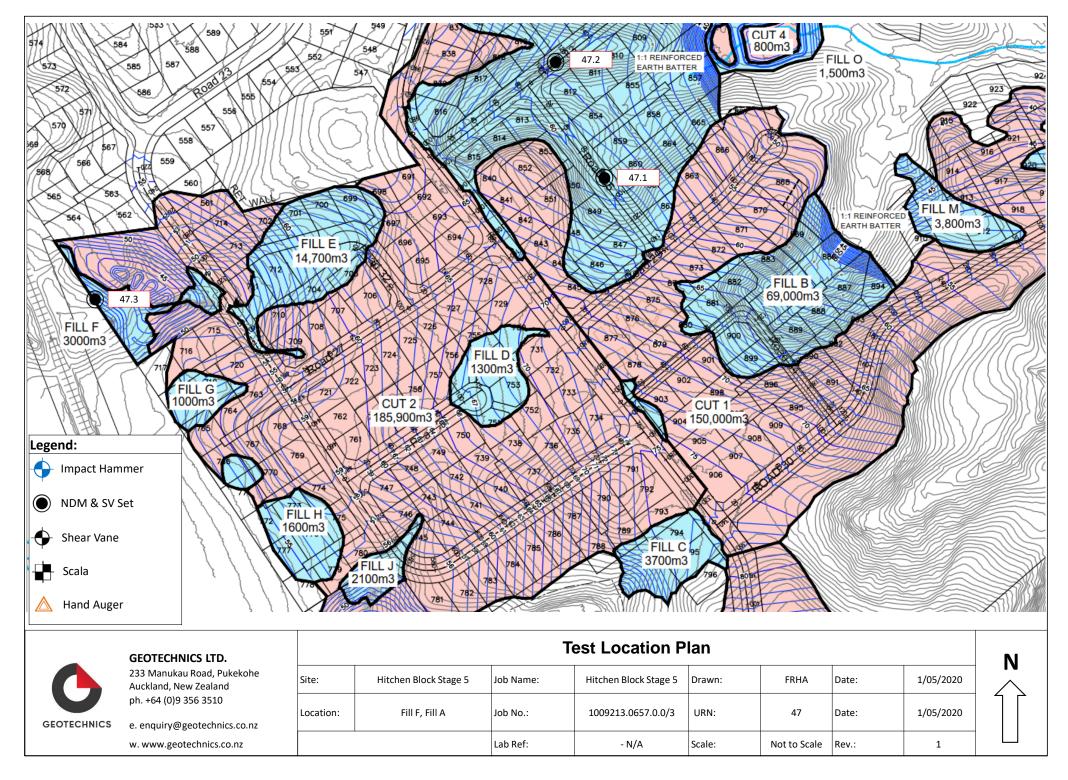


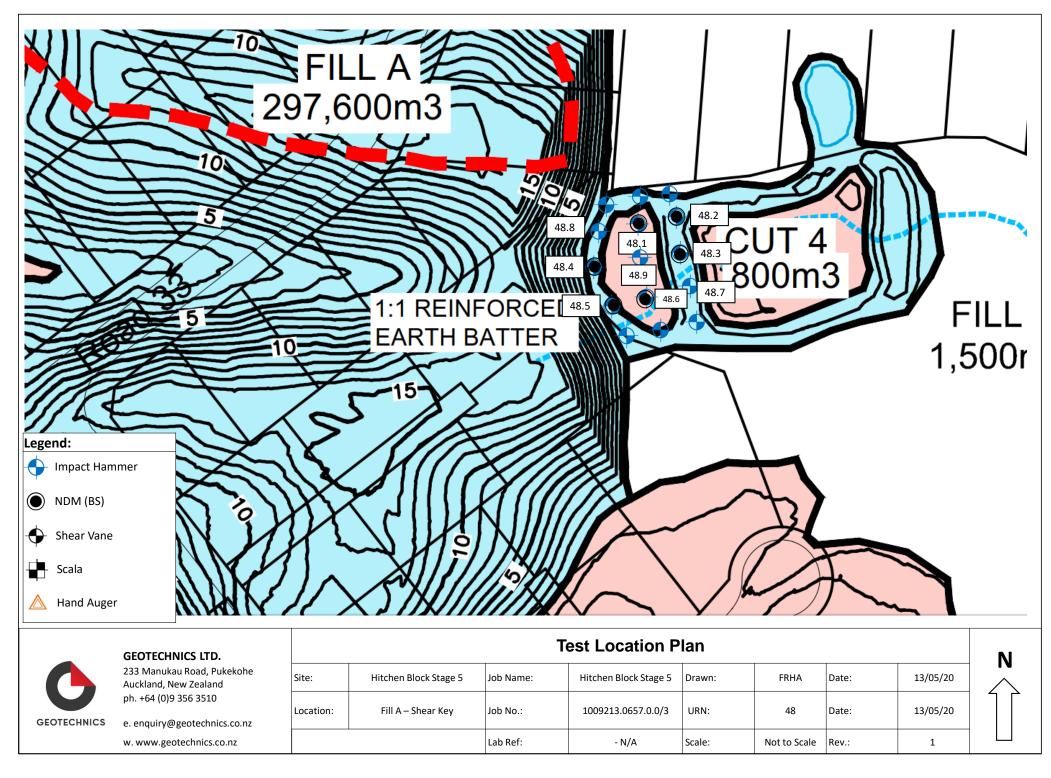


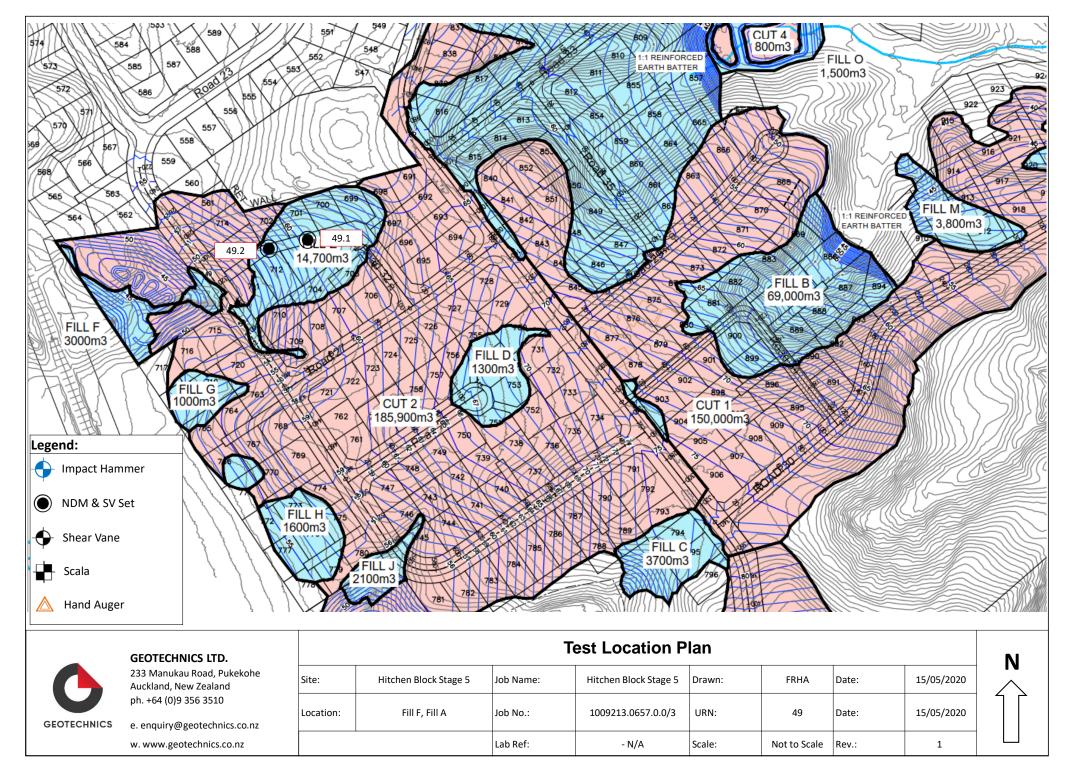






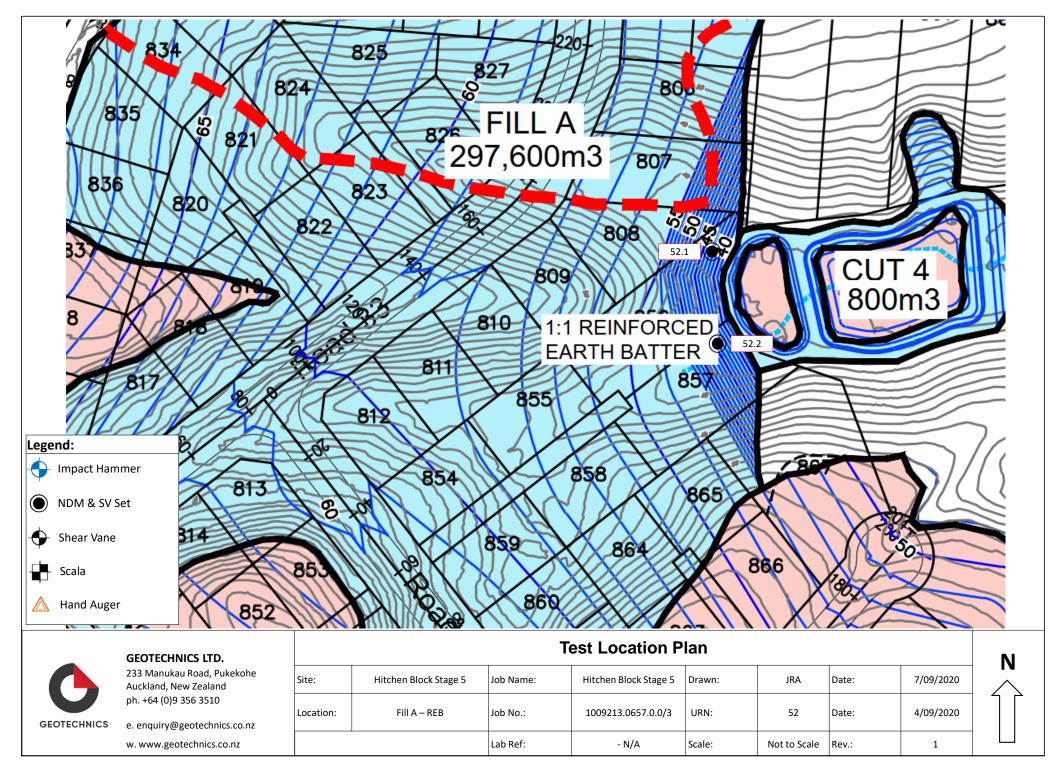


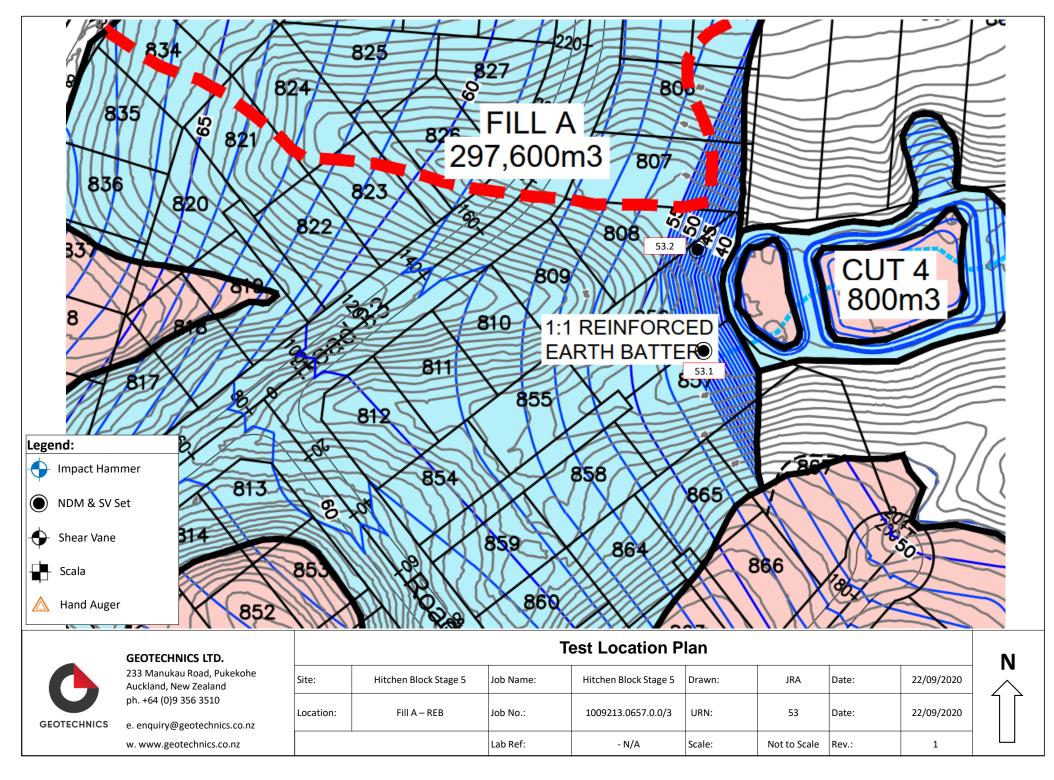


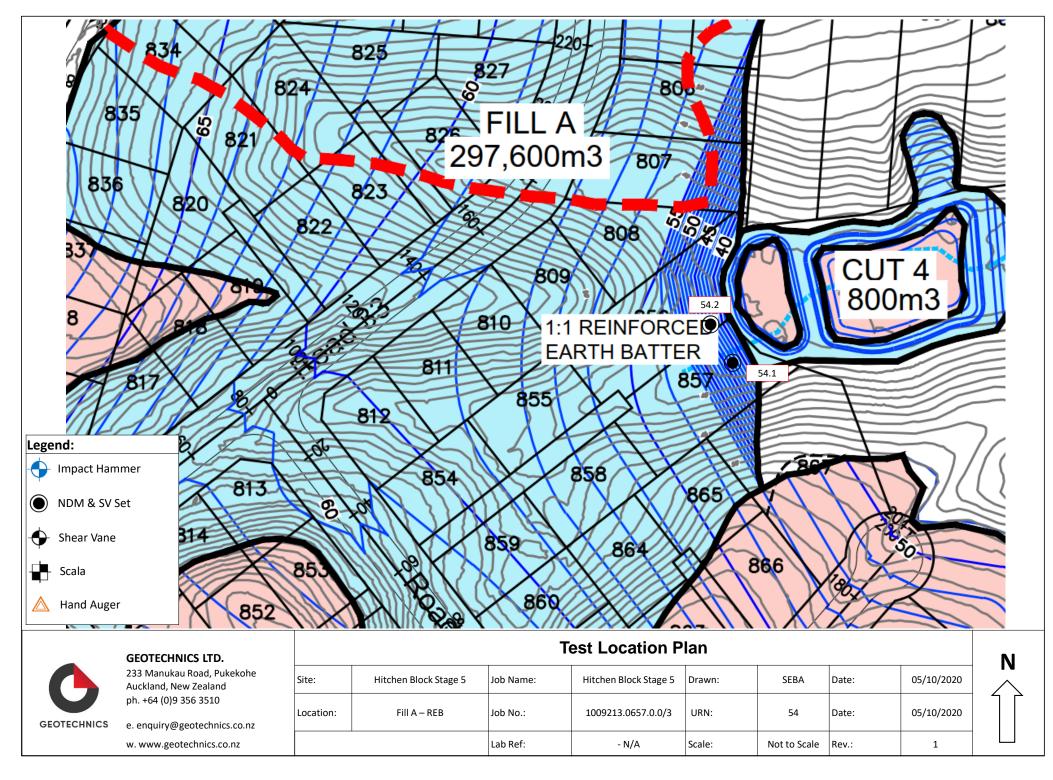


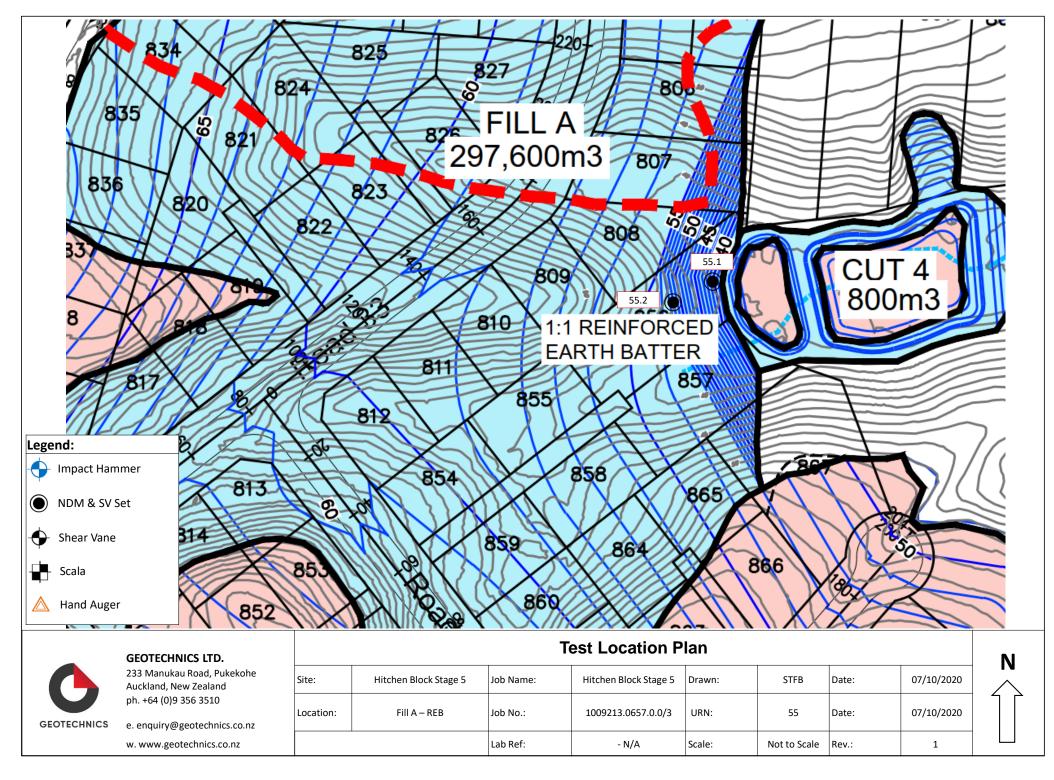
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GEOTECHNICS LTD.	Test Location Plan							N	
233 Manukau Road, Pukekohe Auckland, New Zealand	Site: H	itchen Block Stage 5	Job Name:	Hitchen Block Stage 5	Drawn:	FRHA	Date:	18/05/2020	
ph. +64 (0)9 356 3510 GEOTECHNICS e. enquiry@geotechnics.co.nz	Location:	Fill A – Shear Key	Job No.: Lab Ref:	1009213.0657.0.0/3	URN: Scale:		Date:	18/05/2020	

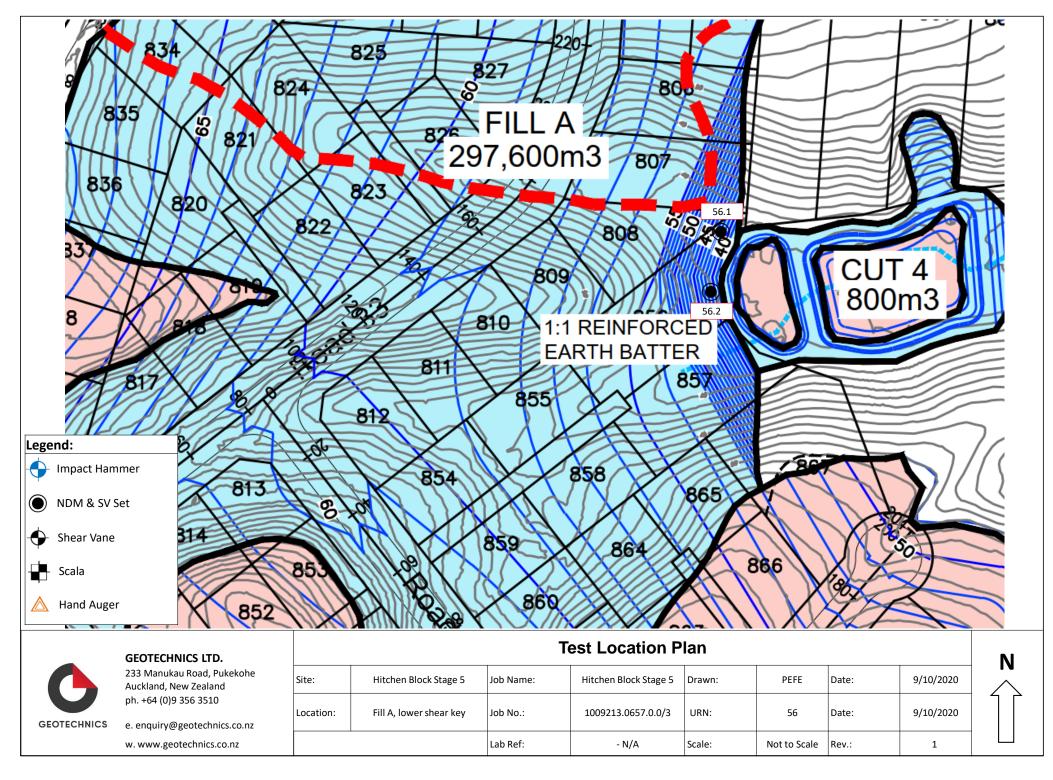
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Hand Auger					Test Location F	Plan				
	GEOTECHNICS LTD. 233 Manukau Road, Pukekohe Auckland, New Zealand	Site:	Hitchen Block Stage 5	Job Name:	Hitchen Block Stage 5	Drawn:	FRHA	Date:	17/06/2020	N ^
GEOTECHNICS	e. enquiry@geotechnics.co.nz	Location:	Fill A – Shear Key	Job No.:	1009213.0657.0.0/3	URN:	51	Date:	17/06/2020	
	w. www.geotechnics.co.nz			Lab Ref:	- N/A	Scale:	Not to Scale	Rev.:	1	

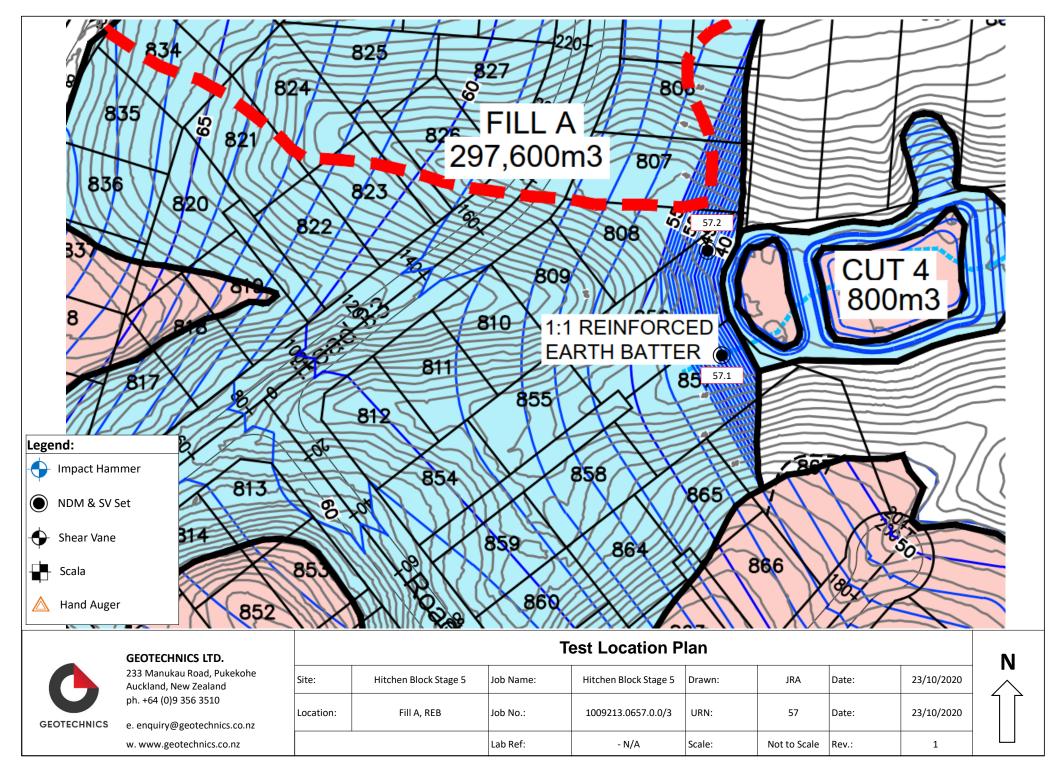


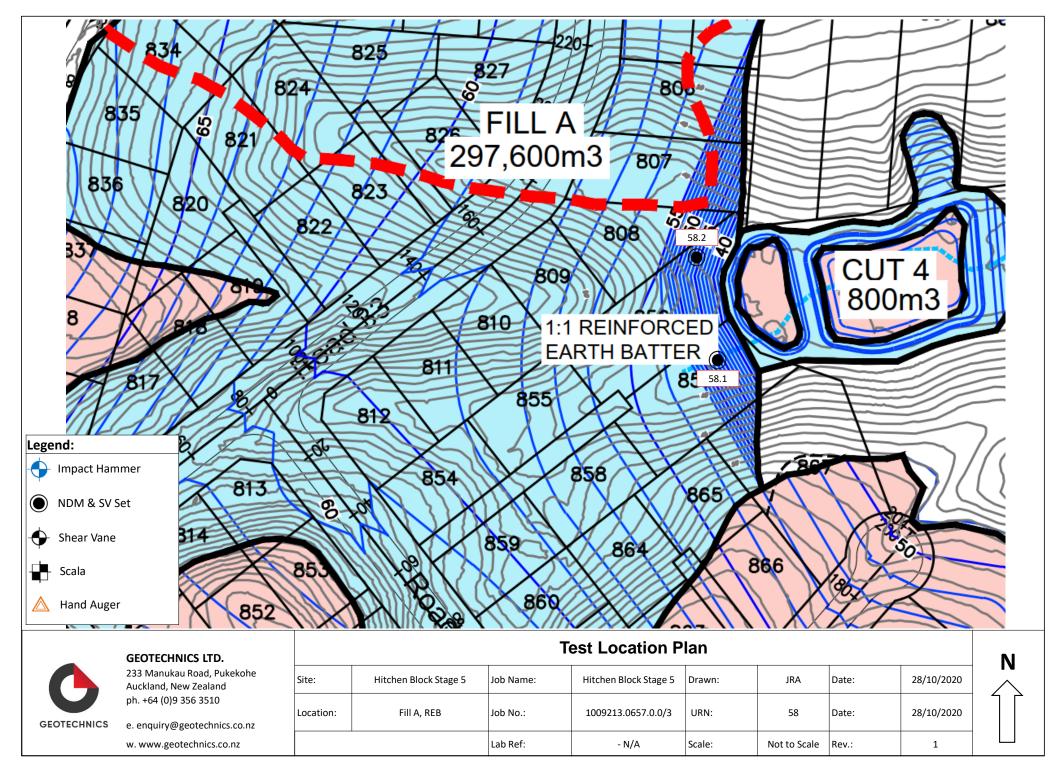


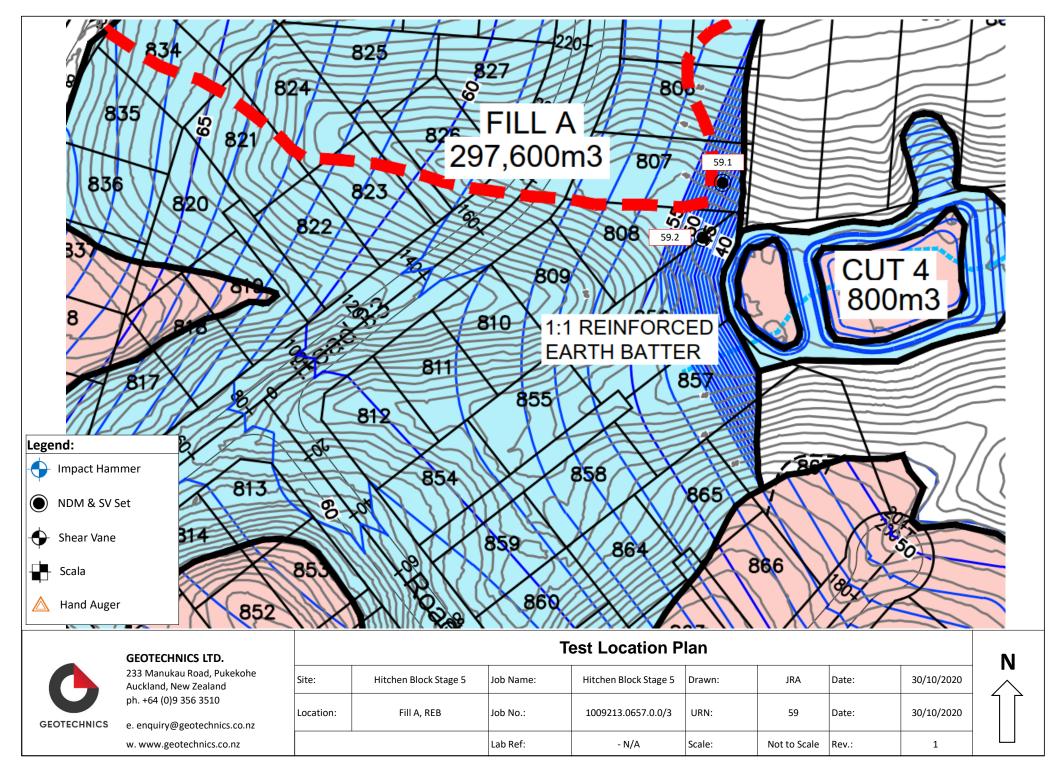


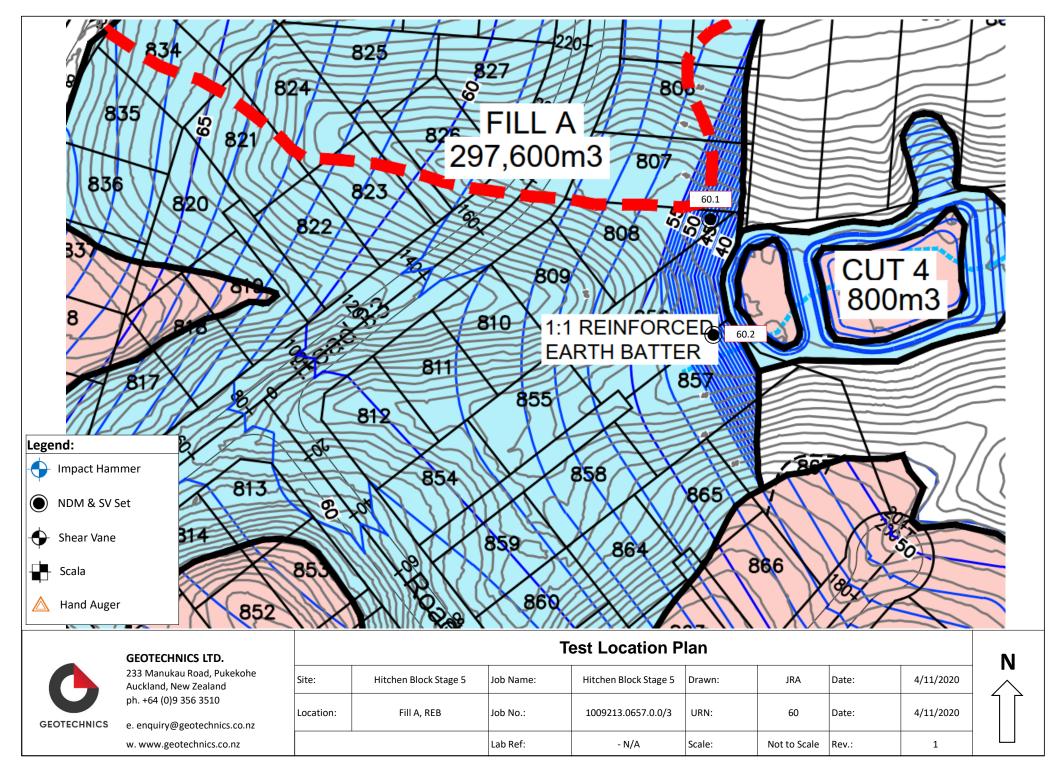


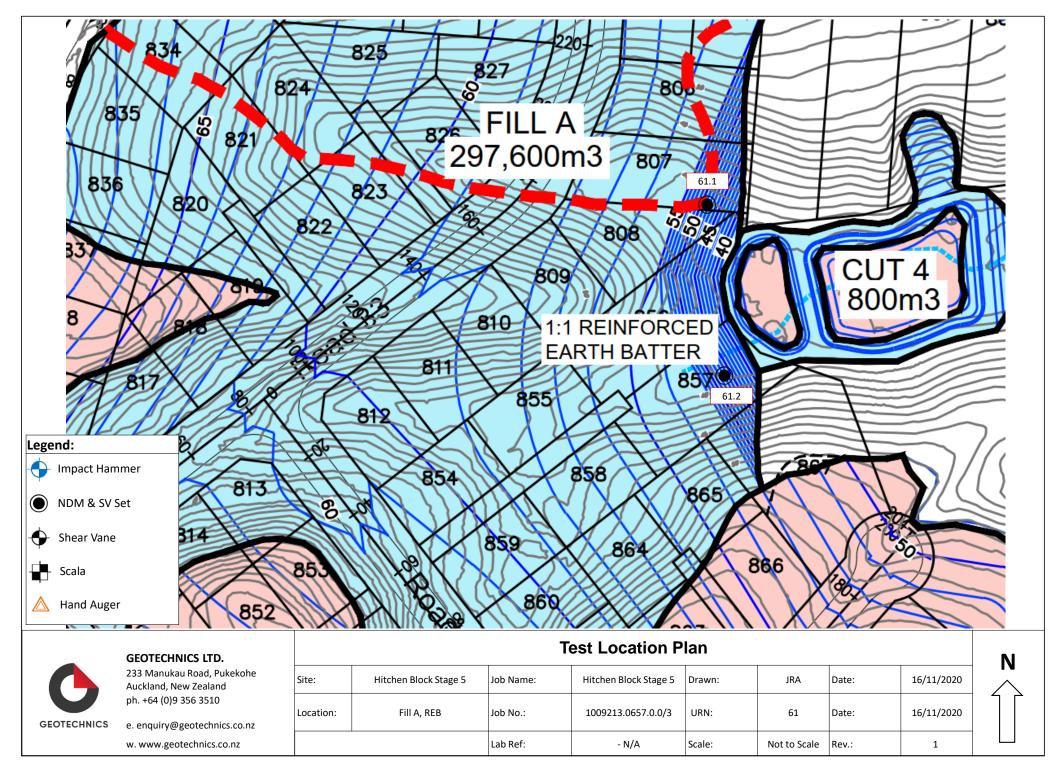


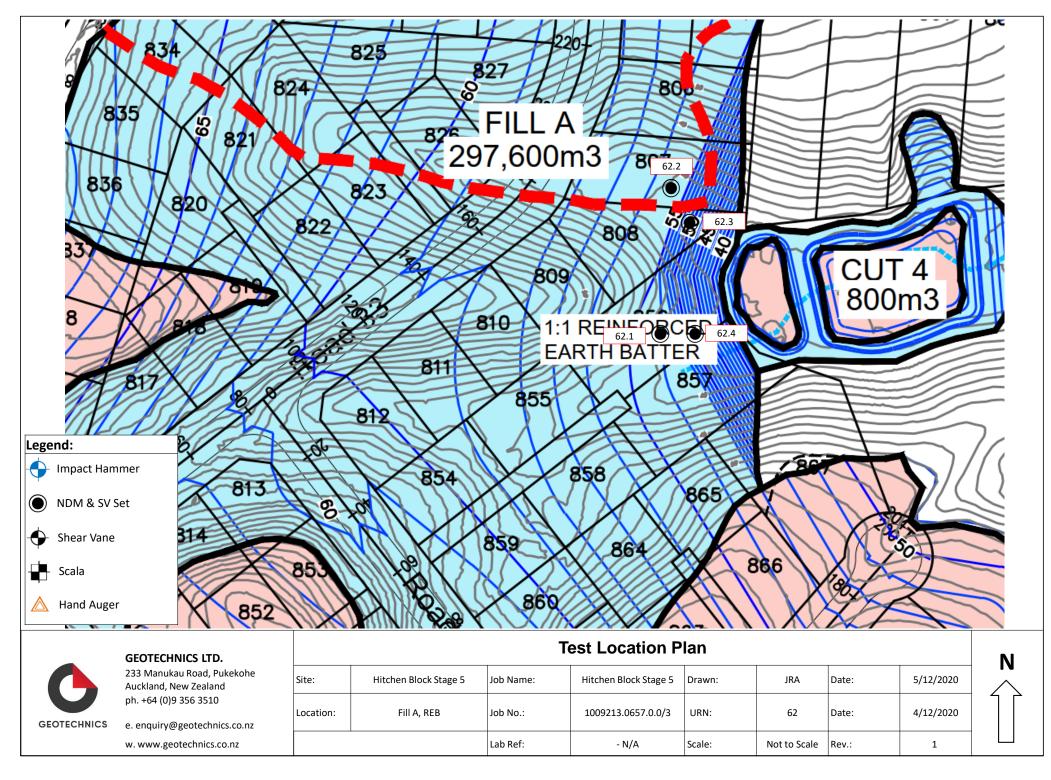


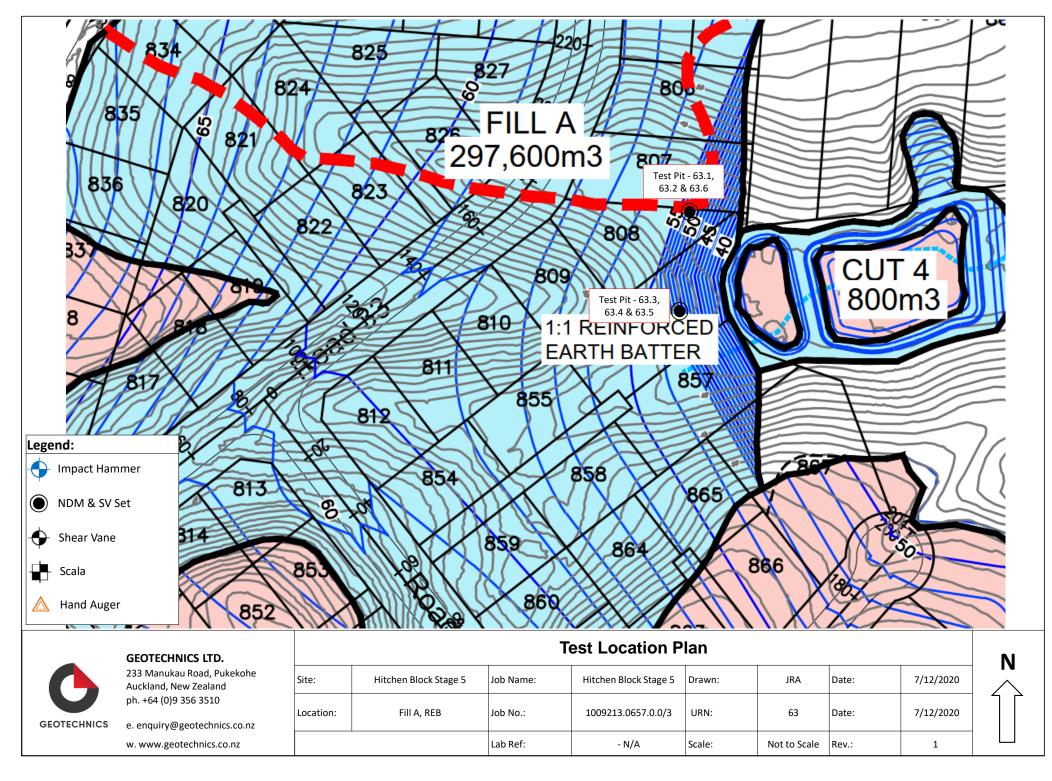


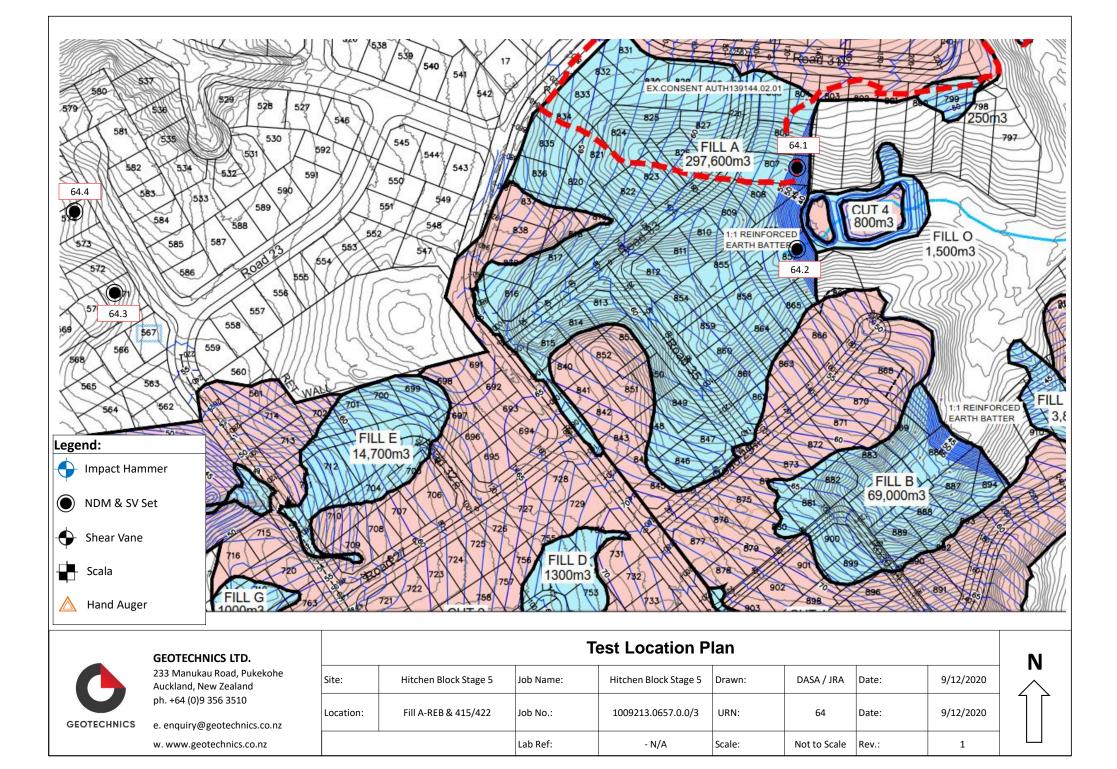


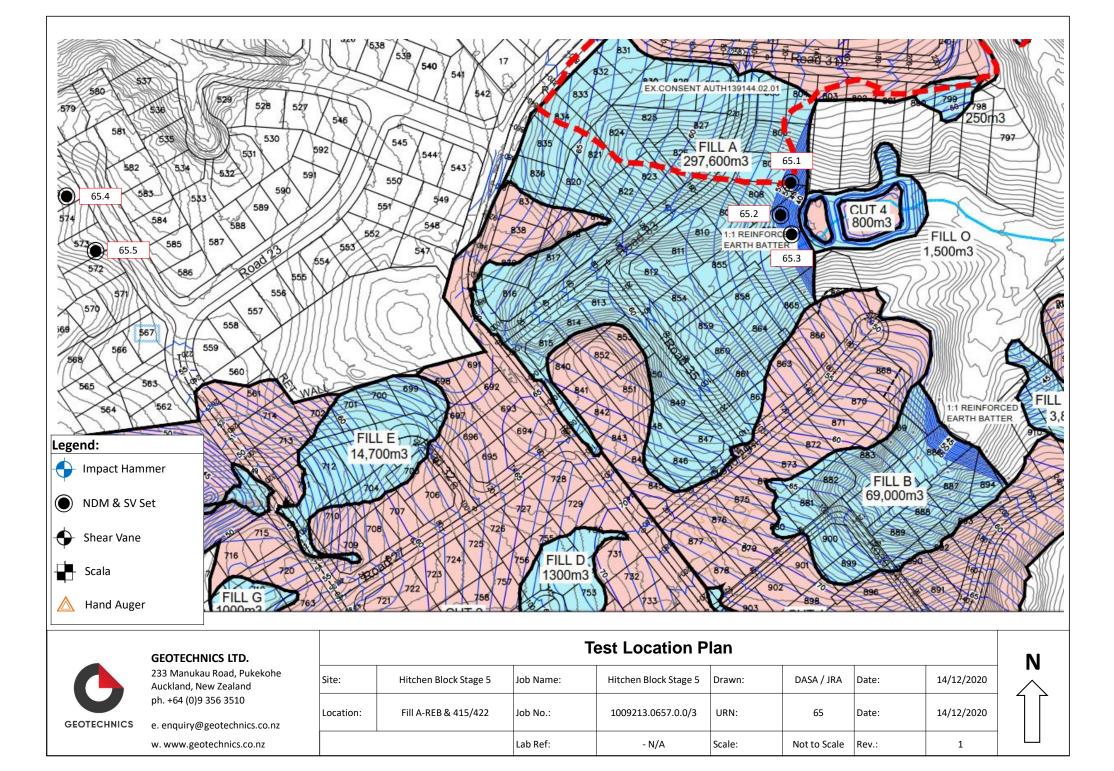


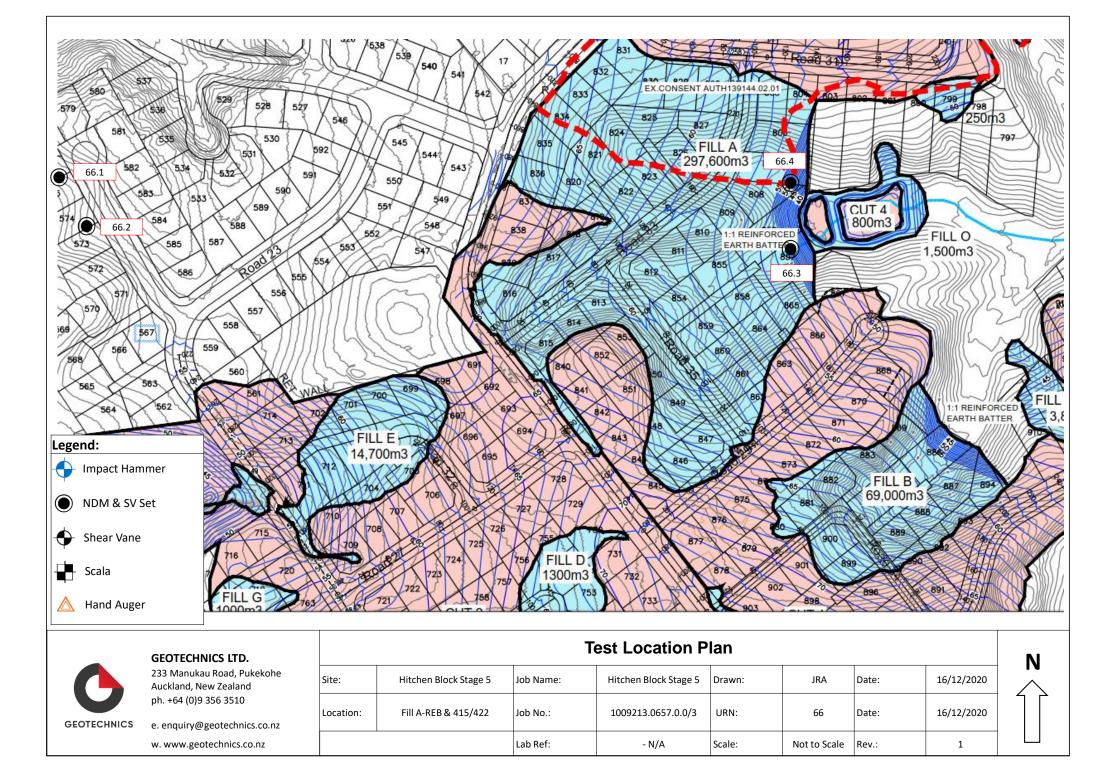


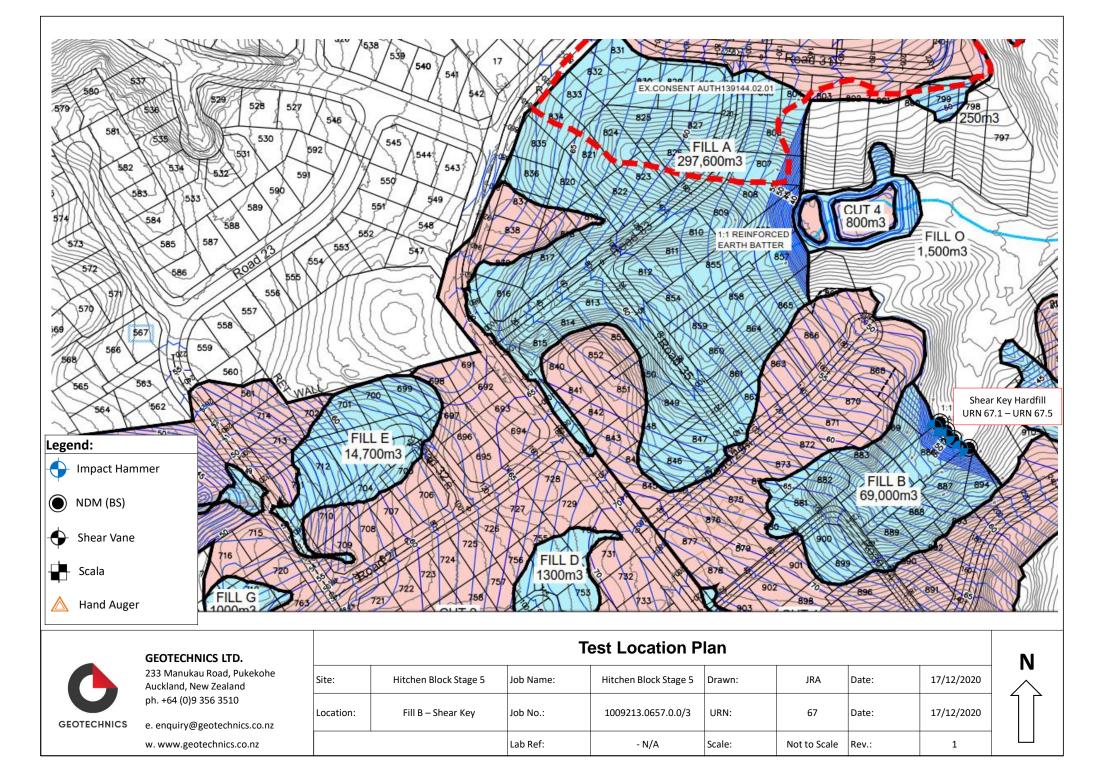


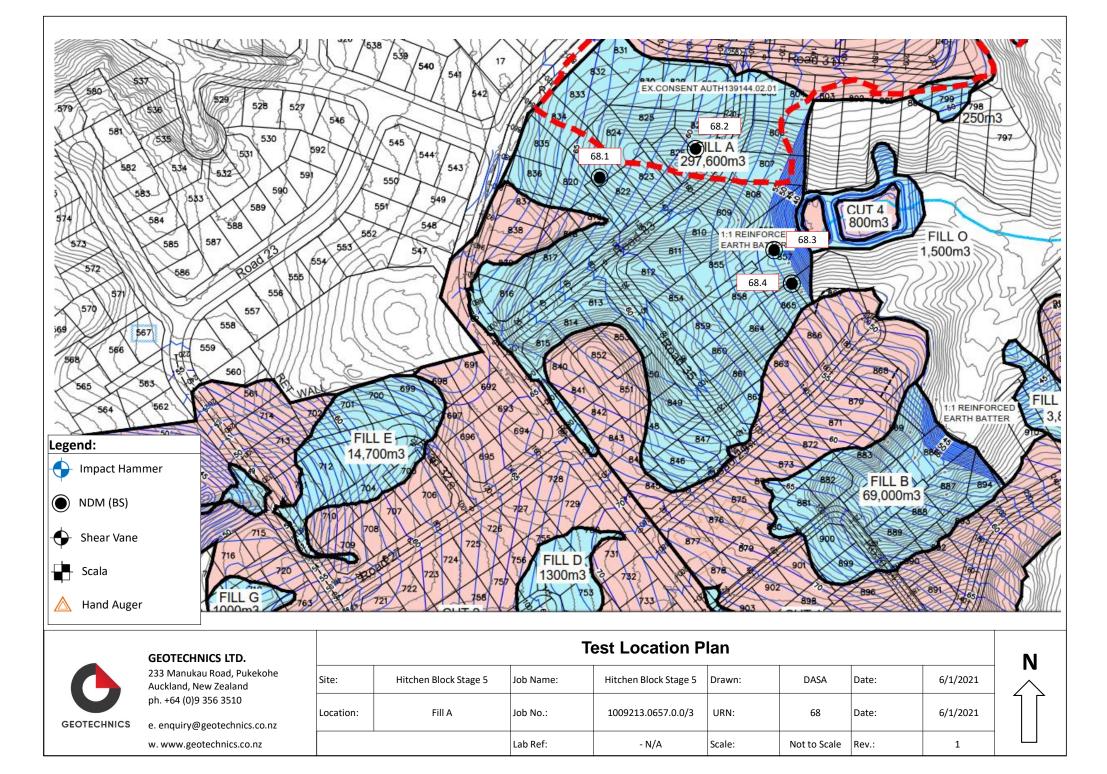


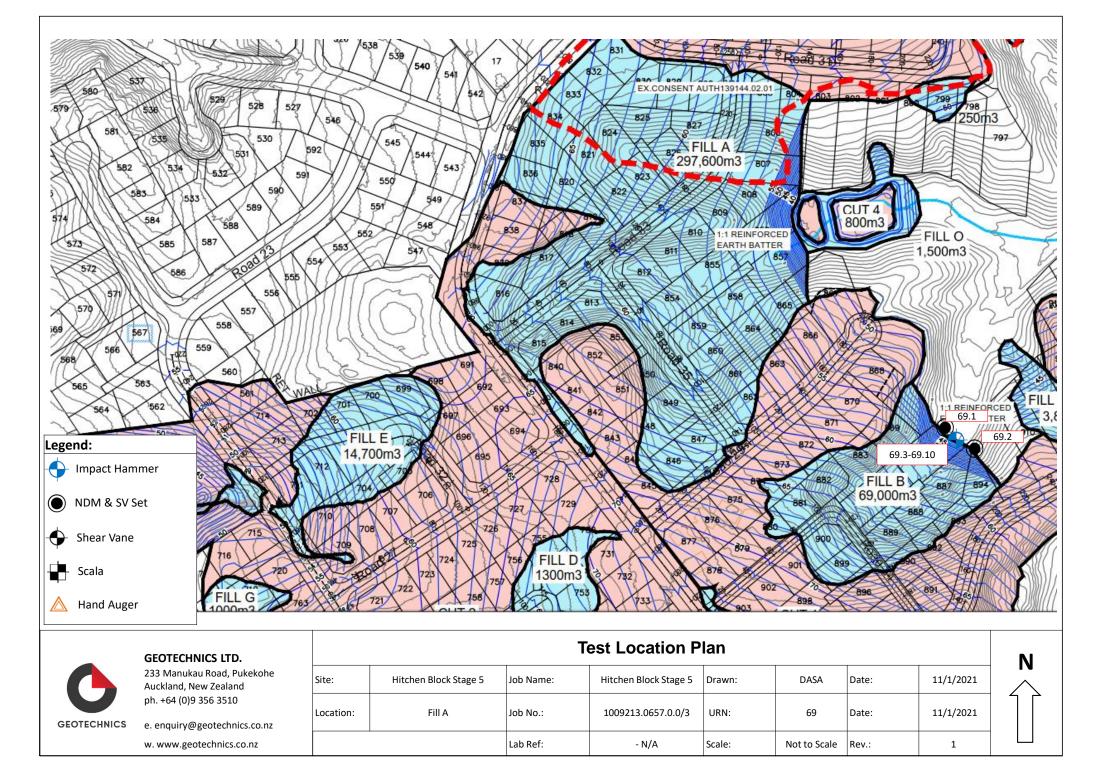


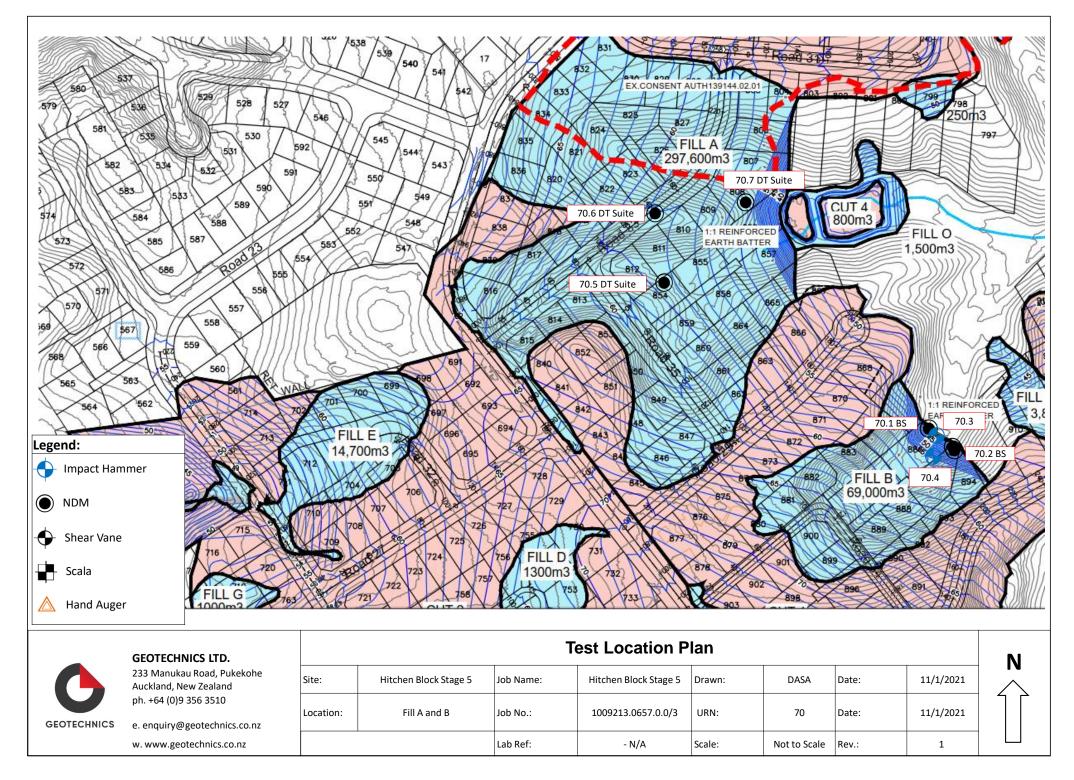


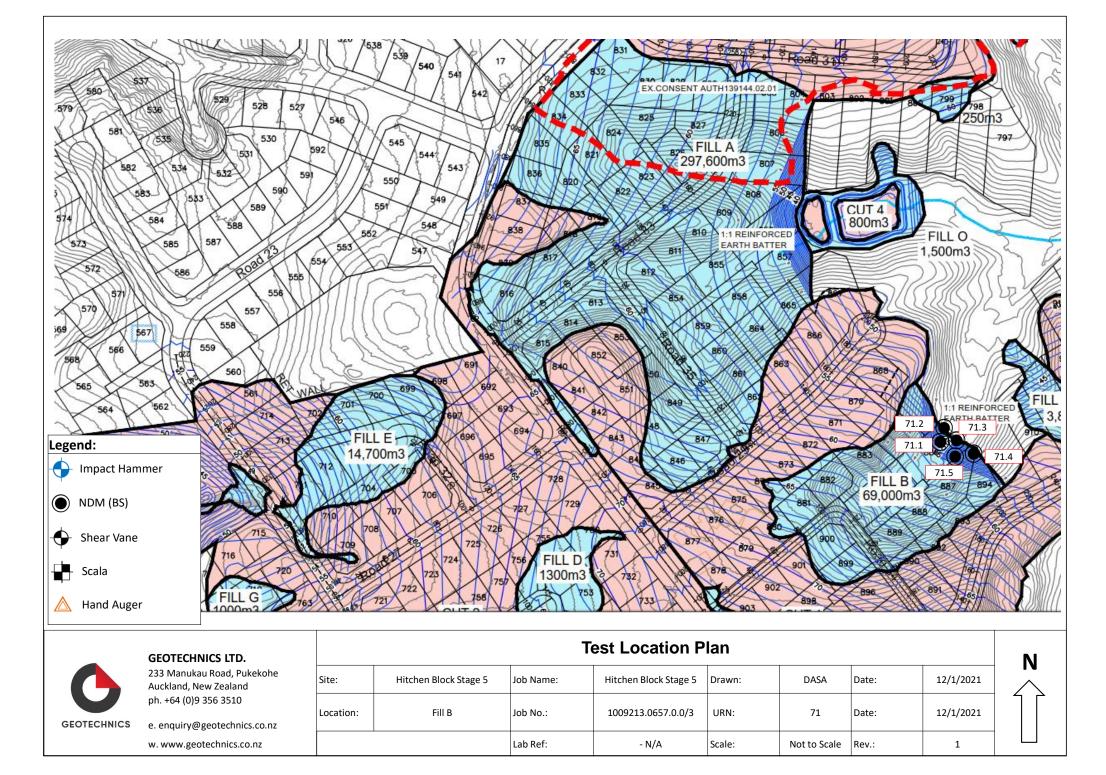


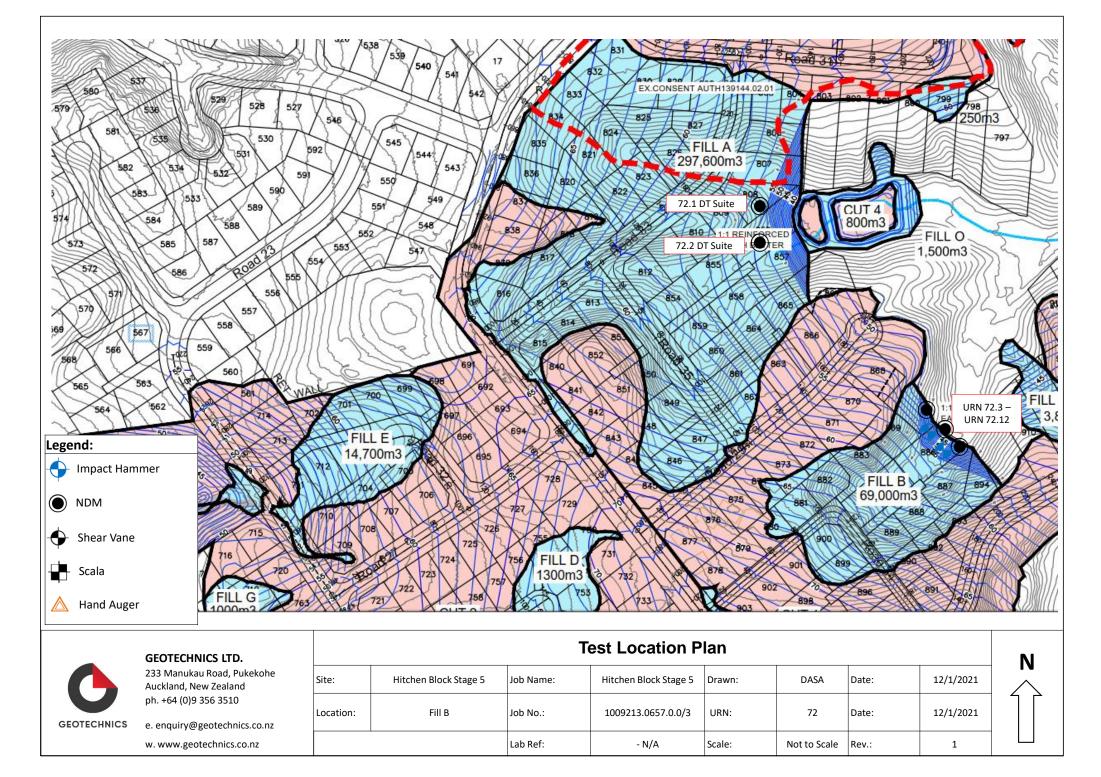


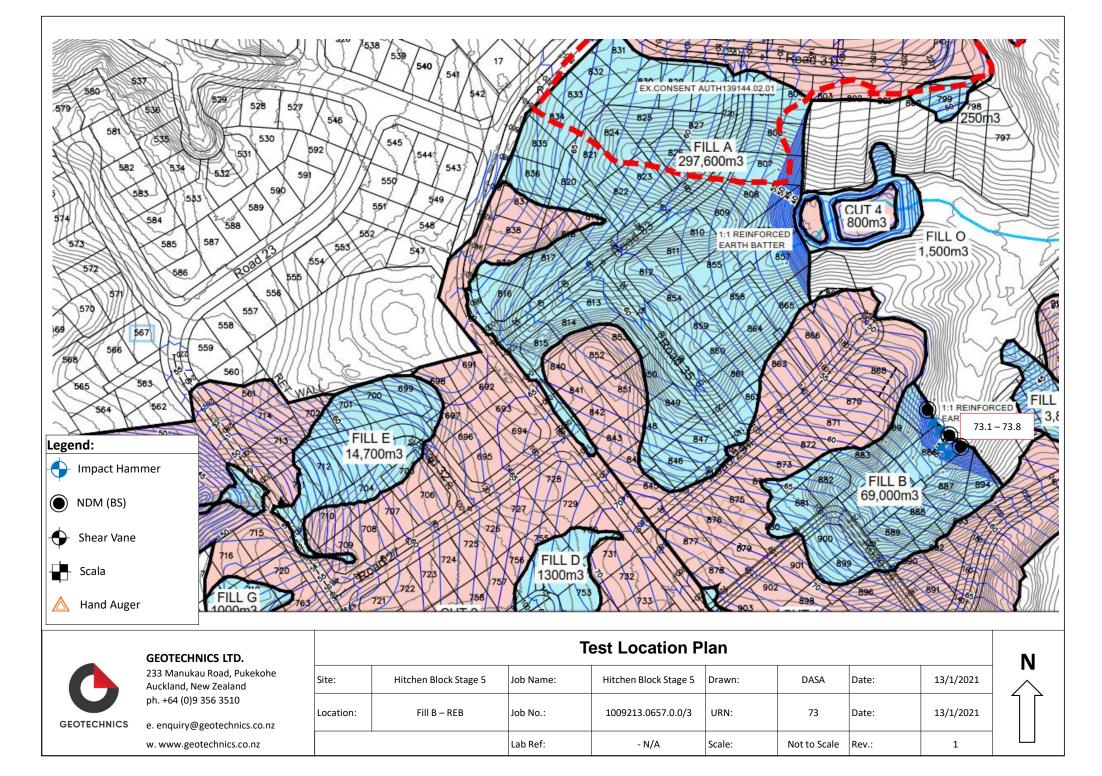


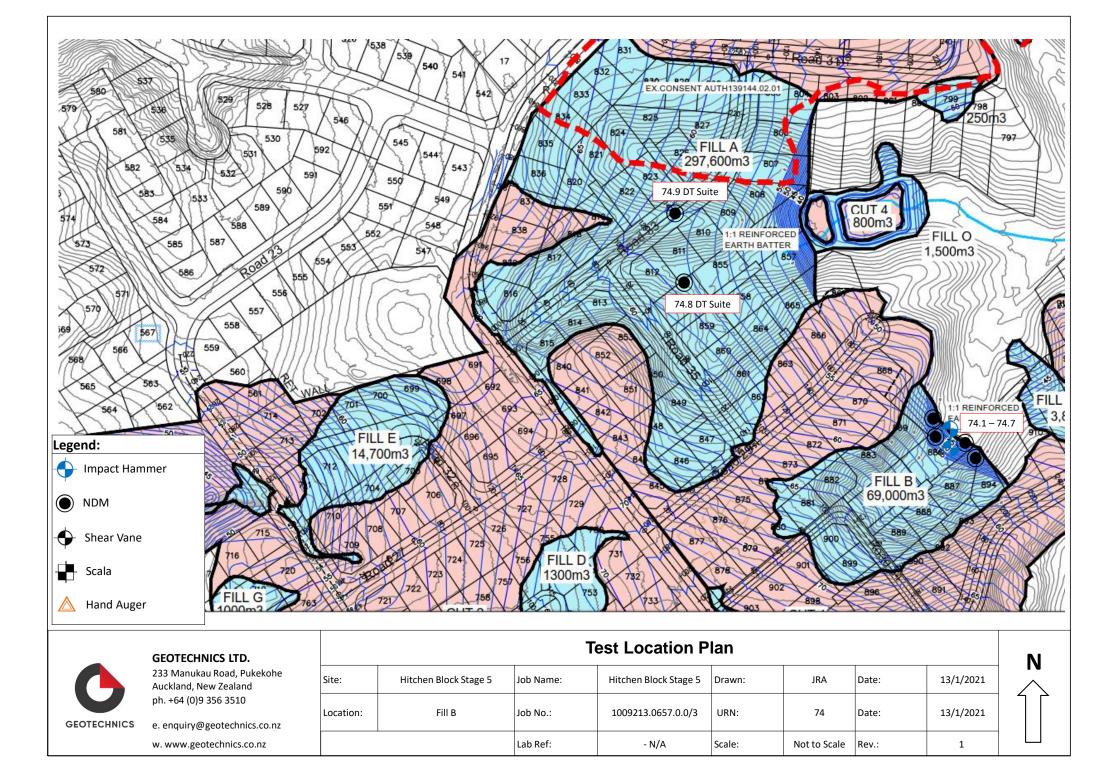


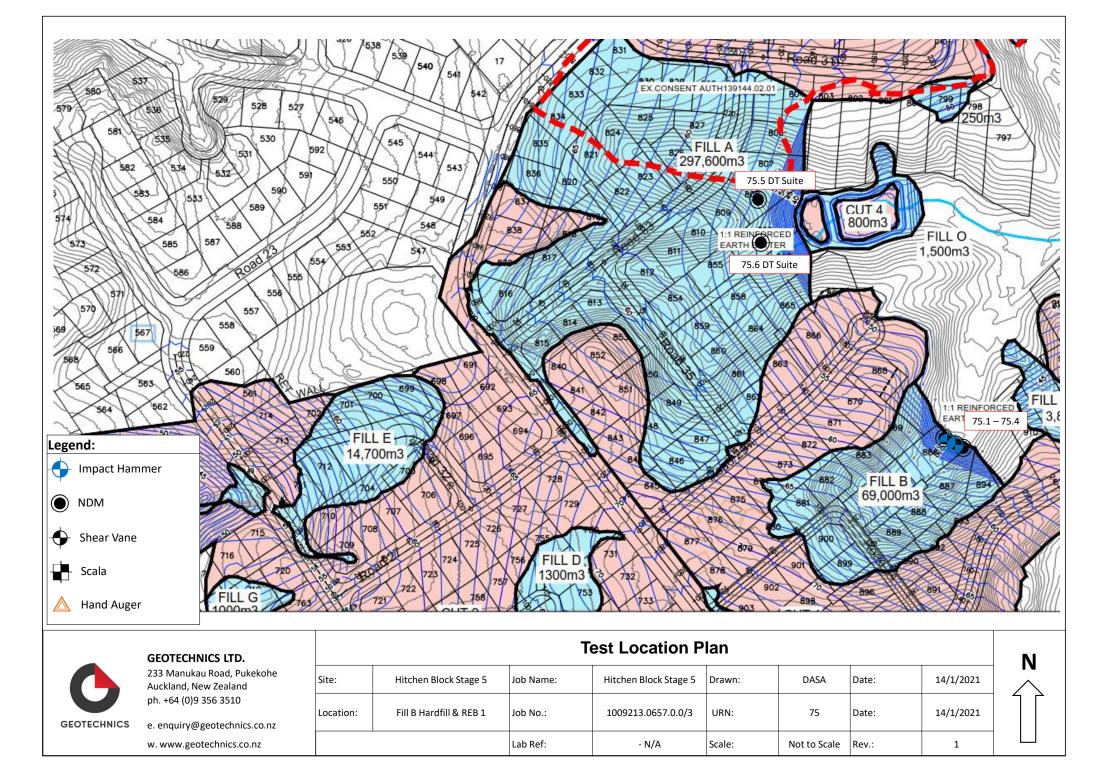


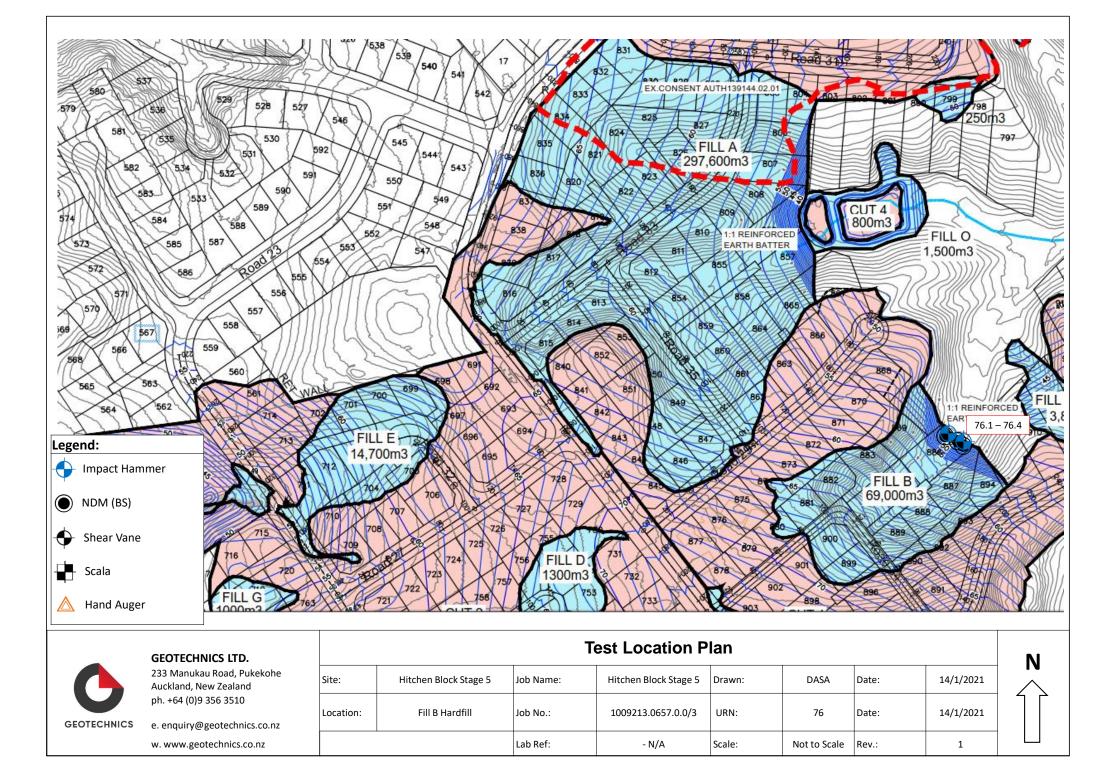


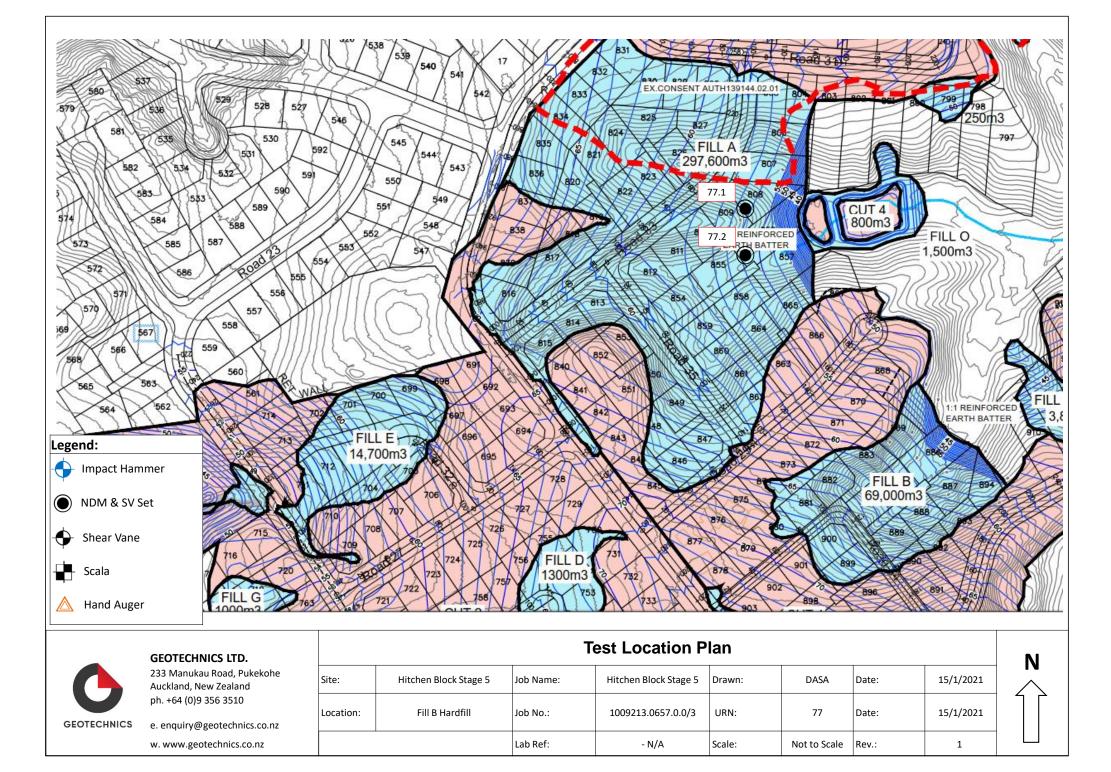


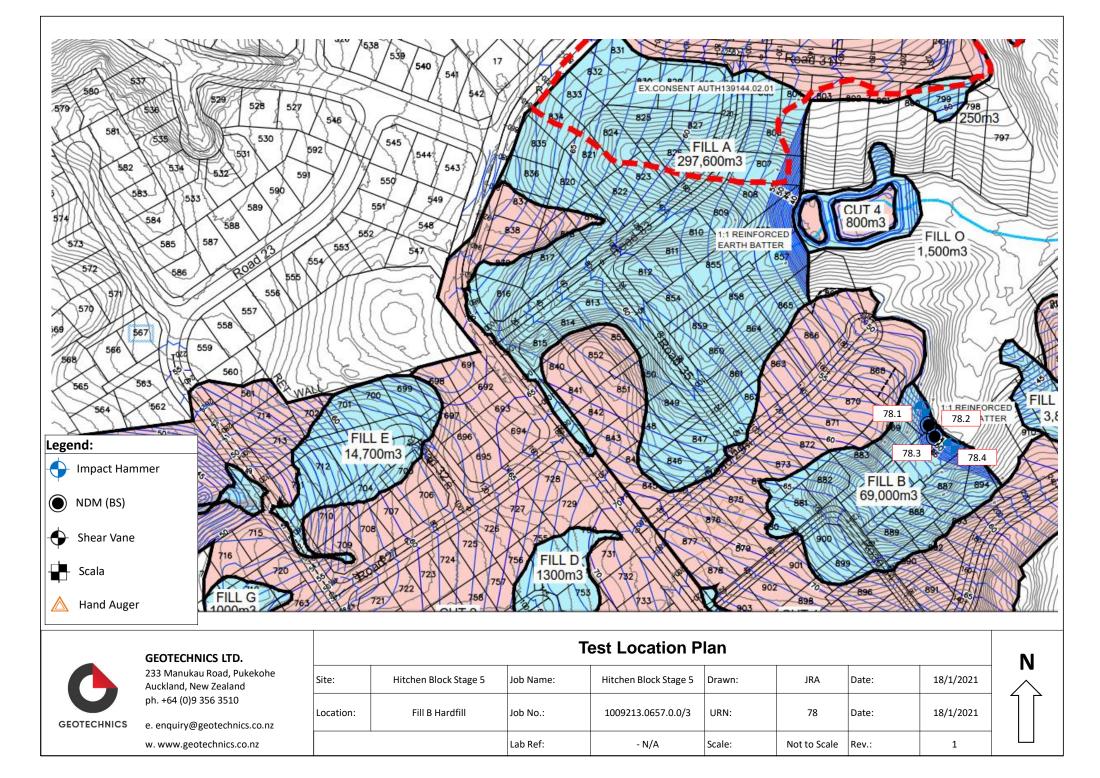


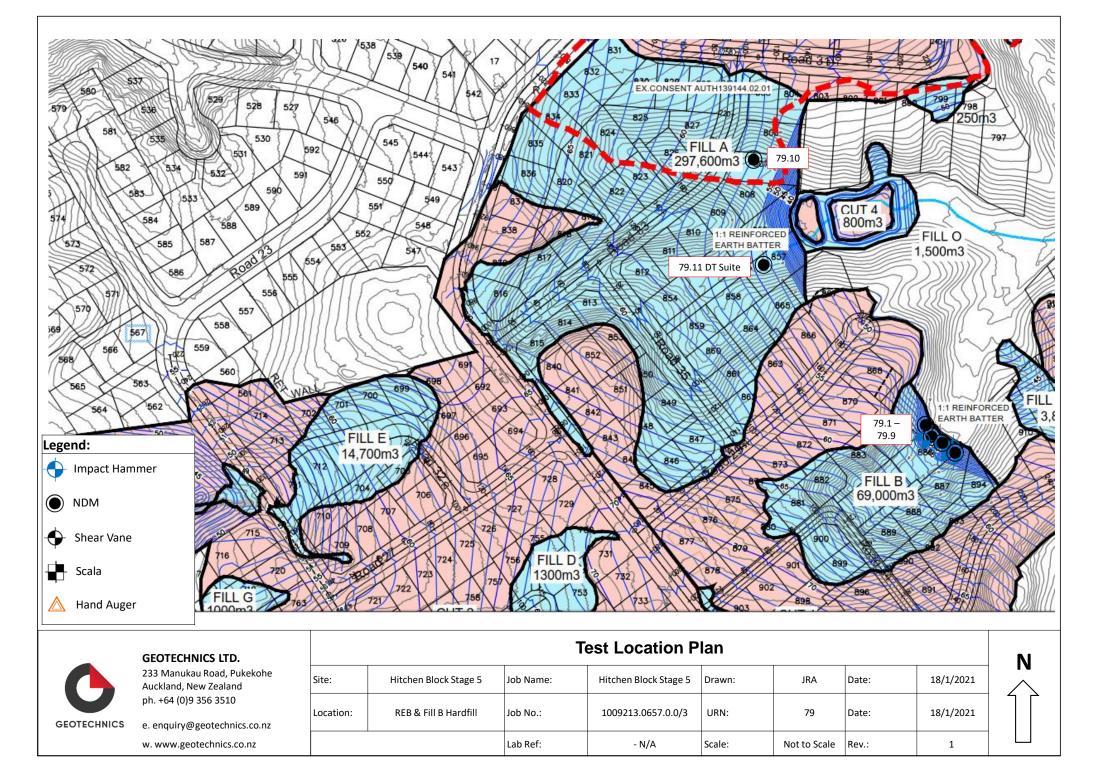


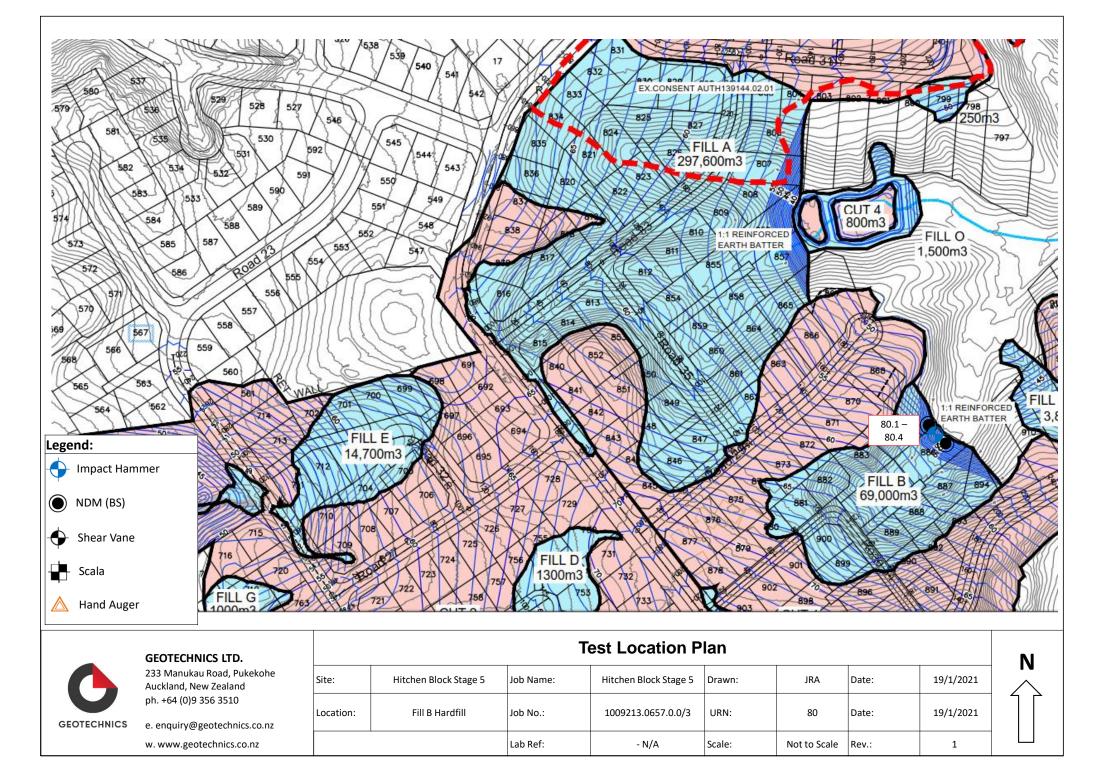


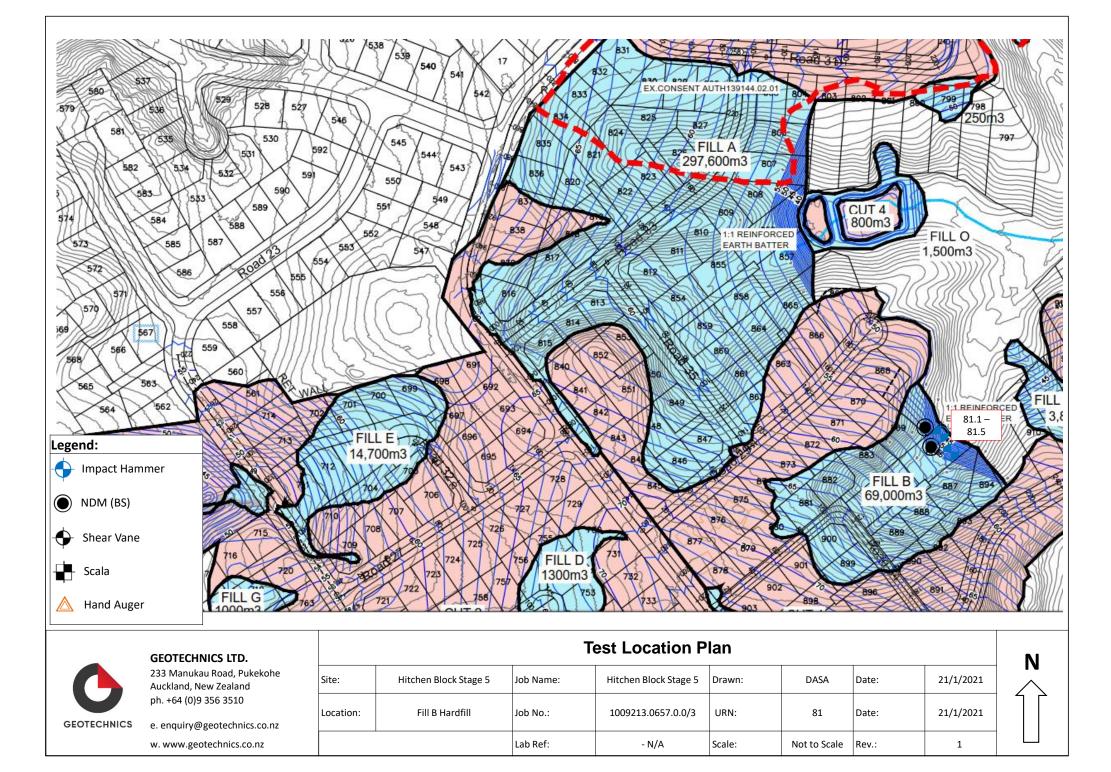


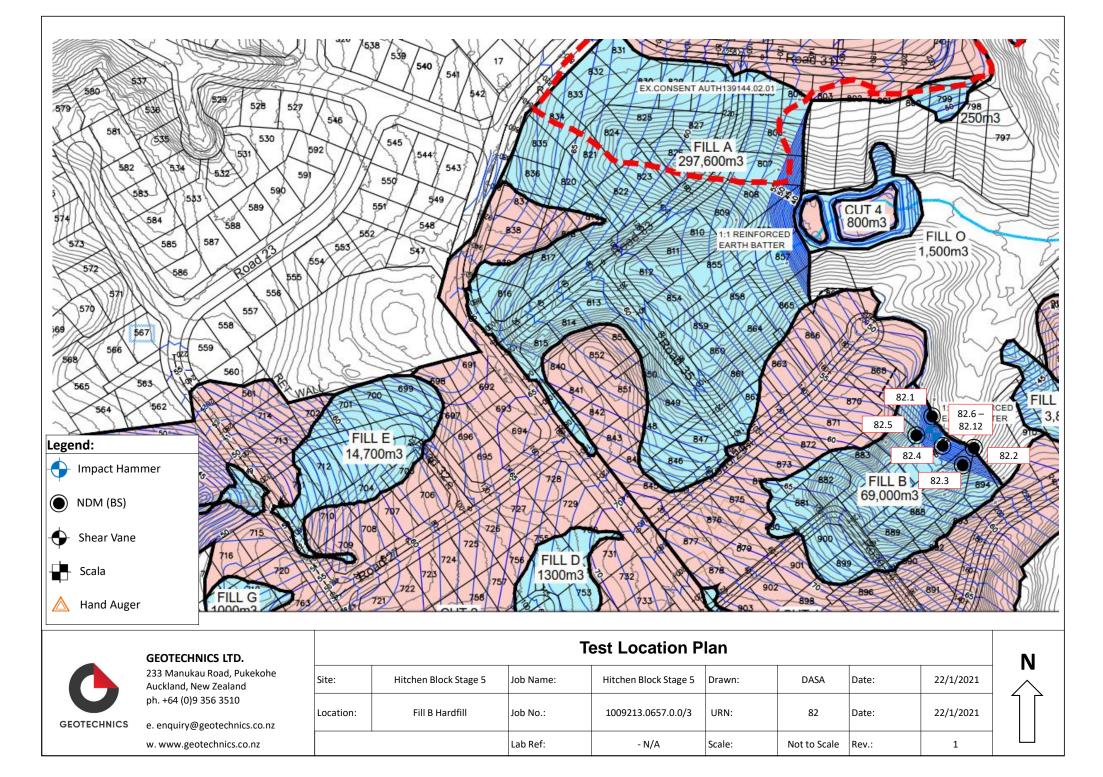


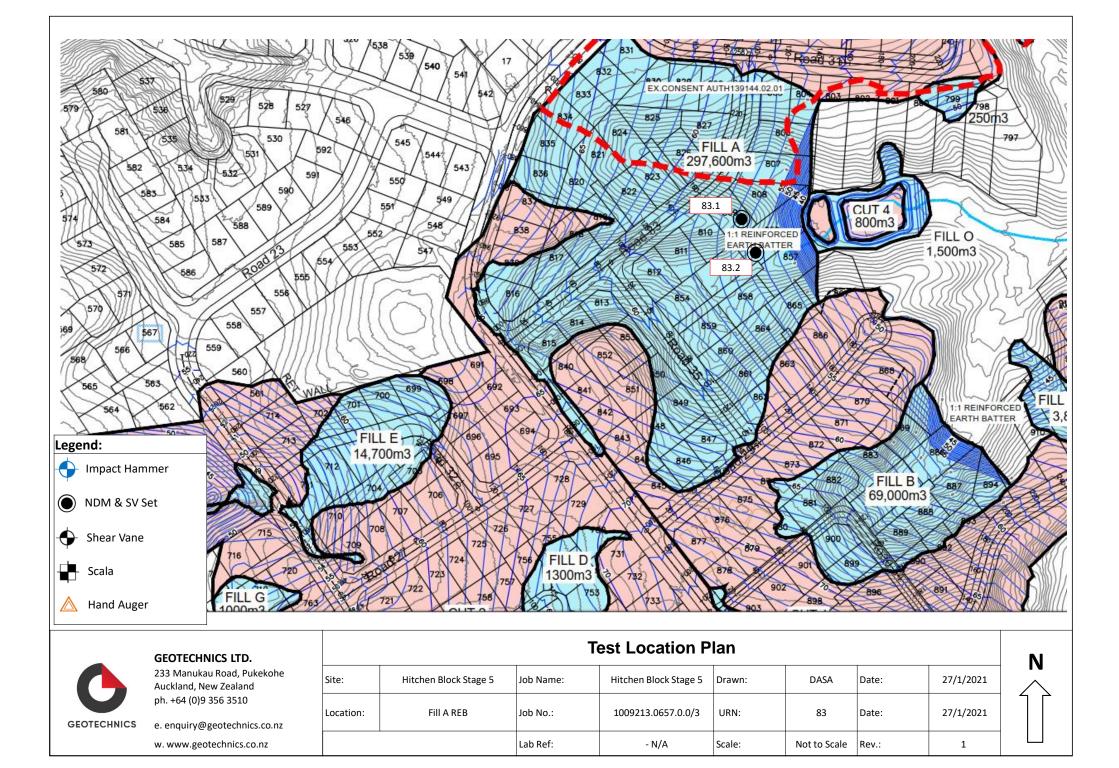


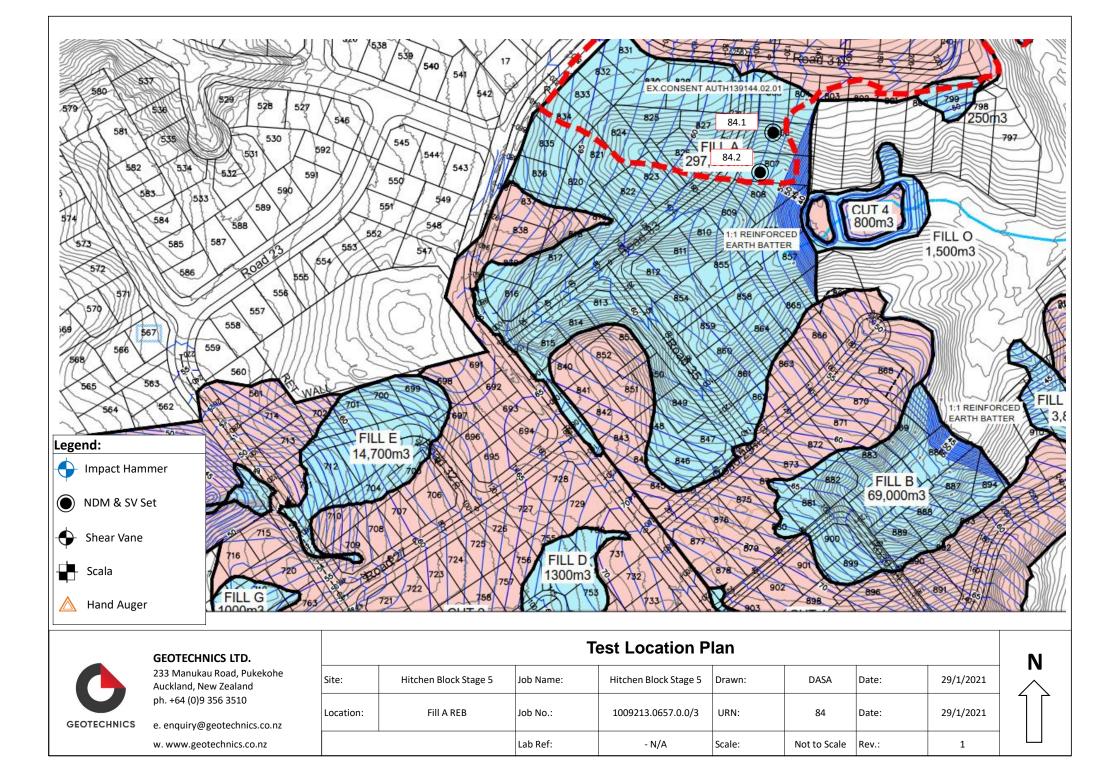


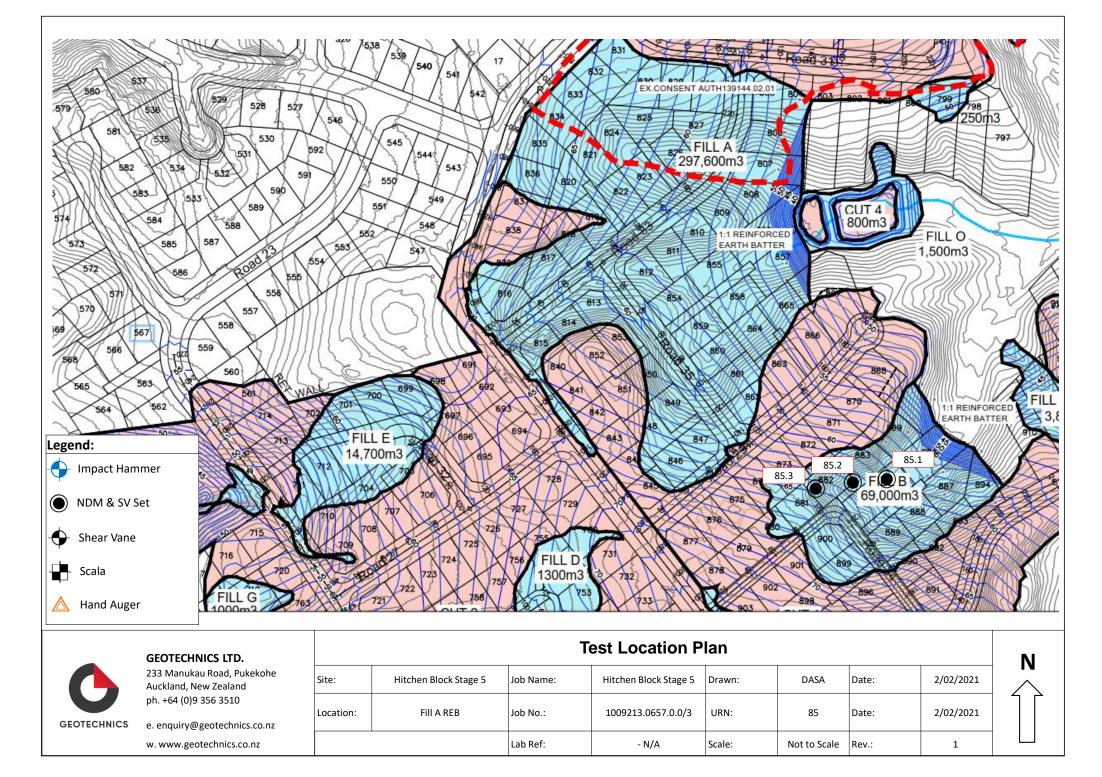


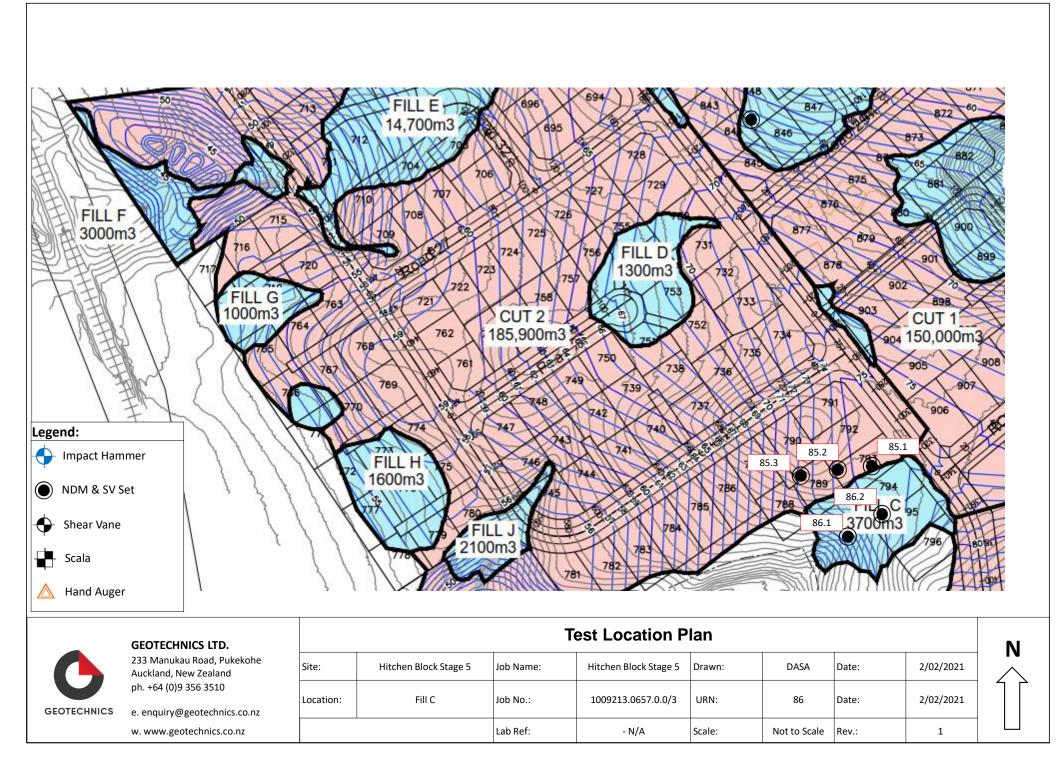


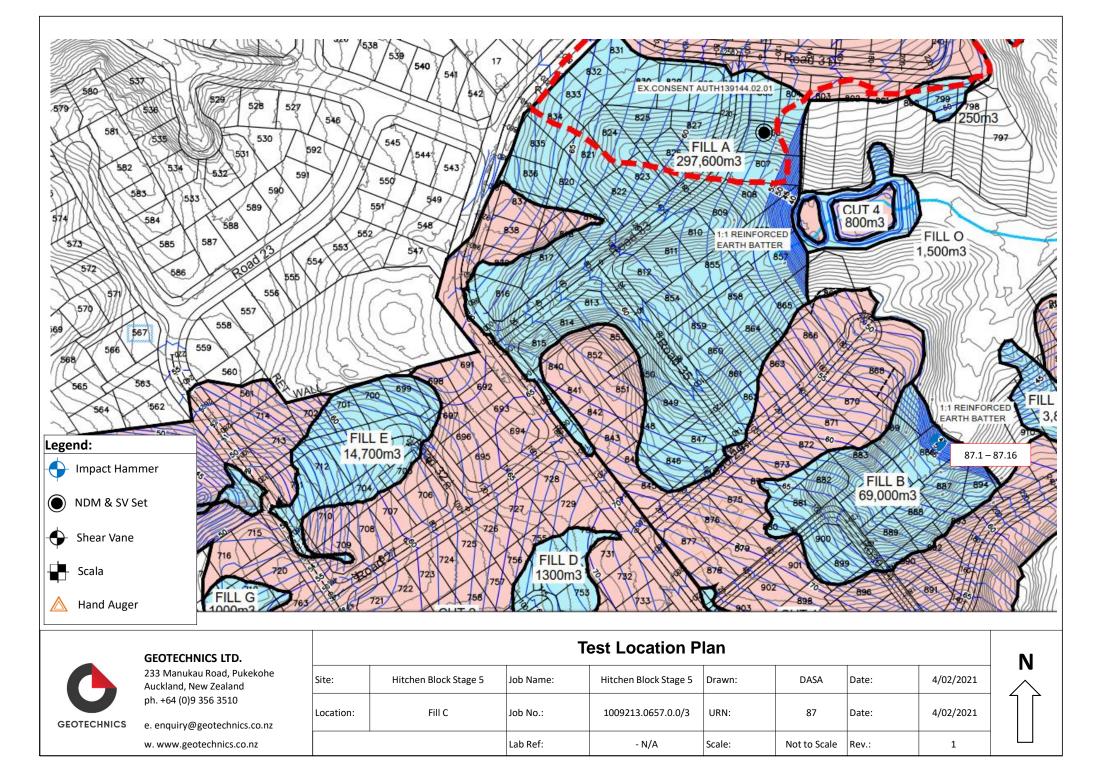


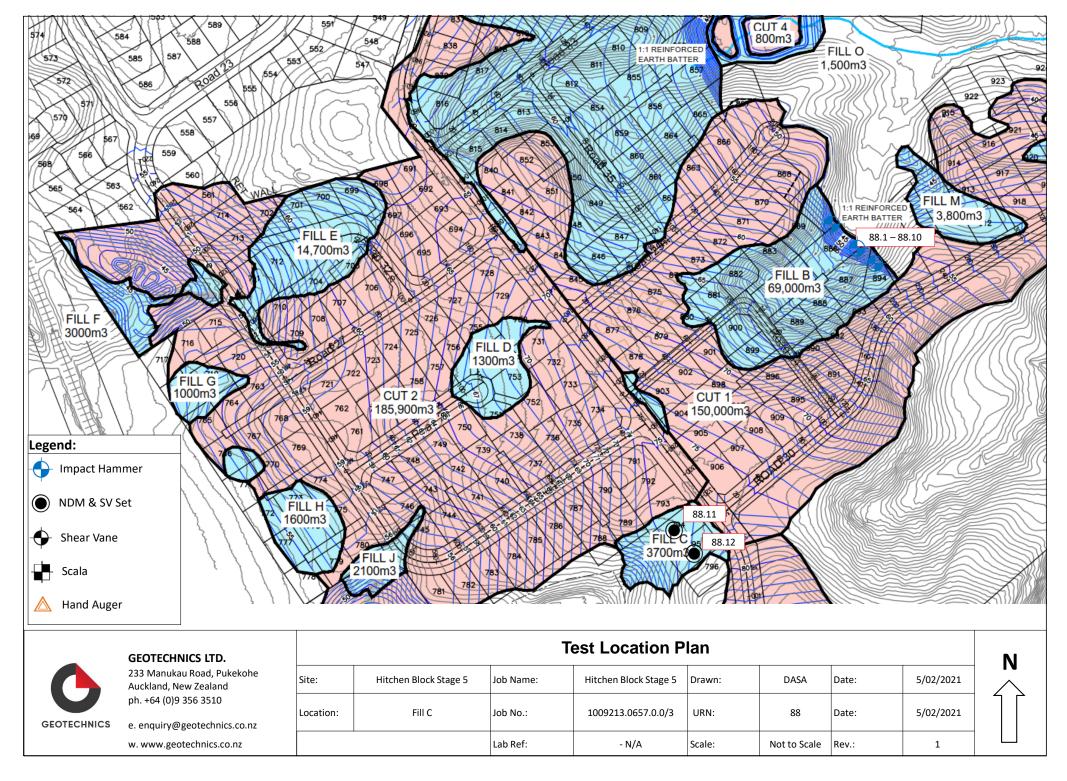


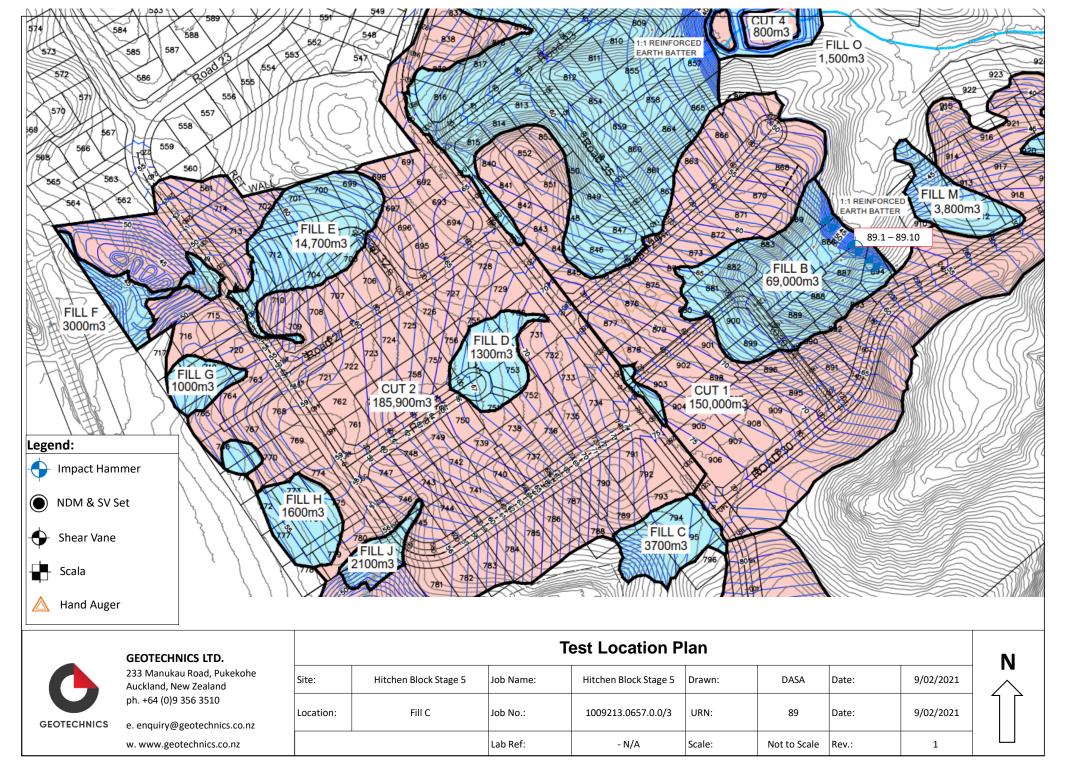


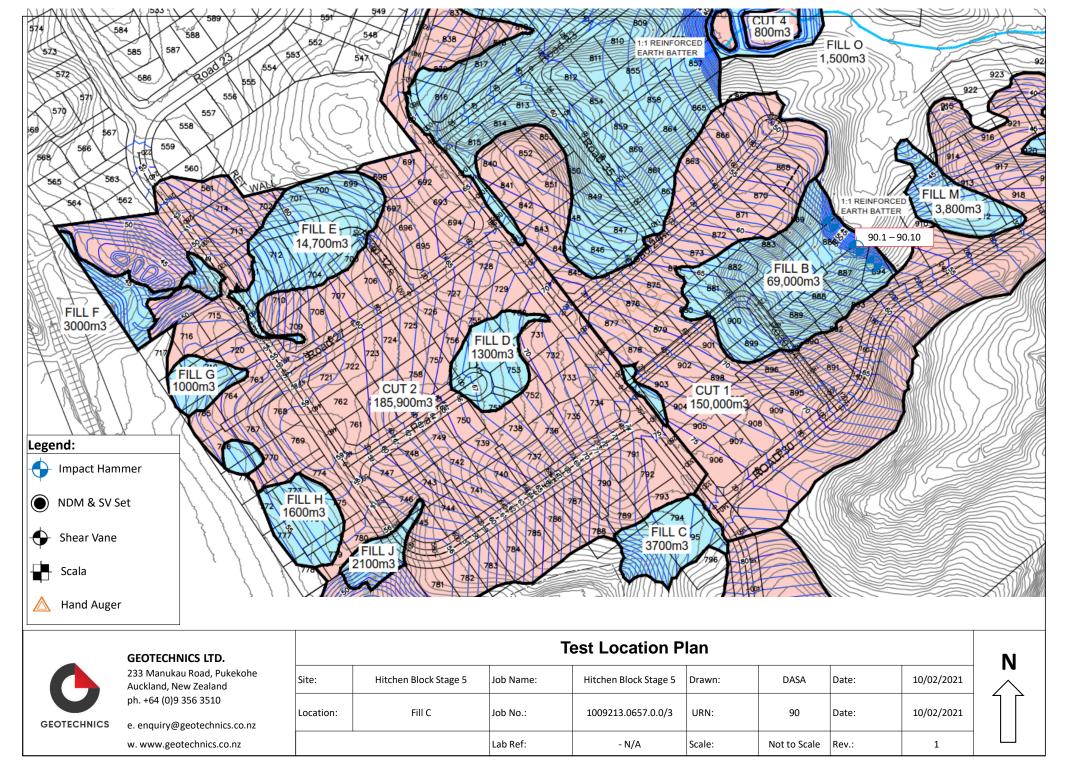


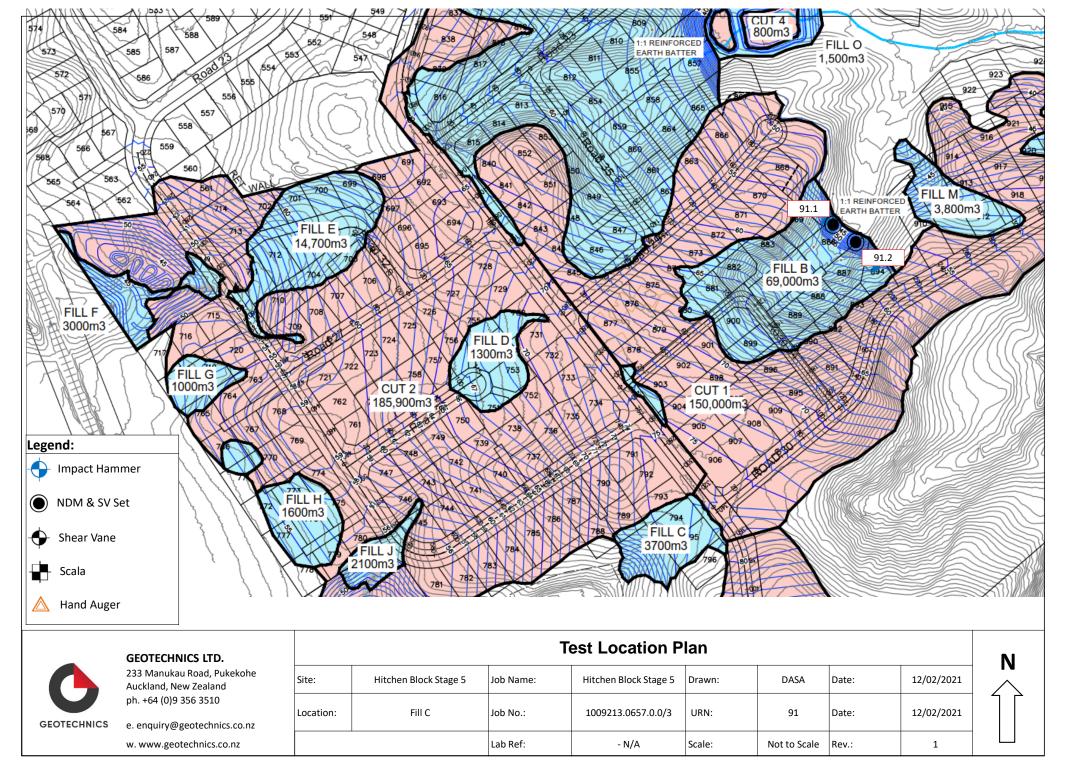


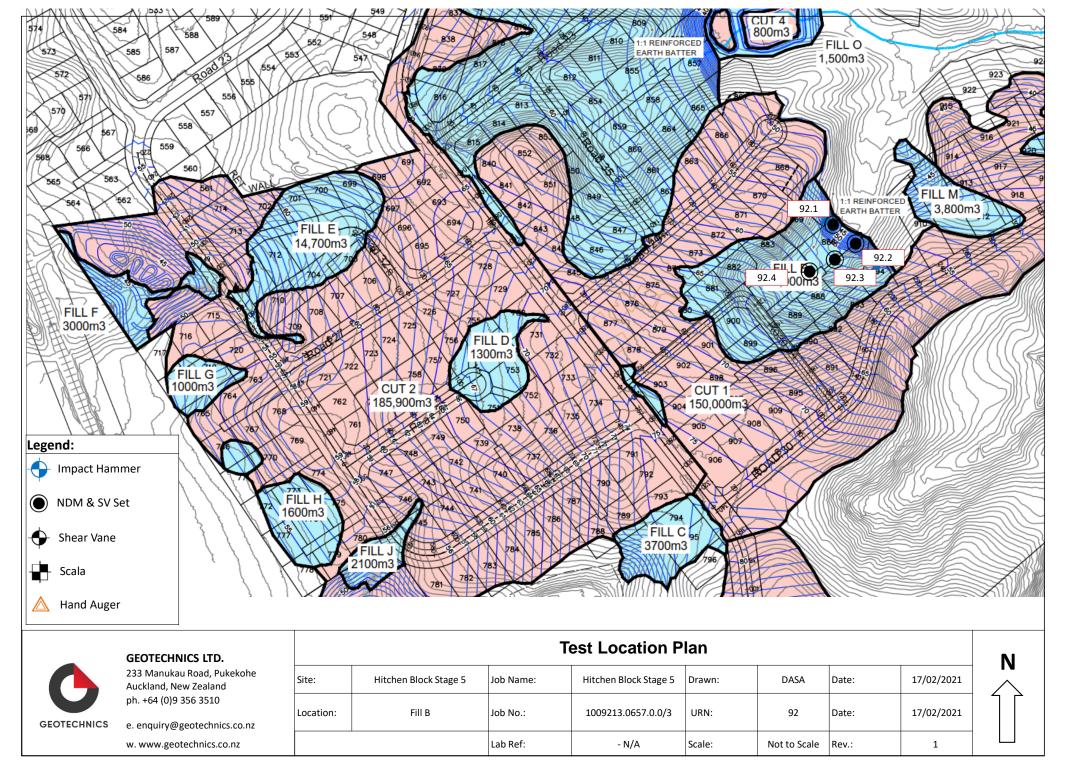


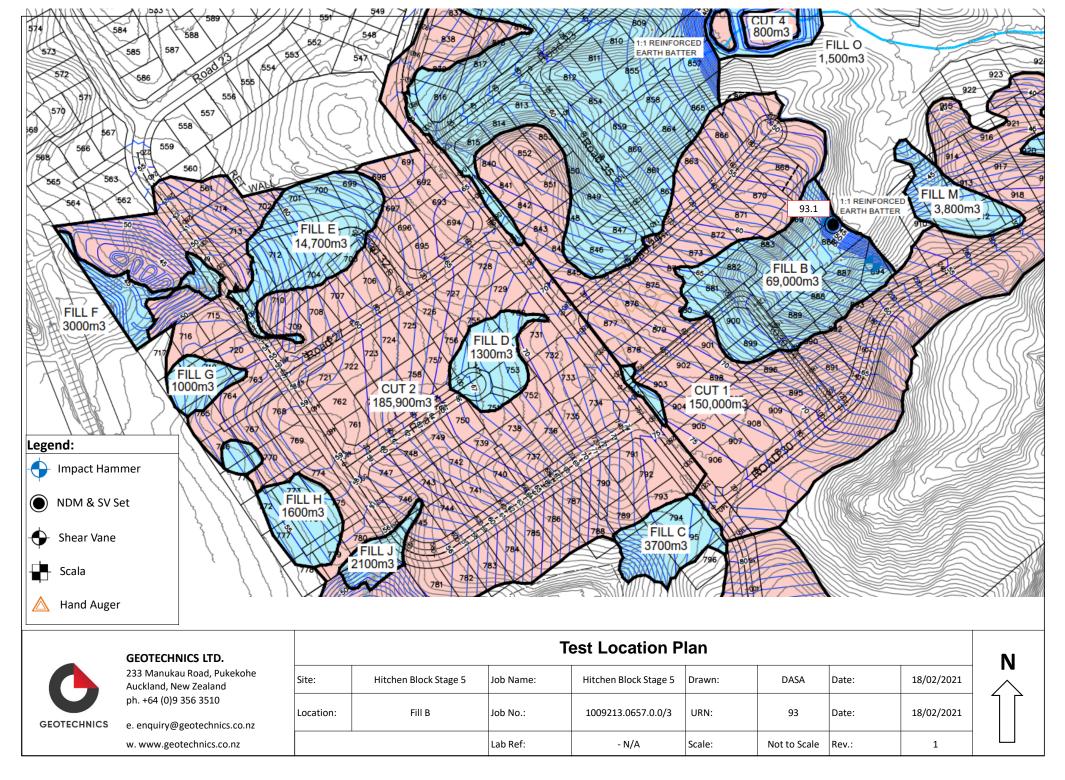


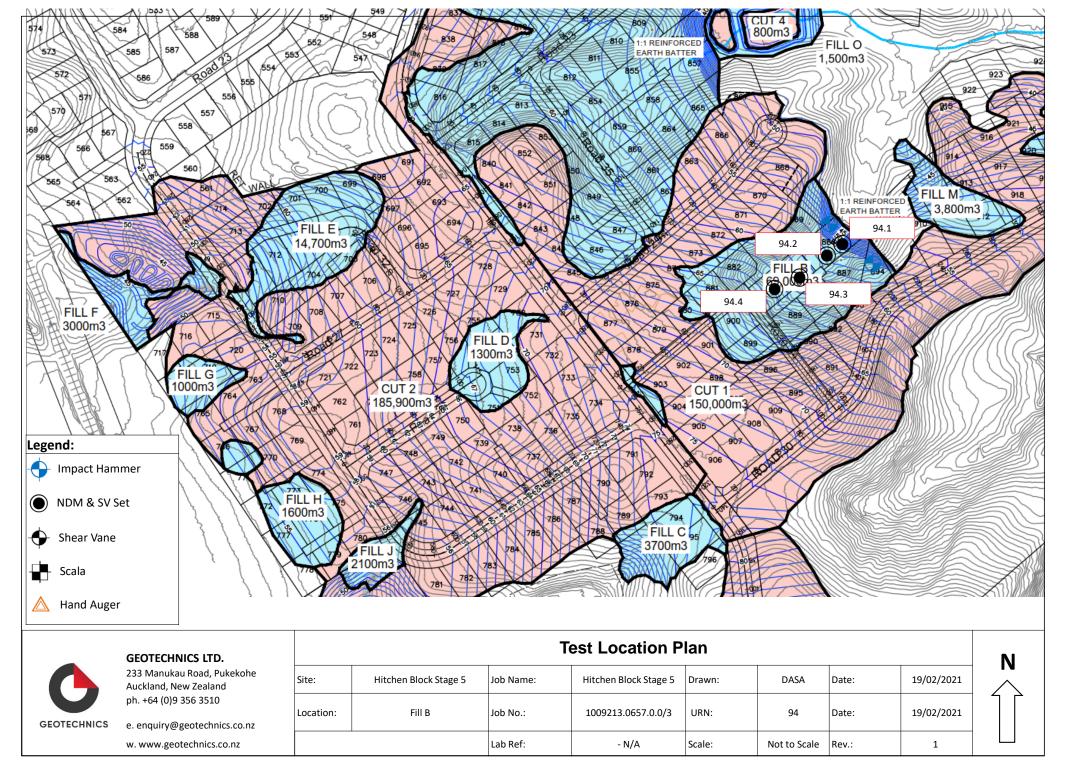


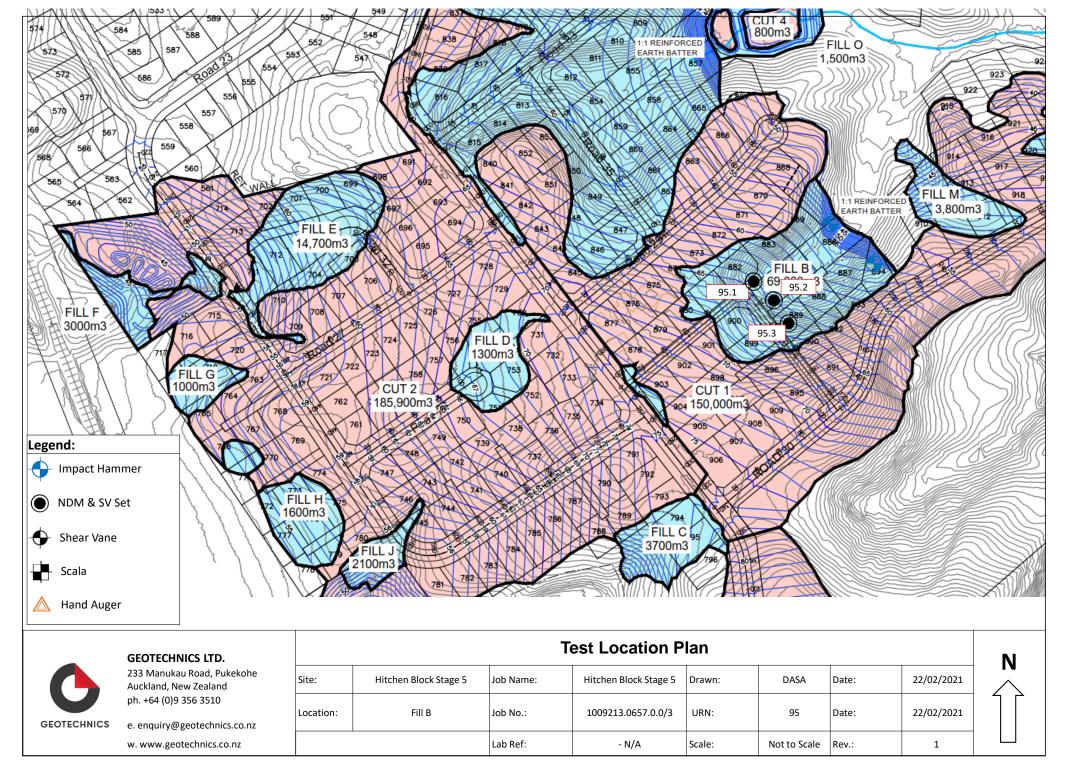


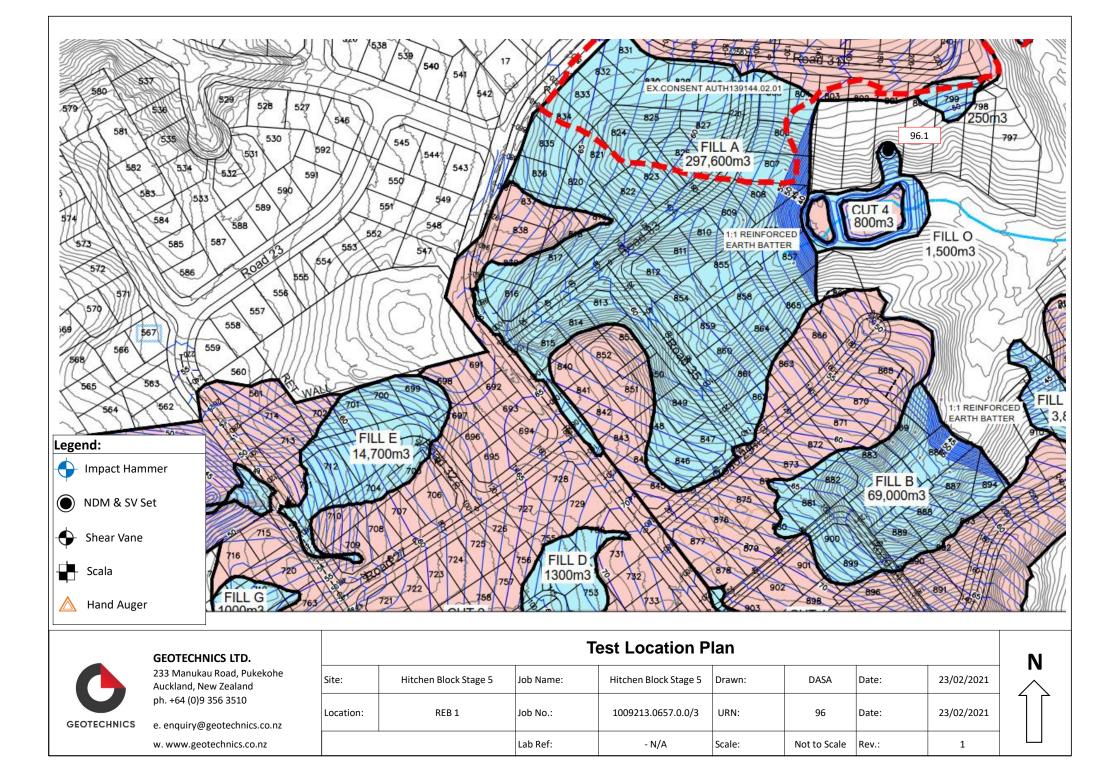


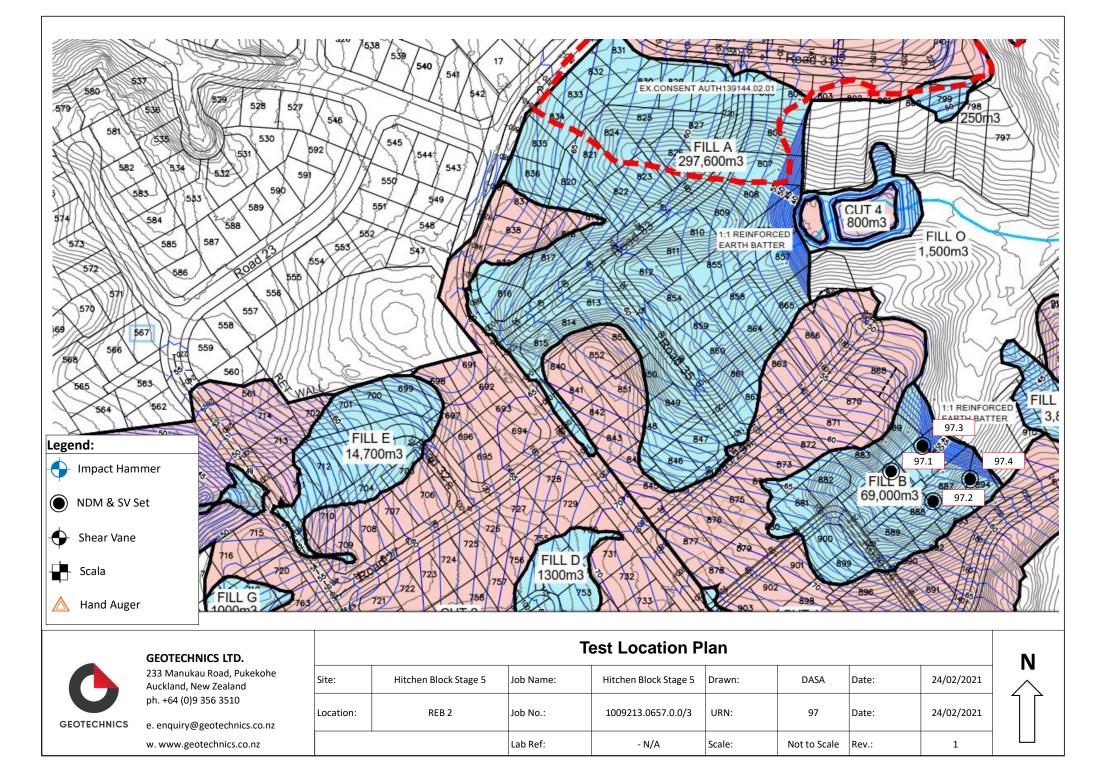


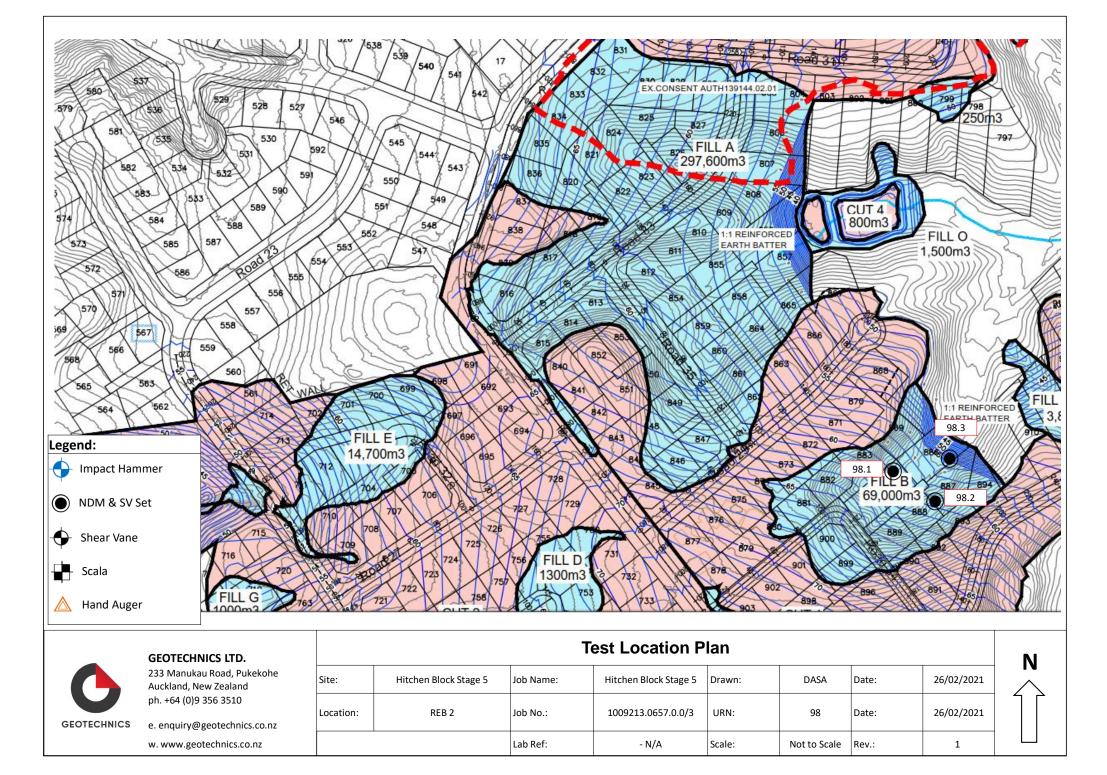


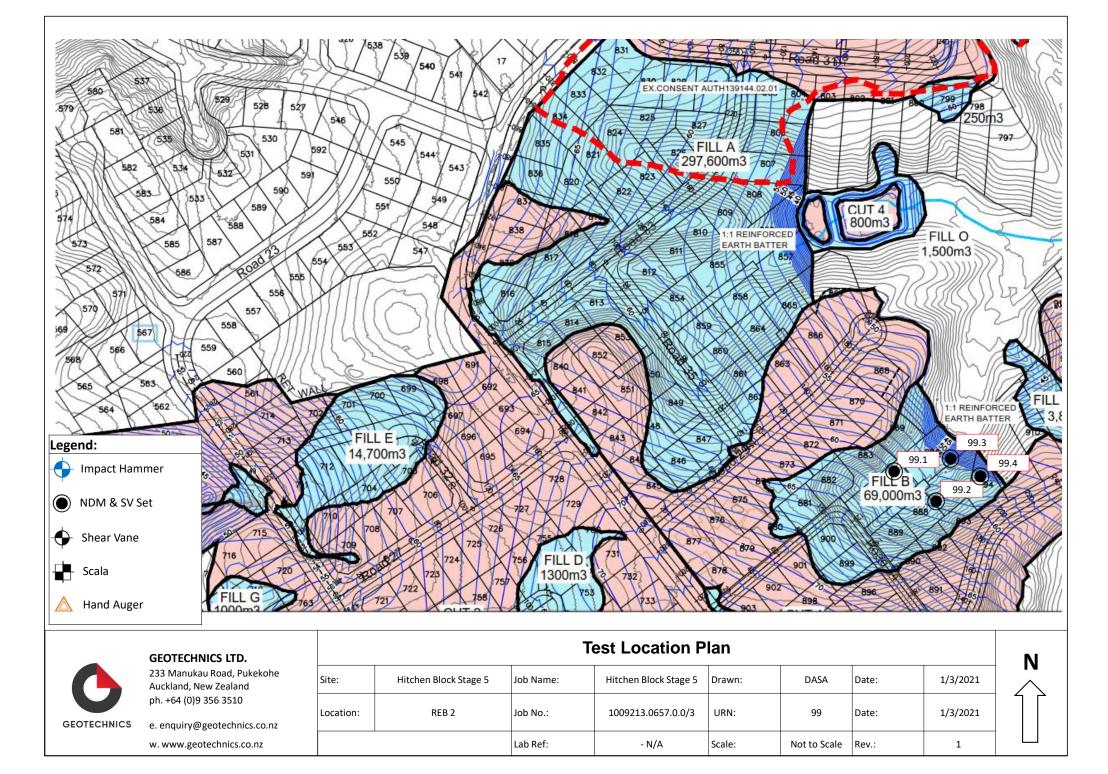


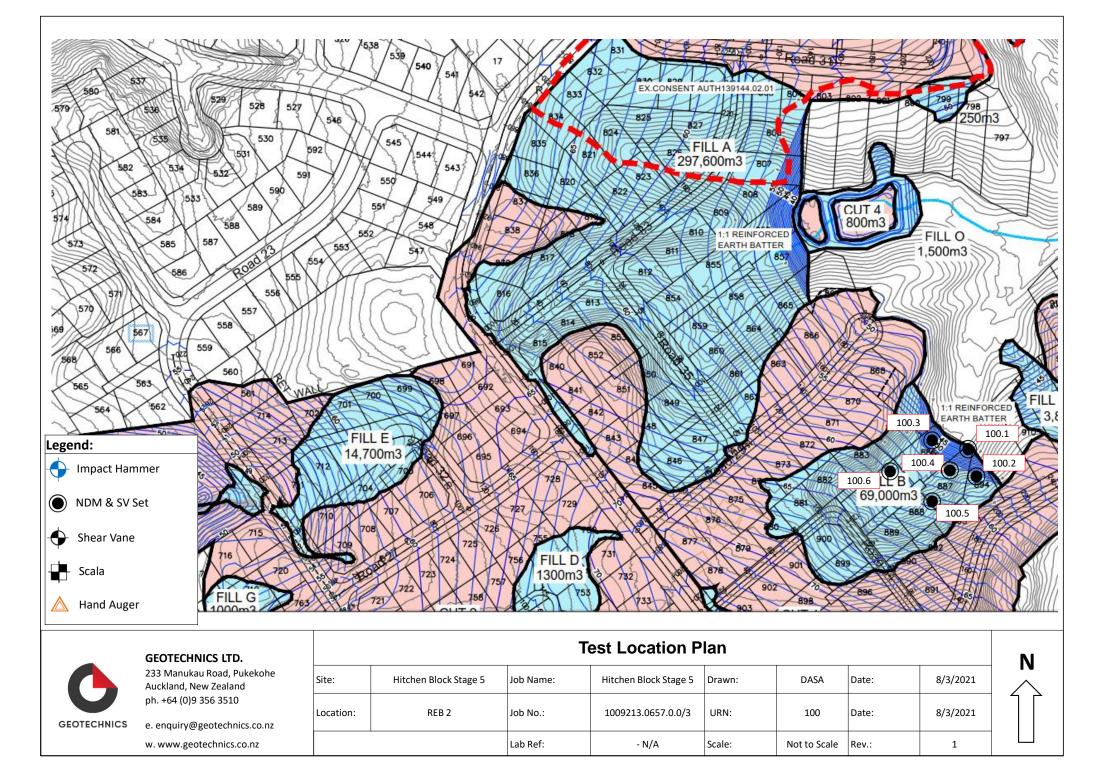


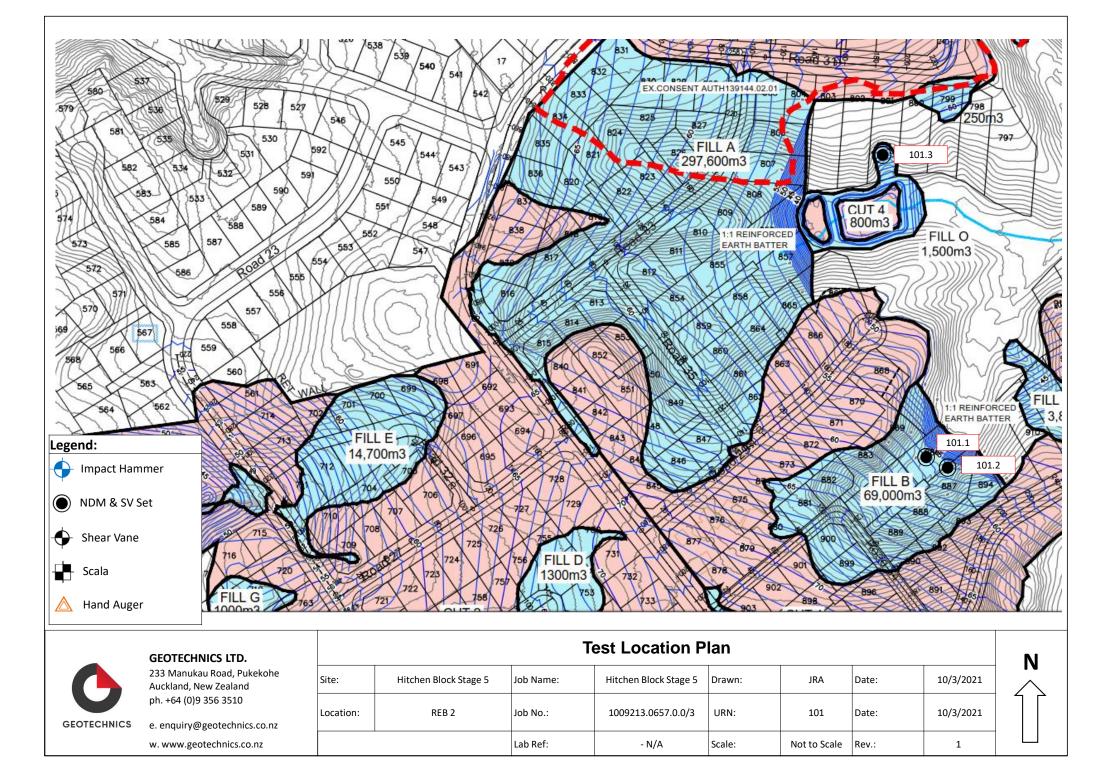


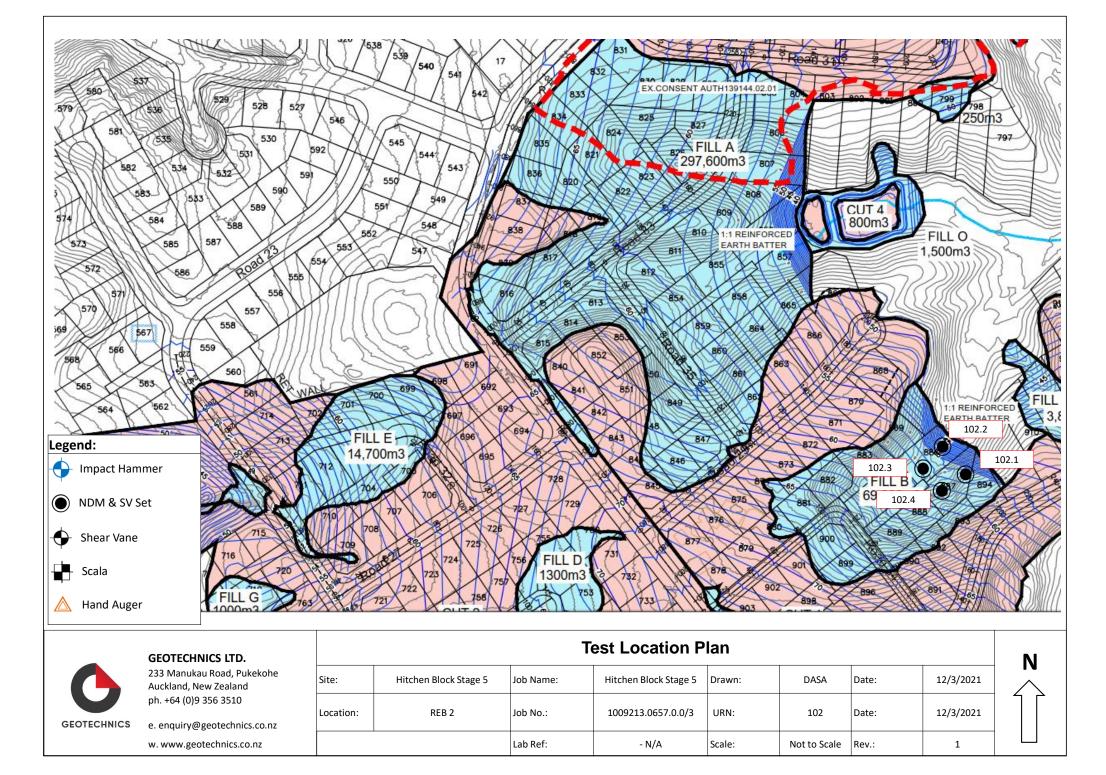


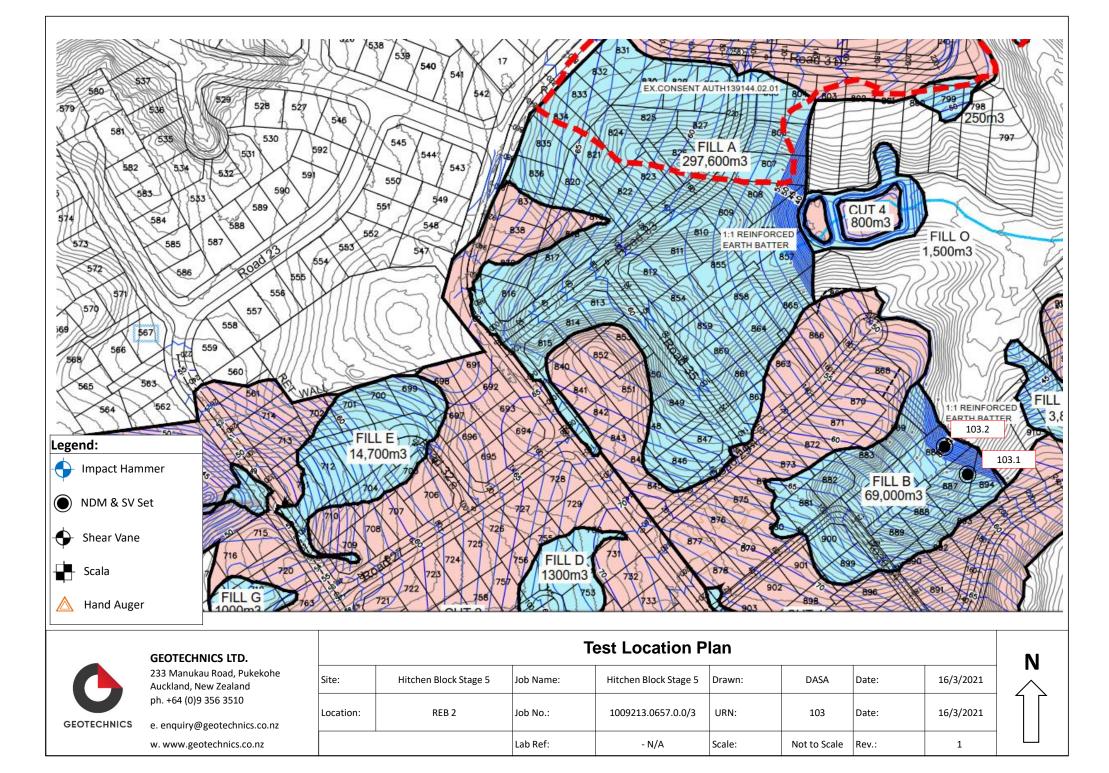


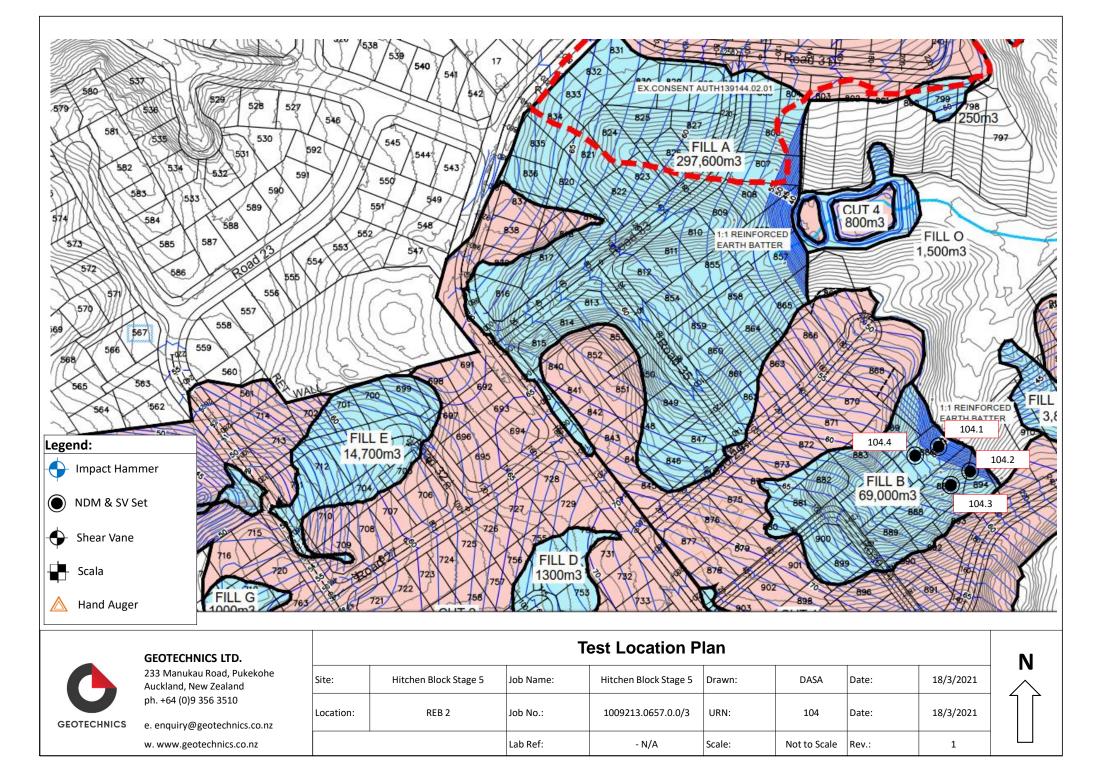


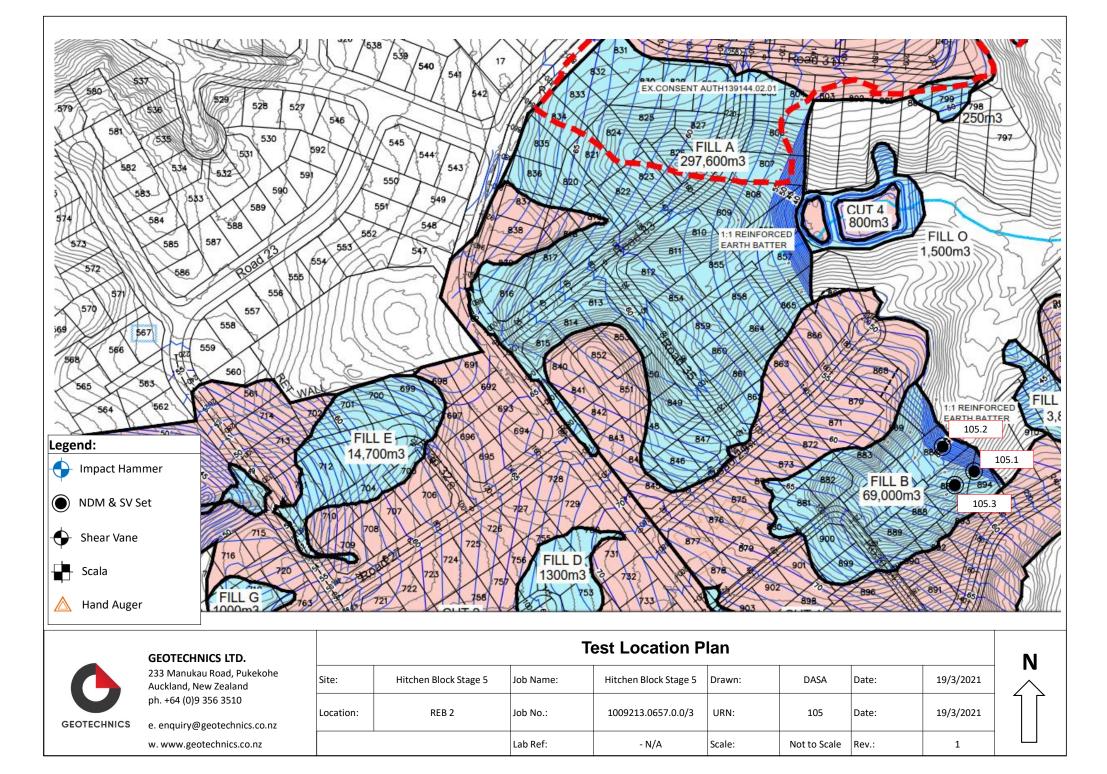


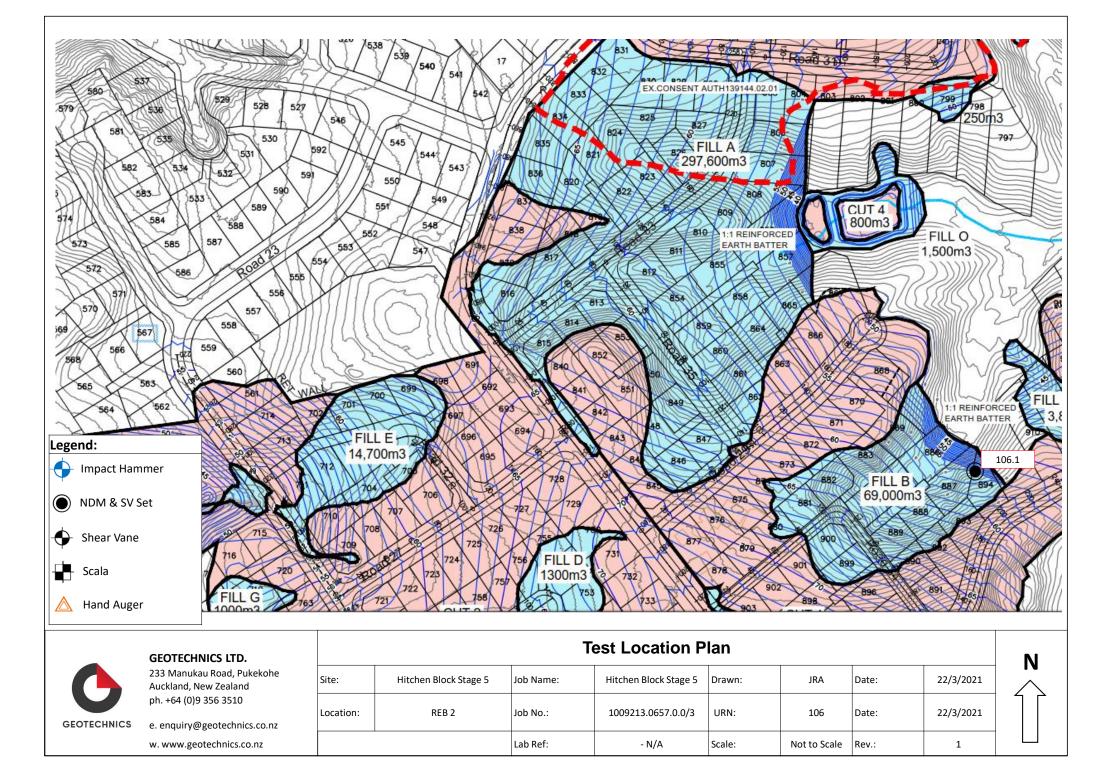


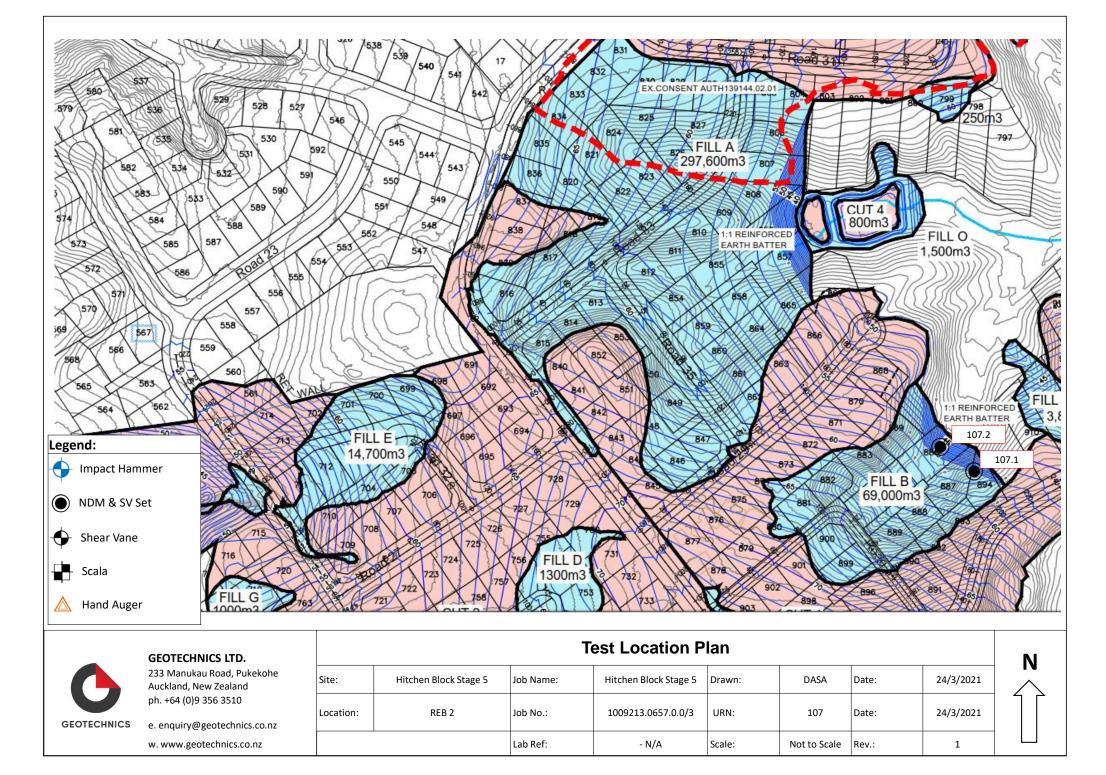


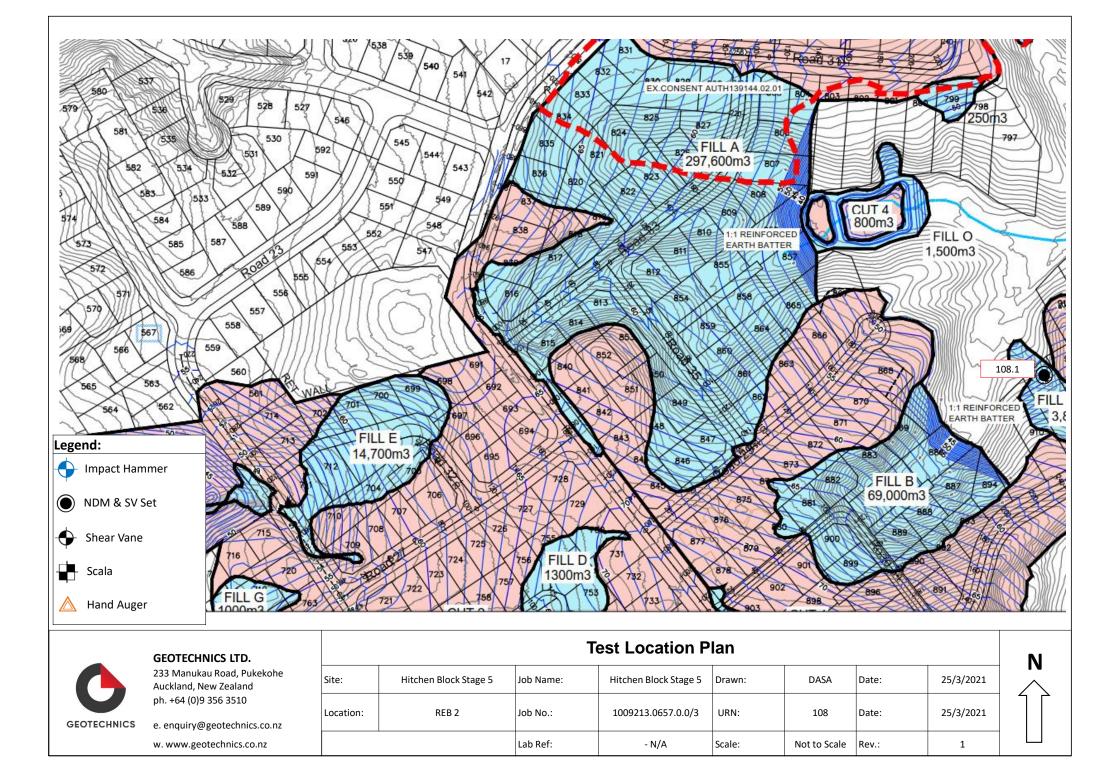


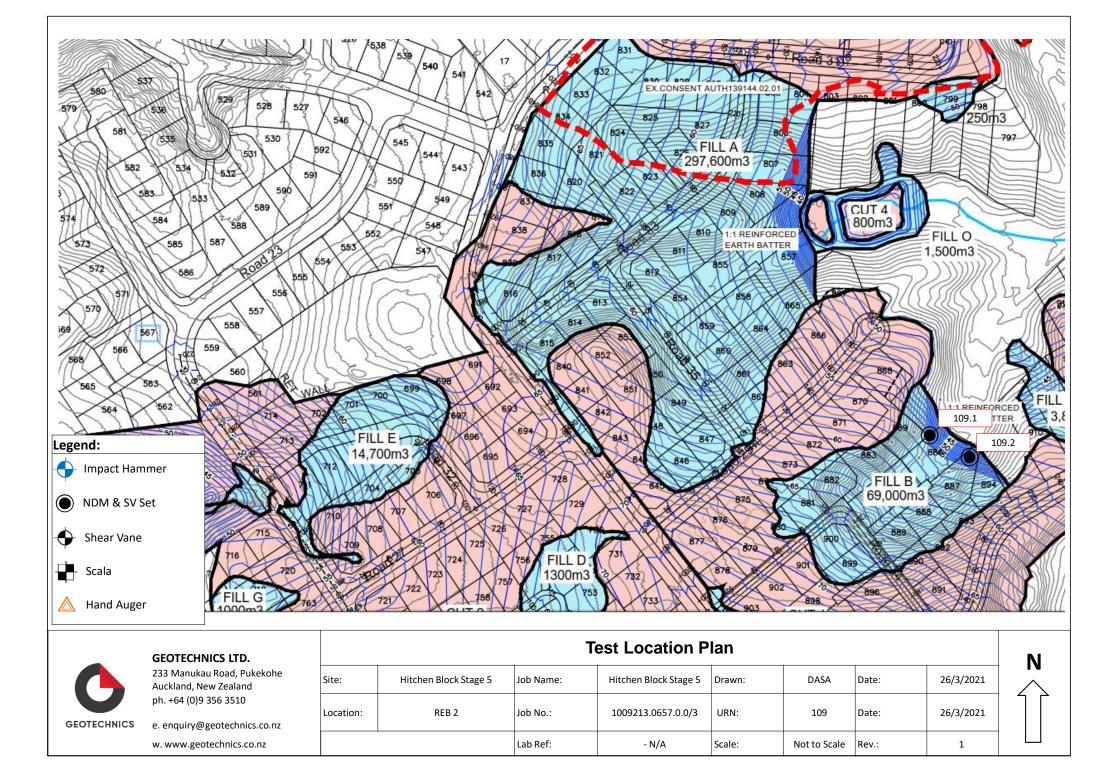


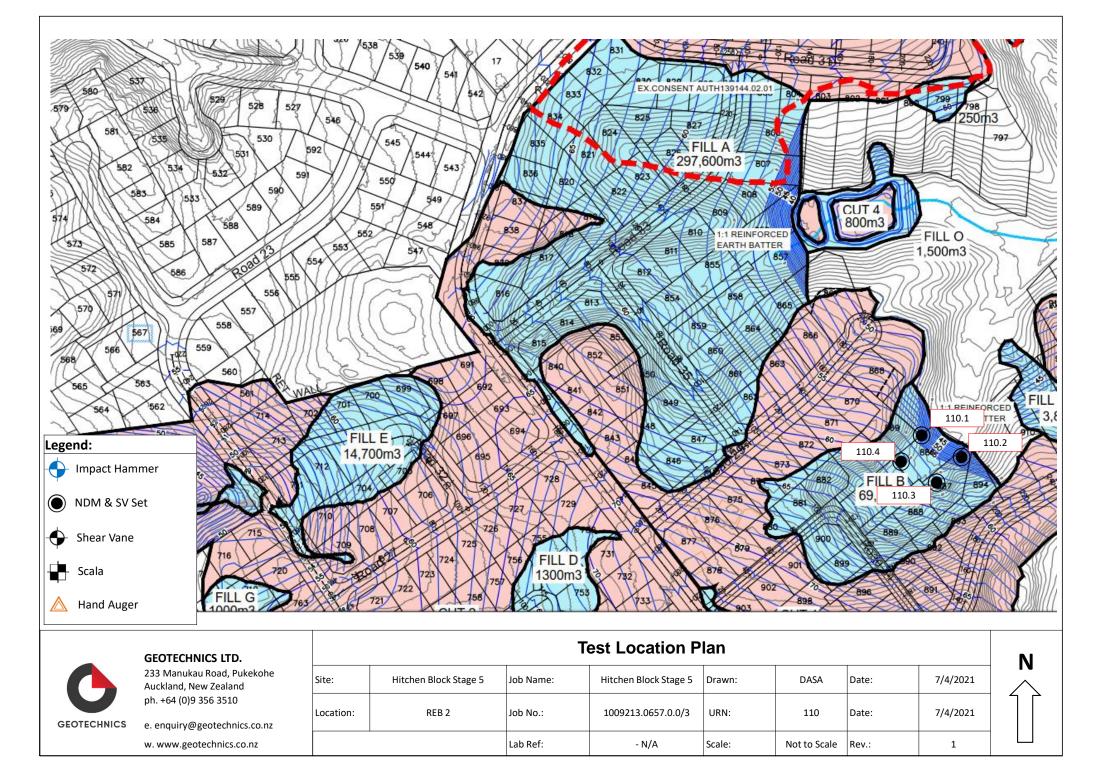


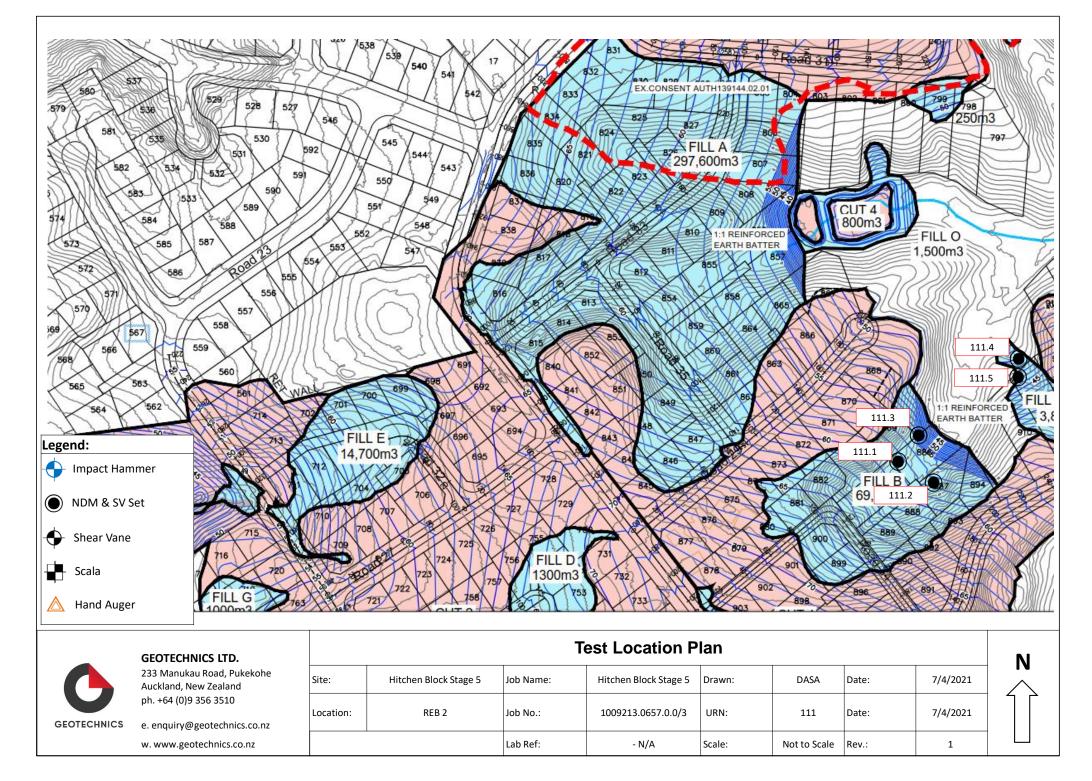


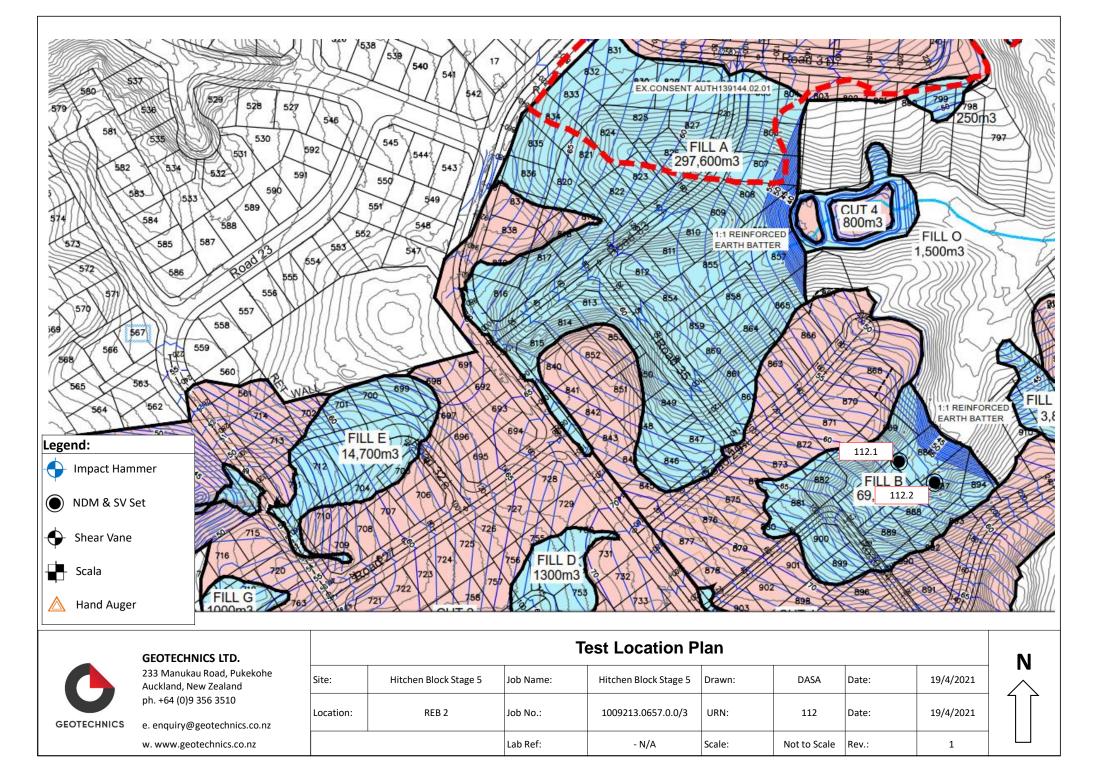


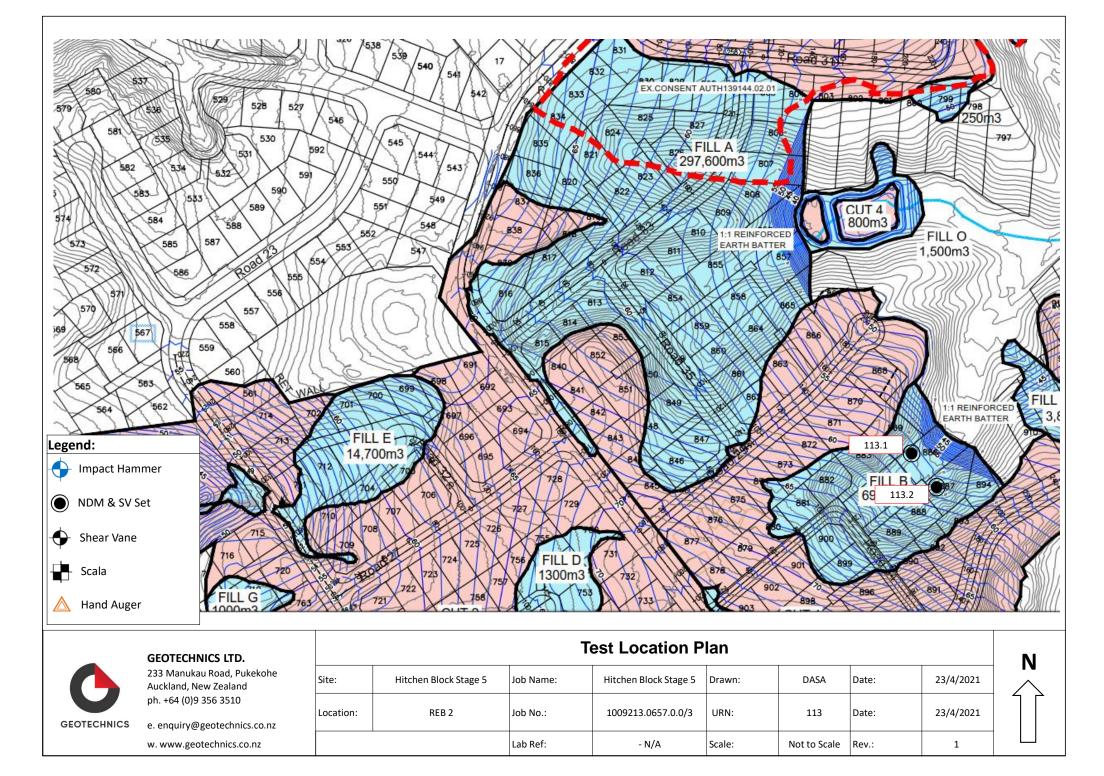


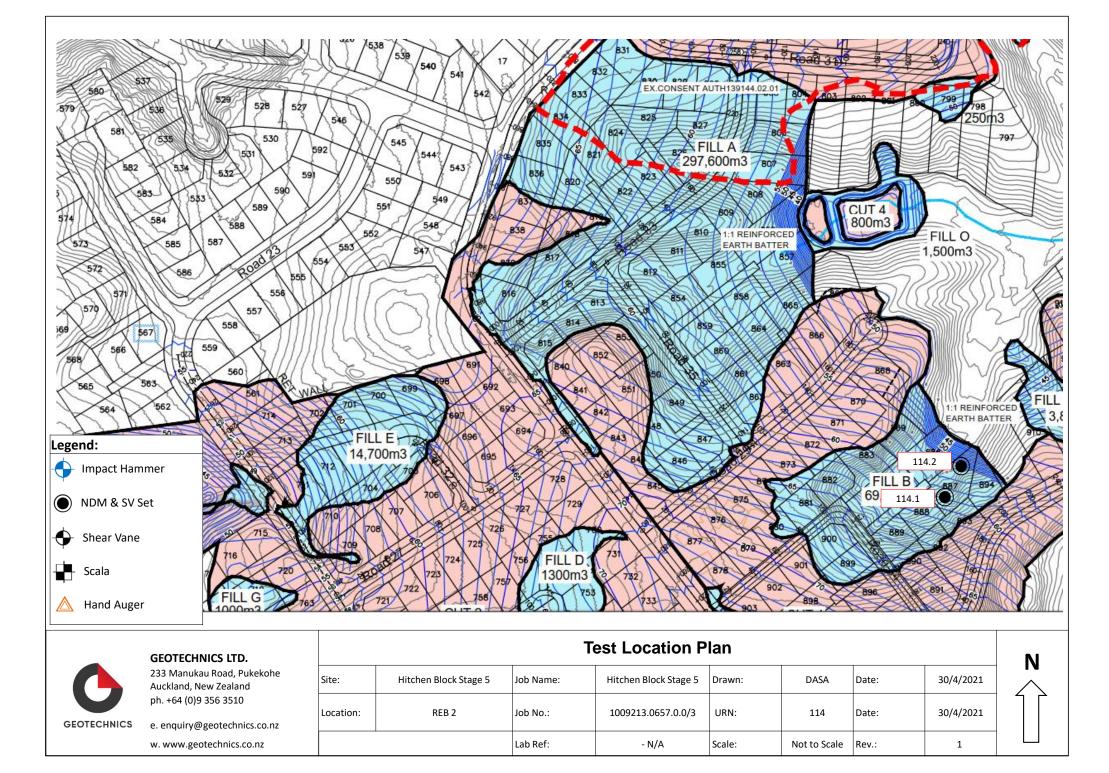


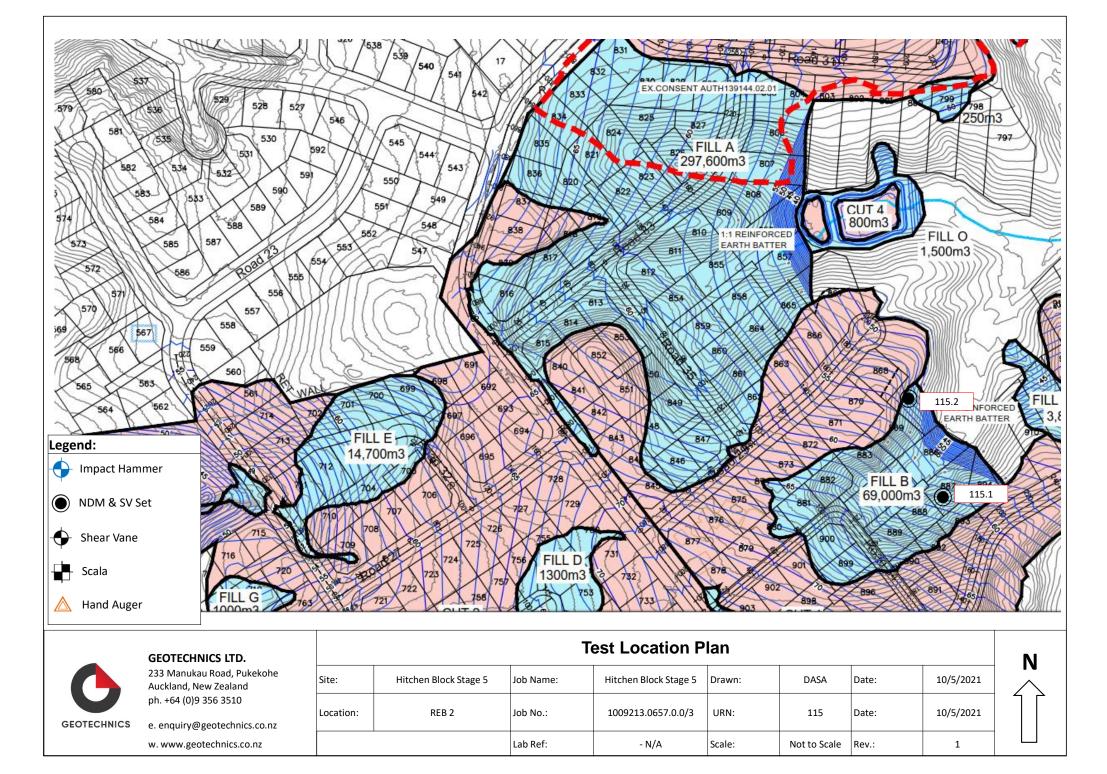














# Appendix 3

**Soil Classification Test Results** 



Our Ref: 1009479.1053.0.0/Rep1 Customer Ref: J00113 4 November 2021

Lander Geotechnical Ltd. PO Box 97385, Manukau 2241

Attention: Shane Lander

Dear Shane

## Hitchen Stage 13B, Pokeno

# Laboratory Test Report

The samples we collected from the above mentioned site have been tested according to your instructions and the results are included in this report. Results apply only to the sample(s) tested.

Descriptions are enclosed for your information, but are not covered under the IANZ endorsement of this report.

This report has been prepared for the benefit of Lander Geotechnical Ltd., with respect to the particular brief given to us and it cannot be relied upon in other contexts or for any other purpose without our prior review and agreement.

This report may be reproduced only in full.

Samples not destroyed during testing will be retained for one month from the date of this report before being discarded. If we can be of any further assistance, feel free to get in touch. Contact details are provided at the bottom of this page.

**GEOTECHNICS LTD** 

Report prepared by:

Jack Singh Laboratory Technician Approved Signatory Authorised for Geotechnics by:

Vic O'Connor Project Director



All tests reported herein have been performed in accordance with the laboratory's scope of accreditation

Page 2 of 10

Report checked by:

λ .**.**....

Corey Papu-Gread Christchurch Manager

4-Nov-21 t:\geotechnicsgroup\projects\1009479\1009479.1053\workingmaterial\2021027.1009479.1053.0.0.rep1.docx



Christchurch 45A Parkhouse Road Wigram Christchurch 8042 New Zealand

p +64 3 361 0300

GEOTECHNICS		p · 04 0 001 0000
Material Te	st Report	Report No: MAT:S21CH00069
Customer: Lander Ge Address: Level 3, 3 Manukau, Project: Hitchen S Project No.: 1009479 Customer Reference Report Authorised B	eotechnical Osterley Way 2104 tage 13B Pokeno 0.1053.0.0 <b>No.:</b> J00113	Approved By: Jack Singh (Laboratory Technician) Date of Issue: 3/11/2021 Please reproduce this report in full when transmitting to others or including in internal rep
Sample Details Location Geotechnics ID Sample Reference Sample Description Sample Depth Bottom Depth	Hitchen Stage 13B Pokeno S21CH000696 Lot 778 SILT with minor to some clay, trace sand and minor gravel, yellowish brown, mottled orange. Moist, very high plasticity. 0.5m 1.0m	
Test Results		
Description Moisture Content [NZS Moisture Content (%) Date Tested	Method 4402:1986 Test 2.1]	Result         Limits           34.4         2/11/2021
Comments		
This test result is IANZ acc	redited.	

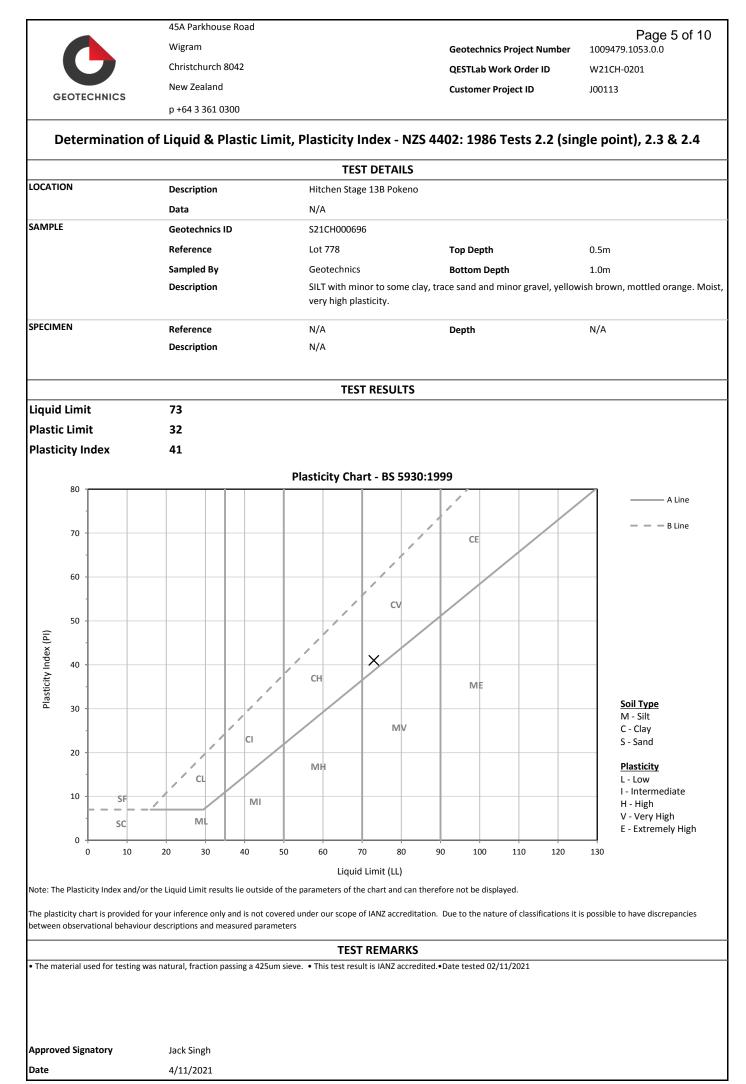
If samples have been taken, and were not destroyed during testing, they will be retained for one month from the date of this report before being discarded. Form No: 18909, Report No: MAT:S21CH000696 © 2000-2018 QESTLab by SpectraQEST.com Page

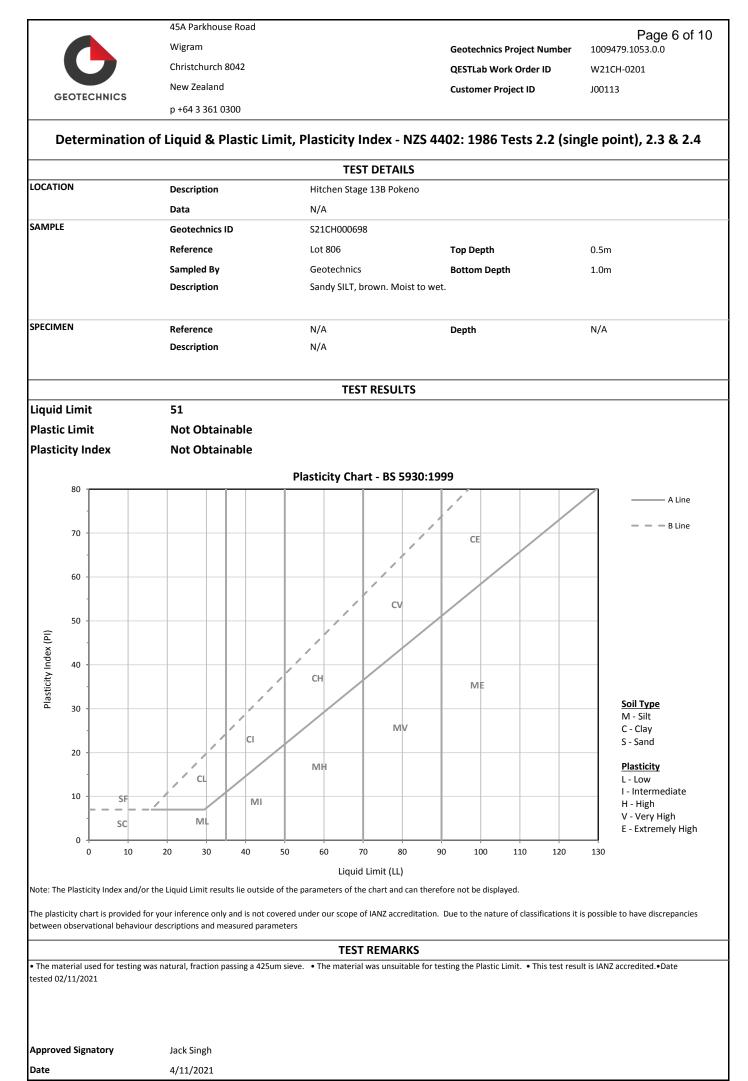


Christchurch 45A Parkhouse Road Wigram Christchurch 8042 New Zealand

GEOTECHNICS	p +64 3 361 0300
	Report No: MAT:S21CH000698
	Issue No: 1
Material Test Report	
Customer: Lander Geotechnical         Address:       Level 3, 3 Osterley Way         Manukau, 2104         Project:       Hitchen Stage 13B Pokeno         Project No.:       1009479.1053.0.0         Customer Reference No.:       J00113         Report Authorised By :       Jack Singh         Sample Details       Location         Location       Hitchen Stage 13B Pokeno         Geotechnics ID       S21CH000698         Sample Reference       Lot 806         Sample Description       Sandy SILT, brown. Moist to wet.         Sample Depth       0.5m         Bottom Depth       1.0m	Approved By: Jack Singh (Laboratory Technician) Date of Issue: 3/11/2021 Please reproduce this report in full when transmitting to others or including in internal reports.
Description       Method         Moisture Content [NZS 4402:1986 Test 2.1]       Moisture Content (%)         Date Tested       Date Tested	<b>Result Limits</b> 34.9 2/11/2021
Comments	
Comments This test result is IANZ accredited.	

If samples have been taken, and were not destroyed during testing, they will be retained for one month from the date of this report before being discarded. Form No: 18909, Report No: MAT:S21CH000698 © 2000-2018 QESTLab by SpectraQEST.com





	45A Parkhouse Road			Page 7 of 10
$\mathbf{C}$	Wigram	Geotechnics Project	t Number	1009479.1053.0.0
	Christchurch 8042	QESTLab Work Orde	er ID	W21CH-0201
GEOTECHNICS	New Zealand	Customer Project IE	)	J00113
	p +64 3 361 0300			
	Determination	of the Linear Shrinkage - NZS 4402:1986 Test	t <b>2.6</b>	
0017101		TEST DETAILS		
OCATION	Description	Hitchen Stage 13B Pokeno		
	Data	N/A		
SAMPLE	Geotechnics ID	S21CH000696		
	Reference	Lot 778 Top Depth		0.5m
	Sampled By	Geotechnics Bottom Depth		1.0m
	Description	SILT with minor to some clay, trace sand and minor gravel, very high plasticity.	yellowish bro	wn, mottled orange. Moist,
PECIMEN	Reference	Depth		
	Description			
Linear Shrinkage	16%			
		TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		
• This test result is IANZ accredite	ed.•Date tested 02/11/2021	TEST REMARKS		

	45A Parkhouse Road			Page 8 of 10
	Wigram		Geotechnics Project Number	1009479.1053.0.0
	Christchurch 8042		QESTLab Work Order ID	W21CH-0201
GEOTECHNICS	New Zealand		Customer Project ID	J00113
	p +64 3 361 0300			
	Determination	of the Linear Shrinkage - NZS	4402:1986 Test 2.6	
OCATION	Description	TEST DETAILS		
	Description	Hitchen Stage 13B Pokeno		
SAMPLE	Data Geotechnics ID	N/A S21CH000698		
	Reference	Lot 806	Top Depth	0.5m
	Sampled By	Geotechnics	Bottom Depth	1.0m
	Description	Sandy SILT, brown. Moist to wet.	Bottom Depth	1.011
SPECIMEN	Reference Description		Depth	
	<b>P</b>			
Linear Shrinkage	6%			
		TEST REMARKS		
• This test result is IANZ accredited	d.●Date tested 02/11/2021			
Approved Signatory	Jack Singh			
Approved Signatory Date	Jack Singh 3/11/2021			

	45A Parkhouse Road			Page 9 of 10		
GEOTECHNICS	Wigram Christchurch 8042 New Zealand p +64 3 361 0300		Geotechnics Project Number QESTLab Work Order ID Customer Project ID	1009479.1053.0.0 W21CH-0201 J00113		
	Determination of the S	hrink - Sv	vell Index - AS 1289 Test 7.1.1 - 2003	}		
		TES	T DETAILS			
LOCATION	Description	Hitchen S	tage 13B Pokeno			
	Data	N/A				
SAMPLE	Geotechnics ID	S21CH000	695 <b>BH No</b>	Lot 766		
	Reference	Lot 766	Top Depth	0.5m		
	Sampled By	Geotechn	ics Bottom Depth	1.0m		
	Description	SILT with	minor sand and trace clay, orangish brown. Moist			
SPECIMEN	Reference		Depth			
	Description					
		TEST	RESULTS			
	Applied Pressure	(kPa)	25			
	Initial Water Content	(%)	17.6			
	Bulk Density	(t/m³)	1.83			
SWELL TEST	Dry Density	(t/m³)	1.56			
	Final Water Content	(%)	18.7			
	Swelling Strain	(%)	-0.04			
	Initial Water Content	(%)	18.1			
	Shrinkage Strain	(%)	4.9			
SHRINKAGE TEST	Inert Material Estimate in the Soil Specimen	(%)	None			
	Soil Crumbling During Shrinkage		Minor			
	Cracking of the Shrinkage Specimen		Minor			
	SHRINK - SWELL INDEX	(%)	2.7			
		TES	REMARKS			

• Estimates of inert material, soil cracking and soil crumbling are enclosed for your information, but are not covered under the IANZ endorsement of this report. • This test result is IANZ accredited. • Date tested 27/10/2021

					Page 10 of 10
	45A Parkhouse Road				Ū
	Wigram		Geotechnics Project	t Number	1009479.1053.0.0
	Christchurch 8042	QESTLab Work Ord	er ID	W21CH-0201	
GEOTECHNICS	New Zealand		Customer Project IE	)	J00113
GEOTECHNICS	p +64 3 361 0300				
	Determination of the S	Shrink - Sv	vell Index - AS 1289 Test 7.1	.1 - 2003	
		TES	DETAILS		
LOCATION	Description	Hitchen S	age 13B Pokeno		
	Data	N/A			
SAMPLE	Geotechnics ID	S21CH000	697 BH No		Lot 788
	Reference	Lot 788	Top Depth		0.5m
	Sampled By	Geotechn			1.0m
	Description	SILT with	trace sand and trace clay, brownish re	d. Moist.	
SPECIMEN	Reference				
	Description				
		TEST	RESULTS		
	Applied Pressure	(kPa)		25	
	Initial Water Content	(%)		31.6	
	Bulk Density	(t/m³)		2.00	
SWELL TEST	Dry Density	(t/m³)	1.52		
	Final Water Content	(%)	32.3		
	Swelling Strain	(%)	0.16		
	Initial Water Content	(%)		31.8	
	Shrinkage Strain	(%)		7.1	
SHRINKAGE TEST	Inert Material Estimate in the Soil Specimen	(%)		None	

• Estimates of inert material, soil cracking and soil crumbling are enclosed for your information, but are not covered under the IANZ endorsement of this report. • This test result is IANZ accredited.• Date tested 27/10/2021

TEST REMARKS

(%)

Minor

Minor

4.0

Soil Crumbling During Shrinkage

Cracking of the Shrinkage Specimen

SHRINK - SWELL INDEX

### 3.2 Slab-on-ground in expansive soils

# 3.2.1 NZS 3604 Clause 1.1.2 Buildings covered by this Standard

Amend 1.1.2(a) to read:

"Buildings founded on good ground or on expansive soils where the requirements of 1.1.5 are met"

### **3.2.2 NZS 3604 New Clause** Add new: "Clause 1.1.5 Buildings on expansive soils

Buildings on expansive soils shall be supported on slab-on-ground foundations complying with 7.5.13 and in addition to 1.1.2 shall be limited as follows:

- (a) single storey, stand-alone household unit, and
- (b)maximum length or width of floor of 24.0 m including any attached garage, and
- (c) simple plan shapes such as rectangular, L, T or boomerang, and
- (d) concrete slab-on-ground with a minimum thickness of 100 mm and a minimum concrete compressive strength of 20 MPa, and
- (e)simple roof forms, incorporating hips, valleys, gables or mono pitches, and
- (f) maximum overall height of 7.0 m to roof apex from lowest cleared ground level, and
- (g)maximum roof height of 3.0 m, and
- (h)roof slope between 10° and 35° from the horizontal, and
- (i) maximum span of roof truss 12.0 m, and
- (j) external walls maximum of 2.4 m height studs, other than gable end walls and walls to mono-pitched roofs, which shall not exceed 4.0 m.

## COMMENT:

#### Floor plans

Where floor plans incorporate re-entrant corners then continuity of the exterior ground beam shall be maintained by continuing it as an internal beam, with the exterior beam details continued for a length of at least 1.0 m into the internal beam. This is only applicable where internal beams are specified in Tables 7.4A and 7.4B. This is aimed to bring the solution in NZS 3604 in line with Clause 5.3.8 of AS 2870:2011.

#### Ground movement

Provision for the additional ground movement effects from trees near to foundations in expansive soils should be considered. Trees remove moisture from the soil for a radius equal to the height of the tree. This causes expansive soils to shrink to varying degrees, and when near houses leads to differential settlement occurring under foundations. Movement of the foundations may lead to cracks in the building and door jamming.

Where existing trees (including trees that have been recently removed) are located closer to the foundations than 1.5 times the mature height of a tree, then additional geotechnical advice should be obtained. Planting of new trees should be avoided near foundations of new buildings or neighbouring buildings on sites with expansive soils.

### 3.2.3 NZS 3604 Clause 7.5.1

Add the following paragraph at the end of Clause 7.5.1:

"Slabs on expansive soils for buildings meeting the requirements of 1.1.5 shall, in addition to meeting the requirements of 7.5.1 to 7.5.12, meet the requirements of 7.5.13. Where there is conflict the requirements of 7.5.13 shall apply."

# 3.2.4 NZS 3604 New clause, tables and figures

Add new: Clause 7.5.13 Slab-on-ground in expansive soils

**7.5.13.1 Identification of expansive soils** 7.5.13.1.1 Should reasonable enquiry as outlined in 3.1.3 show any signs of expansive soils, the expansive soil class, as defined in AS 2870, shall be established by one or all of:

(a) enquiry to the local territorial authority, and/or

- (b) reference to the certificate of suitability issued in terms of NZS 4431, and/or
- (c) a soil test undertaken by a suitably qualified soils engineer.

# 7.5.13.1.2 Expansive soil class shall be defined as:

- (a) Slightly 'S', having an I<sub>SS</sub> range of 0–1.9%, and a 500 year design characteristic surface movement return (y<sub>S</sub>) of 22 mm, or
- (b)Moderately 'M', having an I<sub>SS</sub> range of 2.0–3.7% and a 500 year design characteristic surface movement return (y<sub>S</sub>) of 44 mm, or

Amend 19 Nov 2019

Amend 19

Nov 2019

- (c) Highly 'H', having an I<sub>SS</sub> range of 3.8–6.5% and a 500 year design characteristic surface movement return (y<sub>S</sub>) of 78 mm, or
- (d) Extremely 'E', having an I<sub>SS</sub> range of 6.6–7.5% and a 500 year design characteristic surface movement return (y<sub>S</sub>) of 90 mm.

# 7.5.13.2 Maximum aspect ratio of concrete slabs

The aspect ratio of the concrete slabs or bays of concrete slabs, such as in the case of L, T or boomerang concrete slab shapes, shall not exceed 5 to 1 (length to width).

#### 7.5.13.3 Foundation details

7.5.13.3.1 For the identified expansive soil class the foundation details, external and internal thickenings shall be as follows.

- (a) For light wall claddings refer to Table 7.4A and Figure 7.22.
- (b)For medium wall or heavy wall claddings refer to Table 7.4B and Figure 7.23.

7.5.13.3.2 Situations where no internal thickenings shall be required are limited to a rectangular slab with long side not exceeding 17.0 m. Where this limit is exceeded, add additional internal thickenings across the slab with the same cross section dimensions and reinforcing as the external footing, so that the centre to centre spacing of thickenings is always less than 17.0 m.

#### COMMENT:

#### Design constraints:

- a) The characteristic surface movements and the corresponding expansivity classifications have been calculated based on design for ultimate limit state (ULS) conditions for a 1 in 1000 year "extreme" drought event, and the serviceability limit state (SLS) conditions for a 1 in 500 year drought event.
- b) Maximum soil movements are calculated to be based on a 500 year return period for SLS, and a 1000 year return period for ULS\*;

(\*NB: This differed from the recommendations contained within BRANZ Study Report 120A (BSR120A) which used a 300 year return period for the design level drought conditions)

Amend 19 Nov 2019

- c) Climate parameters adopted from BSR120A of  $\Delta u = 1.2$  pF, Hs = 1.5 m, and a crack depth of 0.5 Hs
- d) The I<sub>SS</sub> (soil stability index) ranges attributed to the expansivity classifications as defined in 3.2.4 above have been calculated using the parameters presented in BSR120A and Equation 2.3.1 of AS 2870:2011.
- e) Sites subject to parameters that differ from those mentioned above, in particular sites where the crack depth is less than 0.75 m, such as cut natural ground or clay backfill, require specific engineering assessment to confirm their appropriate site classification.
- f) The effects of nearby trees (whether existing, recently removed, or future planting) are not considered in these solutions. It is recommended that specific geotechnical engineering advice is obtained where a tree is within a lateral distance of 1.5 times its mature height of the foundations.

#### Maintenance of foundations in expansive soils

Normal maintenance is that work generally recognised as necessary to achieve the expected performance over time of the foundation located on expansive soils. Unless otherwise specified by the designer and noted on the drawings, basic normal maintenance tasks should ensure that:

- a) the drainage and wetting of the site is controlled so that extremes of wetting and drying of the soils are prevented, and
- b) the position and operation of gardens adjacent to the dwelling are controlled, and the planting of trees near to foundations is suitably restricted, and
- c) any leaks which develop in plumbing, storm water or sanitary sewage systems are repaired promptly.

Amend 19 Nov 2019

Table 7.4A         Reinforced concrete foundations in expansive soils for light wall claddings           Clause 7.5.13 and Figure 7.22						
Expansive soil class	Slightly 'S'	Moderately 'M'	Highly 'H'	Extremely 'E'		
Soil embedment (De)	375 mm	525 mm	575 mm	625 mm		
Top steel (A <sub>s</sub> top)	2/D 16	2/ D16	2/D16	2/ D16		
Bottom steel (A <sub>s</sub> bottom)	1/ D16	1/ D25	1/ D20	1/ D25		
Stirrups	R6/ 125 crs.	R6/ 125 crs.	R6/300 crs.	R6/ 300 crs.		
Maximum spacing of internal thickenings	no internal thickening	no internal thickening	2.5 m crs.	2.5 m crs.		
Depth of thickening (D1)	-	-	400 mm	450 mm		
Base width (B1)	-	-	300 mm	350 mm		
Top steel (A <sub>s</sub> top)	-	-	2/ D20	2/ D20		
Bottom steel (A <sub>s</sub> bottom)	-	-	2/ D16	2/ D20		
Stirrups	-	-	R6/ 150 crs.	R6/ 150 crs.		

Table 7.4BReinforced concrete foundations in expansive soils for medium wall and heavy wall claddingsClause 7.5.13 and Figure 7.23					
Expansive soil class	Slightly 'S'	Moderately 'M'	Highly 'H'	Extremely 'E'	
Soil embedment (De)	500 mm	550 mm	775 mm	800 mm	
Top steel (A <sub>s</sub> top)	2/ D16	2/ D20	2/ D20	3/ D20	
Bottom steel (A <sub>s</sub> bottom)	2/ D16	2/ D16	2/ D20	2/ D20	
Stirrups	R6/ 125 crs.	R6/ 250 crs.	R6/ 300 crs.	R6/ 300 crs.	
Maximum spacing of internal thickenings	-	2.5 m crs.	2.5 m crs.	2.5 m crs.	
Depth of thickening (D1)	-	350 mm	450 mm	500 mm	
Base width (B1)	-	300 mm	300 mm	350 mm	
Top steel (A <sub>s</sub> top)	-	2/ D16	3/ D20	3/ D20	
Bottom steel (A <sub>s</sub> bottom)	-	2/ D16	2/ D16	2/ D20	
Stirrups	-	R6/ 125 crs.	R6/ 150 crs.	R6/ 150 crs.	

Amend 19 Nov 2019



# Appendix 4

**Post-Construction Borehole Records** 

Client :					Aug	er Bo	oreho		<b>S.</b> Lot 753	
Project Locatio	<b>n:</b> HITCHEN BLOCK STA	GE 13D, POREI	NO	Vane H	lead.	Logge	d By:	Process		of 13
Job Number:	J00113			1750			u by. PL	PL		.10.21
Borehole mN Location: Description:		round R.L.		Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak/ residual</sub>	Soil Sensitivity	Sample Laboratory	
	SOIL DESCRIPTION			Leç	Dep	Sta Wate	V Shea <sup>peak/</sup>	Sens	Tes Deta	
Lot 753										
TOPSOIL					<del>-</del> 0.0					
silty CLAY, orange and d	ark grey streaked brown. Hard, r	noist, medium plas	ticity, with							
trace fine gravel [FILL]	] æd orange/brown. Hard, moist, m	edium plasticity [A	SH1	×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=	-		070			
<ul> <li>becoming orange streake</li> </ul>	-	outant plaotiony [/		×=×=×=×=×=×=× ×=×=×=×=×=×=×=× ×=×=×=×=×	<b>-</b> 0.5		270+			
-				× × × × × × × × × × × × × × × × × × ×	È					
	ge streaked light grey/orange. Me	dium dense, mois	t, no	********	-		070			
plasticity becoming black, dark red	and orange mottled grey				<b>-</b> 1.0		270+			
-					F					
<ul> <li>becoming sensitive</li> </ul>					-		054/60	4.1		
- becoming sensitive					<b>-</b> 1.5		254/62	4.1		
-					F					
-					- 		254/42	6.0		
E.O.B. at 2.0m. Tar	get Depth.				- 2.0		201/12	0.0		
Lot 755					- 0.0					
TOPSOIL										
clayey SILT, light grey, or moderately sensitive [ASI	ange, white and dark grey mottle	d. Hard, moist, lov	v plasticity,		F					
	,				- 		250/116	2.2		
<ul> <li>silty CLAY, orange streak</li> </ul>	ed orange/light grey. Hard, moist	, medium plasticity	, moderat	ely						
_ sensitive				×-×-×-×-×-×-× ×-×-×-×-×-×-×-× ×-×-×-×-×	-					
-				×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=	-1.0		208/104	2.0		
<ul> <li>becoming light grey and b</li> </ul>	prown streaked orange			x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=	È					
	-			x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x x=	E					
<ul> <li>becoming insensitive</li> </ul>				×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×=×=×	-1.5		212/143	1.5		
<ul> <li>becoming dark grey and one</li> </ul>	orange mottled brown			×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=						
- becoming light grey and b	prown/orange streaked orange			×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=	F					
				x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x=x=x=x	-2.0		247/158	1.6		
E.O.B. at 2.0m. Tar	yer Depin.									
					L				•1	+++++++
	Comments:	Borehole Diameter:	Topsoil	- {<<<	and	<u></u>	Sandstone	222222	Plutonic	+++++++++++++++++++++++++++++++++++++++
	Groundwater not encountered. (unless noted)	50mm Checked:	Fill Clay		ravel ganic	******	Siltstone		No Core	
geotechnical	UTP = unable to penetrate. EOB = end of borehole.	RZ	k)	******	imice	******	Limestone Volcanic			

Client :	DFH JOINT VENTURE I				Aug	er Bo	oreho		<b>S.</b> Lot 75	
Project Locatio	<b>n:</b> HITCHEN BLOCK STAC	GE 13B, POKE	NO						Sheet 2	
Job Number:	J00113			Vane H 1750		Logged P	d By: PL	Process PL		.10.21
Borehole MN Location: Description:		ound R.L.		Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak</sub> / residual	Soil Sensitivity	Sample Laboratory	/ / Other
	SOIL DESCRIPTION			Le	De	Sta Wat	She /	Ser	Tes Deta	
slightly clayey SILT, oran sensitive	rown and orange mottled. Hard, m ge mottled grey/brown. Medium de ige and brown mottled dark brown				- 0.0 		270+ 212/39 270+ 212/42	5.5		
Lot 759 TOPSOIL clayey SILT, orange, light	grey and grey mottled. Hard, mois	st, low plasticity [/	ASH]		- 0.0 - - - -		270+			
<ul> <li>silty CLAY, light grey and</li> </ul>	pink streaked orange. Hard, mois	t, medium plastic	ity		<b>-</b> 0.5 - -		270+			
moderately sensitive	d orange mottled grey/brown. Very rey and orange streaked light grey		plasticity,		- - - -		196/62	3.2		
<ul> <li>becoming hard, sensitive</li> <li>-</li> </ul>					- 		266/54	4.9		
E.O.B. at 2.0m. Tar	get Depth.				-2.0		270+			
	Comments:	Borehole Diameter:	Topsoil	s:	and		Sandstone		Plutonic	********
	Groundwater not encountered. (unless noted)	50mm	Fill	///// Gi	ravel		Siltstone	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	No Core	
	UTP = unable to penetrate.	Checked:	Clay -	Or	ganic 🔒	******	Limestone			
georeennical	EOB = end of borehole.	RZ	Silt	**************************************	ımice		Volcanic		ž	

Client :					Aug	er Bo	oreho		<b>S.</b> Lot 76	
Project Locatio	n: HITCHEN BLOCK ST	AGE 13B, POKE	NO			ı.	- 6			of 13
Job Number:	J00113			Vane H 30		Logge F	a By: RZ	Process PL		: 5.10.21
Borehole mN		around R.L.		p	(E	Standing Water Level	e ∢Pa) idual	vity	Sample	e and
Location: Description	Refer to site plan			Legend	Depth (m)	stand ater L	Vane Shear(kPa) <sub>peak</sub> / <sub>residual</sub>	Soil Sensitivity	Laborator Te	y / Other st
	SOIL DESCRIPTION					° Š	S, a	Š	Deta	ails
Lot 761 TOPSOIL					- 0.0					
	aked brown/orange. Hard, moist	modium plaatiaitu			F					
Silly CLAY, light grey stre	aked brown/orange. Hard, moisi	, mealum plasticity	[ASH]	×=×=×=×=×=×=×= ×=×=×=×=×=×=×=×= ×=	~ ~					
-				×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=×=×	-0.5		201+			
-				×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=×	× ×					
-				×=×=×=×=×=×=×= ×=×=×=×=×=×=×=×= ×=	-					
<b>-</b>				x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=	<b>-</b> 1.0		201+			
-				×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=×						
-				×=×=×=×=×=×=×= ×=×=×=×=×=×=×=×= ×=	×					
-				x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=	<b>-</b> 1.5		201+			
-				×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=×						
at 2.0m, becoming very s E.O.B. at 2.0m. Tar				×=×=×=×=×=×=×= ×=×=×=×=×=×=×= ×=×=×=×=×	- - 2.0		164/141	12		
Lot 763										
TOPSOIL					- 0.0					
	grey mottled light brown. Very si	iff, moist, low plast	icity, extr	a 🚧						
_ sensitive [FILL] 					<u>}</u>		1 41 /1 7	0.0		
-					-0.5		141/17	8.2		
-					-					
<ul> <li>becoming insensitive</li> </ul>					- - 		150/83	1.8		
-					1.0		100/00	1.0		
-					}					
<ul> <li>becoming hard</li> </ul>					-1.5		201+			
-					1					
					-					
	ant Dopth				-2.0		201+			
E.O.B. at 2.0m. Tar	ует рерш.									
	l		r –				1			++++++++
	<b>Comments:</b> Groundwater not encountered.	Borehole Diameter: 50mm	Topsoil		and iravel		Sandstone Siltstone		Plutonic No Core	*******
LANDER	(unless noted)	Checked:	Fill Clay		ravel	******	Limestone			
geotechnical	UTP = unable to penetrate. EOB = end of borehole.	RZ	Silt	****	umice		Volcanic			

Client : Project	Locatio		FH JOINT VENTU ITCHEN BLOCK S			NO			Aug	er Bo	orehol			66 & 768
Job Nu			00113					Vane I 30		Logge F	d By: RZ	Process PL	or: Dat	
Borehole Location:	mN Description	:	mE Refer to site plan	Gro	ound R.L.			Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak</sub> residual	Soil Sensitivity	Samp Laborato	ole and ory / Other
		SO	IL DESCRIPTIO	N				Ĺe	Det	Sta Wate	She: Peak	Sen		est tails
plasticity [FI silty CLAY, becoming lig becoming ve at 2.0m, bec	LL] black mottled ght grey and n	orange.	ange and red mottled Hard, moist, high pla	d bro		medium			- 0.0 - 0.5 - 1.0 - 1.5 		201+ 201+ 147/135 201+			
Lot 768 TOPSOIL clayey SILT low plasticity	with trace fin y [FILL] ery stiff, sens , light brown.	ie sand, c itive Very stiff get Dep	orange and light grey			v sensitiv			- 0.0 - 0.5 - 1.0 - 1.5 - 2.0		201+ 141/26 190/58 201+	5.4	Plutonic	
		Comme Groundy	ents: water not encountere	ed.	Borehole Diameter: 50mm	Topsoil Fill		₩-	and :		Sandstone Siltstone		Plutonic	*******
	DER	(unless	noted)	<i>.</i>	Checked:	Clay	////		rganic	******	Limestone			
geotech	nical		nable to penetrate. and of borehole.		RZ	Silt	×××× ×××× ××××	< X X X	umice		Volcanic		Ž.	

Client : Project Locatio	DFH JOINT VENTU		NO		Aug	er Bo	orehol		<b>S.</b> Lot 77( Sheet 5	
-		THE TOD, TONE		Vane H	lead:	Logge	d By:	Process		of 13 :
Job Number:	J00113			307	7		RZ	PL	26	.10.21
Borehole <u>mN</u> Location: Description:	mE Bofor to site plan	Ground R.L.		pu	(m) r	Standing Water Level	Vane Shear(kPa) <sub>peak</sub> / <sub>residual</sub>	il tivity	Sample	e and
Description		N1		Legend	Depth (m)	Stane Vater	Val Shear <sup>peak/re</sup>	Soil Sensitivity	Laboratory Tes Deta	st
	SOIL DESCRIPTIO					>				
Lot 770										
TOPSOIL					<del>-</del> 0.0 -					
silty CLAY, light grey and	d red mottled orange/brown. V	ery stiff, moist, mediu	um plasticit	ty,	-					
_ insensitive [FILL]					-		150/115	1.2		
-					<del>-</del> 0.5 -		150/115	1.5		
-					-					
<ul> <li>becoming moderately set</li> </ul>	ensitive				- 		150/49	3.1		
-					-					
_ clayey SILT, grey mottled	d red. Hard, moist, low plastici	ty [ASH]			_					
_					<del>-</del> 1.5		201+			
-					-					
-					-					
E.O.B. at 2.0m. Tar	get Depth.			******	<del>-</del> 2.0		201+			
Lot 772					- 0.0					
TOPSOIL					-					
clayey SILT, orange/brov	vn. Very stiff, moist, medium p	lasticity, insensitive [	ASH]	×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×=	-					
-				×=×=×=×=×=×=× ×=×=×=×=×=×=×=× ×=×=×=×=×	- 		138/95	1.5		
-				×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×=×=×	-					
-				×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×=×=×=×=×	_					
becoming orange and br	own mottled, low plasticity, wit	h trace fine sand		×-×-×-×-×-× ×-×-×-×-×-×-× ×-×-×-×-×-×-×	<del>-</del> 1.0		150/98	1.5		
	own motified, low plasticity, wit			×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×	F					
-				×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×	-					
<ul> <li>becoming moderately ser</li> </ul>	nsitive			×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=	<b>-</b> 1.5		178/55	3.3		
-					_					
at 2.0m, becoming hard				×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=×=	-					
E.O.B. at 2.0m. Tar	get Depth.			x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=x=	-2.0		201+			
	Comments:	Borehole Diameter:	Topsoil	Sa	and		Sandstone		Plutonic	*******
	Groundwater not encountere	d. 50mm	Fill	////// Gr	avel		Siltstone		No Core	
<b>LANDER</b> geotechnical	(unless noted) UTP = unable to penetrate.	Checked:	Clay	Or	ganic		Limestone		<u></u>	
	EOB = end of borehole.	RZ	Silt	××××××××× ××××××××××××××××××××××××××××	mice		Volcanic		<u></u>	

Client :	Project Location : HITCHEN BLOCK STAGE 13B, POKENO				Aug	er Bo	oreho	e No	<b>S.</b> Lot 776	8 & 778
Project Location	<b>n:</b> HITCHEN BLOCK ST	AGE 13B, POKE	NO						Sheet 6	of 13
Job Number:	J00113			Vane H 1750		Logge F	d By: PL	Process PL		.10.21
Borehole mN Location: Description:		Ground R.L.		p	(m) (	ding Level	ле (kPa) <sup>sidual</sup>	il ivity	Sample	
Location: Description:	Refer to site plan			Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak</sub> residual	Soil Sensitivity	Laboratory Tes Deta	st
Lot 776										
TOPSOIL					<del>-</del> 0.0					
	inge and dark grey streaked or	ange/brown. Hard, r	noist,		-					
medium plasticity, with tra becoming dark grey and					-					
<b>-</b>					<b>-</b> 0.5		270+			
-					-					
plasticity [ASH]	orange/red streaked orange/b	own. Hard, moist, n	nedium	×=×=×=×=×=×=× ×=×=×=×=×=× ×=×=×=×=×=×=×	-					
becoming orange and bro	own streaked light grey/brown			x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x-x-x x-x-x-x-x-x-x-x-x-x	<b>-</b> 1.0		270+			
	k, light grey and dark grey mot	led. Hard, moist, no	)		-					
_ plasticity	plasticity				-		070			
•					<del>-</del> 1.5 -		270+			
					-					
-					- <b>-</b> 2.0		270+			
E.O.B. at 2.0m. Tar	get Depth.				- 2.0		2701			
Lot 778					- 0.0					
TOPSOIL					-					
clayey SILT, orange, blac	k and brown mottled. Hard, mo	ist, low to no plastic	ty [ASH]		-					
silty CLAY, black and light	t grey streaked orange/brown. I	Hard, moist, mediun	n plasticity	×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=× ×=×=×=×=×=×=×=×=×=×	- 		270+			
<ul> <li>becoming dark red and or</li> </ul>	range streaked orange/brown			×-×-×-×-×-×-×-× ×-×-×-×-×-×-×-× ×-×-×-×-×-×-×-× ×-×-×-×-×-×-×-×	-					
-	nd orange streaked light grey/o	ange. Hard, moist,	medium	×=×=×=×=×=×=× ×=×=×=×=×=× ×=×=×=×=×=×=×	-					
<ul> <li>plasticity</li> <li>becoming orange streake</li> </ul>	ed light grey, moderately sensiti	ve		×=×=×=×=×=×=× ×=×=×=×=×=×=×=× ×=×=×=×=×	-1.0		254/119	2.1		
<ul> <li>becoming light grey and r</li> </ul>	ed streaked orange			x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x x=x=x=x=x=x=x=x=x=x x=	-					
-				×=×=×=×=×=×=×=× ×=×=×=×=×=×=× ×=×=×=×=×	-					
slightly clavev SILT. orang	<ul> <li>slightly clayey SILT, orange, light grey and black streaked brown. Dense, moist, no</li> </ul>				-1.5		270+			
plasticity					-					
at 2.0m, becoming sensitive					-					
	E.O.B. at 2.0m. Target Depth.				-2.0		231/35	6.7		
	gor D'oprin									
	• ·	Develope D'	[N					1	• Dista	
	Comments: Groundwater not encountered	Borehole Diameter: 50mm	Topsoil Fill	Sa Gra	nd avel		Sandstone Siltstone	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	Plutonic	******
LANDER	(unless noted) UTP = unable to penetrate.	Checked:	Clay		janic 🛱		Limestone			
geotechnical	EOB = end of borehole.	RZ	Silt XX	××××××× ××××××××××××××××××××××××××××××	nice		Volcanic			

Client : Project Locatio	DFH JOINT VENTUR		NO		Aug	er Bo	orehol		<b>S.</b> Lot 78 Sheet 7	
-				Vane H		Logge		Process	or: Date	):
Job Number:	J00113			1750		I	2	PL	2	6.10.21
Borehole <u>mN</u> Location: Description:	mE Refer to site plan	Ground R.L.		Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak/ residual</sub>	Soil Sensitivity	Samp Laborato	e and y / Other
	SOIL DESCRIPTIO	N		Leg	Dep	Sta Wate	Shea Peak	Sens	Te Det	st
plasticity, moderately ser at 0.4m, becoming orang	ge, light grey and brown streak isitive [ASH] e and light grey streaked brow and dark brown mottled orange	ed red/pink. Very stif n/orange	f, moist, low		- 0.0 		169/81 177/73 173/65 162/81	2.1 2.4 2.6 2.0		
Lot 783 TOPSOIL					- 0.0					
low to no plasticity, sensi	e sand, orange and light grey tive [ASH] y clayey SILT, no plasticity	streaked orange/red.	Hard, moisi		- - - - - - - - - -		216/31 216/19	7.0		
- - - - - - -					- - - - - - - - -		189/23 200/15	8.2		
E.O.B. at 2.0m. Tar	get Depth. Comments:	Borehole Diameter:	Topsoil	Si	and		Sandstone	1	Plutonic	+++++++++++++++++++++++++++++++++++++++
	Groundwater not encountered (unless noted)	d. 50mm	Fill	//// G	ravel		Siltstone	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	No Core	
<b>LANDER</b> geotechnical	(UTP = UTP	Checked: RZ	Clay	×××××	ganic S Imice		Limestone			

Project Location : HITCHEN BLOCK STAGE 138, POKENO         Job Number: J00113         Workered: Larged by PUM       Processor Dire: 7002153         Receive and manual space of the set plan         TOPSOL         Job Number: J00113         SOIL DESCRIPTION         SOIL DESCRIPTION         Lat 785         TOPSOL         Interfere to the set plan         TOPSOL         Lot 785         Colspan="2">Soil DESCRIPTION         Lot 786         Interfere to the set of the s	Client :	DFH JOINT VENTURE				Aug	er Bo	oreho	e No	<b>S.</b> Lot 78	5 & 786
JOD NUMBER:         JOD 113         Trop2153         PLUM         PL         28.10.21           Borehold Location         mit         Rear to ale glan         mg         g <t< th=""><th>Project Locatio</th><th>n: HITCHEN BLOCK ST</th><th>AGE 13B, POKE</th><th>NO</th><th></th><th></th><th></th><th></th><th></th><th>-</th><th></th></t<>	Project Locatio	n: HITCHEN BLOCK ST	AGE 13B, POKE	NO						-	
Lot 785       0.0       <	Job Number:	J00113									
Lot 785       0.0       <	Dorenole		Ground R.L.		pue	(m) ti	iding ' Level	tne r(kPa) esidual	oil itivity	Sampl	e and v / Other
TOPSOIL       -0.0       -0.0         sightly dipky SIT with trace fine sand, orange streaked yellow/brown. Very stiff, molet, it with an plasticity (ASH)       -0.5       154.99       4.0         silly CLAY, orange streaked orange/ight grey       -0.5       154.99       4.0         becoming orange streaked orange/ight grey       -0.5       154.99       4.0         clayer, SIT, orange streaked orange/ight grey.       -1.5       216.92       3.5         E.O.B. at 2.0m. Target Depth.       -0.0       -1.5       270+         E.O.B. at 2.0m. Target Depth.       -0.0       -0.5       181/135       1.3         becoming well       -0.5       181/135       1.3       -1.5       2.0       2.0         E.O.B. at 2.0m. Target Depth.       -0.5       181/135       1.3       -1.5       1.5       2.2         becoming high plasticity       -0.5       181/135       1.3       -1.5       2.2       2.0       1.0       -1.5       2.2       2.0       1.0       -1.5       2.2       2.0       1.0       -1.5       2.2       2.0       1.0       -1.5       1.5       1.5       -1.5       1.5       2.2       2.0       1.0       -1.5       1.5       2.2       -1.5       1.5       2.2		•			Leg	Dept	Star Watei	Va Shea <sub>peak/ 1</sub>	S. Sens	Te	st
TOPSOUL shipty Gray SIT with trace fine sand, orange streaked yellow/brown. Very stiff, moist, it with on a plasticity (ASH)       -0.0       154.39       4.0         slipty CLAY, orange streaked orange/light grey       -0.0       -0.5       154.39       4.0         becoming orange streaked orange/light grey       -0.0       -0.5       154.39       4.0         clayer, SIT, orange streaked orange/light grey.       -1.5       270.4       -1.5       270.4         E.O.B. at 2.0m. Target Depth.       -0.0       -1.5       270.4       -1.5       270.4         E.O.B. at 2.0m. Target Depth.       -0.5       191/135       1.3         becoming high plasticity       -0.0       -1.5       191/135       1.3         becoming high plasticity       -1.5       191/135       1.3         tat 2.0m. Target Depth.       -1.5       190/11       122         becoming high plasticity       -1.5       190/11       1.2         tat 2.0m. becoming intensitive       -1.5       190/11       1.5         tat 2.0m. becoming intensitive       -1.5       2.0       170/116       1.5         tat 2.0m. becoming intensitive       -1.5       1.5       1.5       1.5         tat 2.0m. becoming intensitive       -1.5       1.5       1.5											
LDPSOIL       Eighthy days SILT with trace fine sand, orange streaked yellow/brown. Very stiff, moist, involve no plasticity, sensitive       -0.5       154.03       4.0         silptiv days SILT, orange streaked orange/light grey.       -1.5       216.62       3.5         ecoming orange streaked orange/light grey.       -1.5       270.4       -1.5       270.4         ecoming orange streaked orange/light grey.       -1.5       270.4       -1.5       270.4         E.O.B. at 2.0m. Target Depth.       -0.5       151/135       1.3         becoming high plasticity       -0.5       151/135       1.3         becoming high plasticity       -0.5       151/135       1.3         in Constraint       -0.5       151/135 <td>Lot 785</td> <td></td>	Lot 785										
Low to no plasticity (LSH)         54.0         1.54.09         4.0           becoming orange streaked orange/light grey. Very stiff, moist, medium plasticity, moderately         -0.5         1.54.09         4.0           clawy SIL r, orange streaked orange/light grey.         -1.5         216.62         3.5           clawy SIL r, orange streaked orange/light grey. Hard, moist, low plasticity, moderately         -1.5         270+           clawy SIL r, orange streaked orange/light grey. Hard, moist, low plasticity, moderately         -2.0         270+           E.O.B. at 2.0m. Target Depth.         -0.6         181/135         1.3           becoming high plasticity         -0.5         181/135         1.3           thild CLAY, redorange and orange motiled light grey. Very stiff, moist, medium to high         -0.5         181/135         1.3           becoming high plasticity         -0.5         181/135         1.3         -1.0         182/96         1.3           tat 2.0m. becoming insensitive         -1.5         198/61         3.2         -1.5         198/61         3.2           tat 2.0m. becoming insensitive         -1.5         198/61         3.2         -1.5         198/61         3.2           tat 2.0m. becoming insensitive         -1.5         198/61         3.2         -1.5         1.5 <t< td=""><td>TOPSOIL</td><td></td><td></td><td></td><td></td><td><del>-</del> 0.0</td><td></td><td></td><td></td><td></td><td></td></t<>	TOPSOIL					<del>-</del> 0.0					
becoming orange streaked orange/light grey       -10       216/62       3.5         clayey SLT, orange streaked orange/light grey. Hard, moist, low plasticity, moderately       -10       216/62       3.5         e.o.B. at 2.0m. Target Depth.       -20       270+       -15       270+         E.O.B. at 2.0m. Target Depth.       -00       -00       -00       -00         silly (2.4V, redorange and orange motied light grey. Very stiff, moist, medium to high plasticity       -00       181/135       1.3         becoming wat becoming motiently sensitive       -1.5       198/61       2.2       198/61       2.2         tat 2.0m. Target Depth.       -1.5       198/61       1.3       -1.5       198/61       2.2         becoming wat becoming insensitive       -1.5       198/61       2.2       170/116       1.5         E.O.B. at 2.0m. Target Depth.       -2.0       170/116       1.5       1.5       1.5       1.6         becoming wat becoming insensitive       -2.0       170/116       1.5       1.5       1.5       1.6         E.O.B. at 2.0m. Target Depth.       5mm       5mm       5mm       5mm       5mm       1.6       1.6       1.6         E.O.B. at 2.0m. Target Depth.       5mm       5mm       5mm       5mm </td <td></td> <td></td> <td>yellow/brown. Ver</td> <td>y stiff, moi</td> <td>st,</td> <td>-</td> <td></td> <td></td> <td></td> <td></td> <td></td>			yellow/brown. Ver	y stiff, moi	st,	-					
clayey SLT, orange streaked orange/light grey. Hard, moist, tow plasticity, moderately       -1.0       216/92       3.5         ensitive       -1.5       270+       -1.5       270+         E.O.B. at 2.0m. Target Depth.       -2.0       270+       -2.0       270+         silly CLAY, redorange and orange motited light grey. Very still, moist, medium to high plasticity, insensitive [ASH]       -0.0       -0.0       -0.0         silly CLAY, redorange and orange motited light grey. Very still, moist, medium to high plasticity, insensitive [ASH]       -0.5       181/135       1.3         becoming high plasticity       -1.5       189/61       3.2       -1.5       198/61       3.2         at 2.0m, becoming insensitive       -2.0       170/116       1.5       -2.0       1.0       1.5       -2.0       1.0       1.5         becoming wet       -2.0       120/116       1.5       -2.0       1.0       1.5       -2.0       1.0       1.5         E.O.B. at 2.0m. Target Depth.       -2.0       5       5       5       5       -2.0       1.0       1.5       -2.0       1.0       1.5         Bartele Damete:       Comments:       Commentare       Commentare       Commentare       Commentare       1.0       5       5       -1.0	silty CLAY, orange strea	ked light grey. Very stiff, moist, r	nedium plasticity, s	sensitive	2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 -	- 		154/39	4.0		
seisitive         10         2.10         2.10         0.3           E.O.B. at 2.0m. Target Depth.         20         270+         2           E.O.B. at 2.0m. Target Depth.         -0.0         -0.0         -0.0           Silv CLAY, redorange and orange motiled light grey. Very still, moist, medium to high plasticity.         -0.5         181/135         1.3           becoming high plasticity         -1.5         199/61         3.2         -1.5         199/61         3.2           becoming wet         -0.0         -1.5         199/61         3.2         -1.5         199/61         3.2           to comming insensitive         -1.5         199/61         3.2         -2.0         170/116         1.5           to comming insensitive         -2.0         170/116         1.5         1.5         1.5           to comming insensitive         -2.0         170/116         1.5         1.5         1.5         1.5           to comming insensitive         -1.5         199/61         3.2         -2.0         170/116         1.5           to comments:         Groundwater not encountered, UTP = unable to ponetrate.         Topol         Sand         Sand         Sand         Sand         Sand         Sand         Sand         Sand	<ul> <li>becoming orange streake</li> </ul>	ed orange/light grey				-					
E.O.B. at 2.0m. Target Depth.       20       270+         Lot 786       -0.0       -0.0         TOPSOIL       -0.0       -0.0         sity CLAY, redorange and orange motiled light grey. Very still, moist, medium to high plasticity       -0.5       181/135       1.3         becoming high plasticity       -0.5       181/135       1.3         becoming wet becoming moderately sensitive       -1.0       182/96       1.9         becoming moderately sensitive       -1.5       198/61       3.2         E.O.B. at 2.0m. Target Depth.       -2.0       170/116       1.5         FORMERTS:       Sorthole Damter:       -2.0       Sandatore       -2.0         Connents:       Sorthole Damter:       -2.0       Sandatore       -2.0         File - end of benchole.       -2.0       20       -2.0       -2.0		aked orange/light grey. Hard, mo	pist, low plasticity, r	moderatel	y xxxxxxxx xxxxxxxxx xxxxxxxxxx xxxxxxxx	- 		216/62	3.5		
E.O.B. at 2.0m. Target Depth.       -2.0       270+         Lot 786       -0.0       -0.0         TOPSOIL       -0.0       -0.0         sitty CLAY, redorange and orange motiled light grey. Very stiff, moist, medium to high plasticity       -0.5       181/125       1.3         becoming high plasticity       -0.5       181/125       1.3         becoming moderately sensitive       -1.0       182/36       1.9         becoming moderately sensitive       -1.5       198/61       3.2         E.O.B. at 2.0m. Target Depth.       -2.0       170/116       1.5         Comments:       Sorthole Dameter       Topsal       Sard       Sard target Plance       Plance         Groundwater not encountered, UTP = unable to penetrate.       RZ       Paraget Market Plance       Plance       Plance         DECONTING       RZ       Paraget Market Plance       Plance       Plance       Plance       Plance	-					-					
E.O.B. at 2.0m. Target Depth.	- -					- 		270+			
E.O.B. at 2.0m. Target Depth.	-					-					
TOPSOIL       0.0       0.0       0.0         silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH]       0.0       181/135       1.3         becoming high plasticity       0.0       181/135       1.3       1.3         becoming wet       0.0       182/96       1.9         becoming moderately sensitive       1.5       198/61       3.2         at 2.0m, becoming insensitive       2.0       170/116       1.5         E.O.B. at 2.0m. Target Depth.       50mm       5mm       5mm       Sand       Sandatore       9000000000000000000000000000000000000	- E.O.B. at 2.0m. Tar	get Depth.				- 		270+			
TOPSOIL       -0.0       -0.0       -0.0         silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH]       -0.5       181/135       1.3         becoming high plasticity       -0.5       181/135       1.3         becoming wet       -0.5       182/96       1.9         becoming moderately sensitive       -1.0       182/96       1.9         at 2.0m, becoming insensitive       -1.5       198/61       3.2         et 2.0m, becoming insensitive       -2.0       170/116       1.5         E.O.B. at 2.0m. Target Depth.       Sand       Sand to 2       Sand to 2         Groundwater not encountered. (nees noted)       Othecked:       Checked:       Carvet       Sand to 2         UTP = unable to penetrate.       Sand       Sand to 2       Sand to 2       No Core         Checked:       Cay       Carvet       Sand to 2       Sand to 2       No Core											
TOPSOIL       -0.0       -0.0       -0.0         silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH]       -0.5       181/135       1.3         becoming high plasticity       -0.5       181/135       1.3         becoming wet       -0.5       182/96       1.9         becoming moderately sensitive       -1.0       182/96       1.9         at 2.0m, becoming insensitive       -1.5       198/61       3.2         et 2.0m, becoming insensitive       -2.0       170/116       1.5         E.O.B. at 2.0m. Target Depth.       Sand       Sand to 2       Sand to 2         Groundwater not encountered. (nees noted)       Othecked:       Checked:       Carvet       Sand to 2         UTP = unable to penetrate.       Sand       Sand to 2       Sand to 2       No Core         Checked:       Cay       Carvet       Sand to 2       Sand to 2       No Core											
TOPSOIL       -0.0       -0.0       -0.0         silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH]       -0.5       181/135       1.3         becoming high plasticity       -0.5       181/135       1.3         becoming wet       -0.5       182/96       1.9         becoming moderately sensitive       -1.0       182/96       1.9         at 2.0m, becoming insensitive       -1.5       198/61       3.2         et 2.0m, becoming insensitive       -2.0       170/116       1.5         E.O.B. at 2.0m. Target Depth.       Sand       Sand to 2       Sand to 2         Groundwater not encountered. (nees noted)       Othecked:       Checked:       Carvet       Sand to 2         UTP = unable to penetrate.       Sand       Sand to 2       Sand to 2       No Core         Checked:       Cay       Carvet       Sand to 2       Sand to 2       No Core											
silty CLAY, red/orange and orange mottled light grey. Very stiff, moist, medium to high plasticity, insensitive [ASH] becoming high plasticity -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 181/135 1.3 -0.5 182/96 1.9 -1.5 198/61 3.2 -1.5 198/61 3.2 -1.5 198/61 3.2 -1.5 198/61 3.2 -2.0 170/116 1.5 Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = not of borehole. BZ Stat During Comments: C						- 0.0					
plasticity, insensitive [ASH]         becoming high plasticity         -0.5         181/135         1.3         becoming high plasticity         -1.0         182/96         -1.0         182/96         -1.0         182/96         -1.0         182/96         -1.0         182/96         -1.0         182/96         1.9         -1.5         198/61         3.2         -1.5         198/61         3.2         -2.0         170/116         1.5         2.0         170/116         1.5         2.0         170/116         1.5         2.0         170/116         1.5         2.0         170/116         1.5         2.0         2.0         2.0         2.0         2.0         2.0         2.0         2.0         2.0         2.0		nd orange mottled light grev. Ver	v stiff moist medi	um to hiat	×	-					
becoming high plasticity = 1.0   182/96   1.9 = 1.0   182/96   1.9 = 1.0   182/96   1.9 = 1.5   198/61   3.2 = 1.5   198/61   3.2 = 2.0   170/116   1.5 E.O.B. at 2.0m. Target Depth. = 2.0   170/116   1.5 = 2.0   170/116   1.5   1.5 = 2.0   170/116   1.5			y stin, moist, mean	uni to nigi	×=×=×=×=×=×=× ×=×=×=×=×=×=× ×=×=×=×=×=×	-					
becoming wet becoming moderately sensitive at 2.0m, becoming insensitive E.O.B. at 2.0m. Target Depth. Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole. Borehole Diameter: Somm Fill Checked: Clay Corganic Sand Sand Sand Sand Sand Sand Sand Sand Sand Sittsone Corganic Sittsone Corganic Sittsone Sitt	<ul> <li>becoming high plasticity</li> </ul>					<del>-</del> 0.5 - -		181/135	1.3		
becoming wet becoming moderately sensitive at 2.0m, becoming insensitive E.O.B. at 2.0m. Target Depth. Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole. Borehole Diameter: Somm Fill Checked: Clay Corganic Sand Sand Sand Sand Sand Sand Sand Sand Sand Sittsone Corganic Sittsone Corganic Sittsone Sitt	-				× × × × × × × × × × × × × × × × × × ×	- -		100/00			
becoming moderately sensitive       198/61       3.2         at 2.0m, becoming insensitive       -2.0       170/116       1.5         E.O.B. at 2.0m. Target Depth.       -2.0       170/116       1.5         Image: Sensitive Sensitive       -2.0       170/116       1.5         Image: Sensitive Sensitive Sensitive       -2.0       170/116       1.5         Image: Sensitive Senst	- - -							182/96	1.9		
at 2.0m, becoming insensitive       -2.0       170/116       1.5         E.O.B. at 2.0m. Target Depth.       -2.0       170/116       1.5         Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole.       Borehole Diameter: 50mm       Topsoil       Sand       Sand       Sandstone       Plutonic       Edited to the penetrate. EOB = end of borehole.						-					
E.O.B. at 2.0m. Target Depth.       Plutonic         E.O.B. at 2.0m. Target Depth.       Plutonic         Image: Second Secon	<ul> <li>becoming moderately se</li> </ul>	nsitive				<del>-</del> 1.5 - -		198/61	3.2		
E.O.B. at 2.0m. Target Depth.       Plutonic         E.O.B. at 2.0m. Target Depth.       Plutonic         Image: Second Secon	- at 20m becoming incom	sitive				-					
LANDER       Groundwater not encountered. (unless noted)       50mm       Fill       Gravel       Siltstone       222222222222222222222222222       No Core         UTP = unable to penetrate.       EOB = end of borehole.       RZ       Silt       Silt       Values of the second of the	-				<u>x-x-x-x-x-x-x</u> <u>x-x-x-x-x-x-x</u>	<del>-</del> 2.0		170/116	1.5		
LANDER       Groundwater not encountered. (unless noted)       50mm       Fill       Gravel       Siltstone       222222222222222222222222222       No Core         UTP = unable to penetrate.       EOB = end of borehole.       RZ       Silt       Silt       Values of the second of the											
LANDER       Groundwater not encountered. (unless noted)       50mm       Fill       Gravel       Siltstone       222222222222222222222222222       No Core         UTP = unable to penetrate.       EOB = end of borehole.       RZ       Silt       Silt       Values of the second of the		Commente	Borehole Diamator	Tanas		nd 🖸		Condeter		Plutonia	+++++++++++++++++++++++++++++++++++++++
LANDER geotechnical UTP = unable to penetrate. EOB = end of borehole. RZ Silt ******** Pumice ************************************					- <u>K&lt;&lt;&lt;&lt;</u>		22222		222222		******
geotechnical EOB = end of borehole. RZ		(unless noted)					******				
					×××××××× ××××××××	64		5.		<u>루</u>	

Client : Project Locatio	DFH JOINT VENTURE I		NO		Aug	er Bo	oreho		<b>S.</b> Lot 788 Sheet 9	
Job Number:	J00113	-	Vane H		Logge	-	Process	or: Date:	:	
		ound R.L.		2153		NL e		PL	26	.10.21
Borehole MN Location: Description:				Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak</sub> / <sub>residual</sub>	Soil Sensitivity	Sample Laboratory	/ / Other
	SOIL DESCRIPTION			Leç	Dep	Sta Wate	V Shea peak∕	Sen	Tes Deta	
<ul> <li>moist, medium to high pla</li> <li>[FILL]</li> <li>becoming yellow/brown, find</li> <li>becoming hard</li> <li>silty CLAY with trace fine</li> </ul>	eaching	trace fine to med	ium gravel		- 0.0 		191/88 216+ 216+ 216+	2.2		
Lot 790					- 0.0					
TOPSOIL					-					
	range, yellow/brown and grey/brown trace fine to medium gravel [FILL]		rey. Hard,		- - - - - -		216+			
silty CLAY, orange mottle FORMATION]	ed light grey. Hard, moist, high plas	ticity [PUKETOK	A	X-X-X-X-X-X-X X-X-X-X-X-X-X X-X-X-X-X-X	<b>-</b> 1.0		216+			
becoming orange and rec becoming very stiff, inser					- - - - - - - - - -		198/136 185/142	1.5		
E.O.B. at 2.0m. Tar	get Depth.								:1	<u> +++++++++</u>
	Comments: Groundwater not encountered.	Borehole Diameter: 50mm	Topsoil Fill	<del>}}}}</del>	and avel		Sandstone Siltstone		Plutonic	*******
LANDER	(unless noted) UTP = unable to penetrate.	Checked:	Clay		ganic	*******	Limestone			
geotechnical	EOB = end of borehole.	RZ	Silt	××××××× ×××××××× ××××××××	mice		Volcanic		~	

Client :	DFH JOINT VENTURE L				Aug	er Bo	oreho		<b>S.</b> Lot 79	5 & 797
Project Locatio	n: HITCHEN BLOCK STAC	GE 13B, POKEI	NO							of 13
Job Number:	J00113			Vane H 2153		Logge J	d By: M	Process PL		.10.21
Borehole mN	mE Gro	ound R.L.			Ê	ng evel	e Pa) <sup>dual</sup>	ity	Sample	and
Location: Description:	Refer to site plan			Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak</sub> / <sub>residual</sub>	Soil Sensitivity	Laborator	/ Other
	SOIL DESCRIPTION			Le	De	St Wat	She	Sei	Deta	
Lot 795										
					<del>-</del> 0.0					
silty CLAY, orange and yo [FILL]	ellow/brown mottled light grey. Ver	y stiff, moist, higł	n plasticity	y						
<ul> <li>silty CLAY with minor fine</li> <li>becoming high plasticity,</li> </ul>	e sand, yellow/brown. Hard, moist, with trace fine sand	medium plasticity	/ [ASH]		-0.5 -		216+			
<ul> <li>becoming very stiff, insen</li> </ul>	sitive				- 		193/130	1.5		
-										
<ul> <li>becoming hard</li> <li>.</li> </ul>					<del>-</del> 1.5 -		216+			
<ul> <li>becoming orange and rec</li> </ul>	d/orange mottled light grey				F					
E.O.B. at 2.0m. Tar	get Depth.			<u>x=x=x=x=x=x=x</u>	- 2.0		216+			
Lot 797					- 0.0					
TOPSOIL										
silty CLAY with trace fine moist, medium plasticity,	sand, orange and red/orange mot insensitive [ASH]	ted light grey. Ve	ry stiff,		- - - - - -		178/102	1.7		
- becoming light yellow/bro -	wn, stiff, wet, with trace fine to me	dium sand			- - -		142/73	1.9		
- 					- 		77/42	1.8		
-					-					
E.O.B. at 2.0m. Tar	get Depth.			<u>b-x-x-x-x-x-x</u>	-2.0		99/56	1.8		
	Comments:	Borehole Diameter:	Topsoil	Sa State	and		Sandstone	,	Plutonic	++++++ +++++++ +++++++++++++++++++++++
	Groundwater not encountered. (unless noted)	50mm	Fill	G	ravel		Siltstone	2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	<sup>2</sup> 2 2 2 2 2 2	
<b>LANDER</b> geotechnical	UTP = unable to penetrate.	Checked: RZ	Clay	<	ganic 🕄		Limestone		<u>異</u>	
	EOB = end of borehole.	ΠĽ	Silt	XXXXXXXX Pu	ımice	**************************************	Volcanic			

Client : Project Locatio	NO		Aug	er Bo	oreho		<b>S.</b> Lot 798 Sheet 11			
Job Number:	Job Number:         J00113           Borehole         mN         mE         Ground R.L.					Logge F	d By: RZ	Process PL	sor: Date	
Borehole mN Location: Description:		ound R.L.		Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak</sub> / residual	Soil Sensitivity	Sample	/ Other
	SOIL DESCRIPTION			Le	Del	Sta Wate	∨ She <sub>peak</sub>	Sen	Tes Deta	
Lot 798 silty CLAY, orange. Very stiff, moist, medium plasticity, sensitive [ASH] becoming high plasticity becoming moderately sensitive becoming black mottled brown, low plasticity, with some magnese oxide inculsions becoming hard E.O.B. at 2.0m. Target Depth.					<b>-</b> 0.0 - - - - - - - - - - - - -		115/29 161/46 201+ 201+	4.0		
TOPSOIL silty CLAY, orange and li [FILL]					-0.0 -0.5 -1.0 -1.5 -2.0		201+ 201+		Plutonic	*******
Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole.Borehole Diameter: 50mmTopsoilChecked: RZClay sitt					Gravel Organic		Sandstone Siltstone Limestone	2 2	No Core	

Client :	DFH JOINT VENTURE I				Aug	er Bo	oreho	le No	<b>S.</b> Lot 80	)2 & 806
Project Locatio	n: HITCHEN BLOCK STAC	GE 13B, POKE	NO						Sheet 12	
Job Number:	J00113			Vane I 307		Logge F	d By: RZ	Process PL		e: 6.10.21
Borehole mN	mE Gro	ound R.L.								
Location: Description				Legend	Depth (m)	inding er Lev	Vane Shear(kPa) <sub>peak/ residual</sub>	Soil Sensitivity	Sampl Laborator	ry / Other
	SOIL DESCRIPTION			Leç	Dep	Standing Water Level	Shea Peak	Sen	Te Det	
Lot 802					- 0.0					
TOPSOIL					10.0					
_ (FILL) 	AND, orange mottled brown. Mediu	ım dense, moist,	low plastic	sity	- - - - 0.5		UTP			
EOB at 0.7m. Too hard t - - -	o auger further				- - 					
- - - -					- - - 1.5 -					
- - <b>-</b>					- - - 2.0					
Lot 806										
TOPSOIL					- 0.0 -					
silty fine SAND, brown. N	ledium dense, dry, no plasticity, wi	th trace fine grav	el [FILL]							
EOB at 0.4m. Too hard to	o auger further				-0.5					
-					F					
-					F					
-					- 1.0					
-					F					
-										
-					<b>-</b> 1.5					
-					F					
-					-2.0					
		Developing in	4		 E					+++++++++++++++++++++++++++++++++++++++
	<b>Comments:</b> Groundwater not encountered.	Borehole Diameter: 50mm	Topsoil Fill	*****	and Gravel		Sandstone Siltstone		<ul> <li>Plutonic</li> <li><sup>2</sup>/<sub>2</sub> No Core</li> </ul>	++++++++
LANDER	(unless noted) UTP = unable to penetrate.	Checked:	Clay		rganic	*******	Limestone	222222	2	
geotechnical	EOB = end of borehole.	RZ	Silt	××××××××× ××××××××××××××××××××××××××××	umice		Volcanic			

Client :	DFH JOINT VENTURE I				Aug	er Bo	oreho	le No	<b>S.</b> Lot 80	7 & 809
Project Locatio	n: HITCHEN BLOCK STAC	GE 13B, POKEI	NO			1		-	Sheet 1	
Job Number:	J00113			Vane H 30		Logge F	d By: RZ	Process PL		: 6.10.21
Borehole mN	mE Gro	ound R.L.			Ê	ng evel	⊃a) tual	ity	Sampl	e and
Location: Description	Refer to site plan			Legend	Depth (m)	Standing Water Level	Vane Shear(kPa) <sub>peak/ residual</sub>	Soil Sensitivity	Laborator	y / Other
	SOIL DESCRIPTION			د 	ă	Na Si	Sh. Pea	Se	Det	
<ul> <li>[FILL]</li> <li>becoming low plasticity,</li> </ul>	orown. Medium dense, moist, low p		n plasticity		- 0.0 		UTP			
<ul> <li>insensitive [FILL]</li> <li>becoming brown, with training</li> </ul>	e mottled brown, low plasticity, with		m plasticity		- 0.0 - 0.5 - 1.0 - 1.5 		187/141 201+	1.3		
LANDER geotechnical	Comments: Groundwater not encountered. (unless noted) UTP = unable to penetrate. EOB = end of borehole.	Borehole Diameter: 50mm Checked: RZ	Topsoil Fill Clay	Gi	and ravel ganic		Sandstone Siltstone Limestone Volcanic		No Core	



# Appendix 5

**Construction Observation Records** 

# 09/10/19 - Stage 5 begins - Fill E/F

Wednesday, 9 October 2019 10:52 AM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	09/10/19 - 10:30am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to inspect gully muckout for Fill E and F. Gully stripped to generally inorganic stiff ground.

Small area of organics observed and contractor will chase this out.

Contractor also to install nominal toe key. Discussed with Trevor that this should be a buckets width wide at base and should extend across the face of the gully. Trevor to send photos once this is complete.







Photos from Trevor (10/10/19):







### Photos from Trevor (14/10/19):





# 16/10/19 - Fill E/F

Wednesday, 16 October 2019 12:13 PM

# **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	16/10/19 - 11AM
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

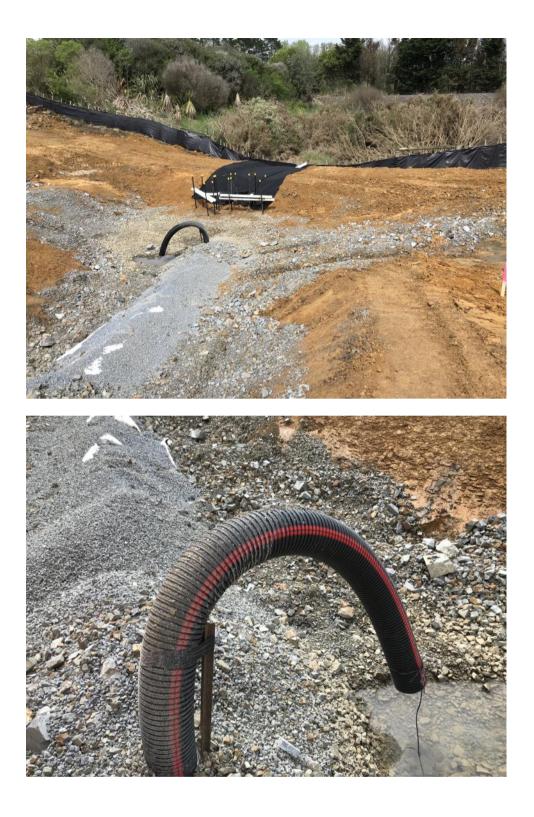
Visited site with Chris and met with Trevor and Dillan (who will be replacing Trevor soon).

Discussed likely outcomes for this season and next at Hitchen Stage 5 and also discussed toe key requirements - these being nominal keys 4-5m wide at base and keyed into the sides of the gullies.

Will return to site tomorrow morning to inspect remediated toe key in fill F which is being widened to meet the 5m width criteria.







# **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	17/10/19 11:30PM
Author:	KM
Plant Operating:	1x 20T excavator
Weather:	Showers

#### Site Observations and Instructions:

Visited site to inspect excavation of toe key in Fill F. Key excavated 5m at base with very stiff material at base as per our requirements. Some water ponding at base which will require pumping prior to filling.





21/10/19 Monday, 21 October 2019 10:17 AM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	21/10/19 - 8:30AM
Author:	КМ
Plant Operating:	Moxys and excavators
Weather:	

#### Site Observations and Instructions:

Visited site to check earthworks operations in Fill E/F areas. Toe key has been filled up and tesed since last visit and contractors are beginning to form permanent pond wall.









# 24/10/19 - Fill K

Thursday, 24 October 2019 3:02 PM

# **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	24/10/19 -
Author:	
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at request of Dylan (Dines) to inspect stripping in area of Fill K / Cut 2. Only a small area within the area of the silt pond had been stripped back to very stiff natural ground. The gully for Fill K had no been mucked out yet, and no excavations to form the silt pond had been started either.

Discussed with Dylan that we should return to site once the gully is mucked out and we can carry out a shallow trial pit in the area of the proposed toe key prior to excavations.









Tuesday, 29 October 2019 10:13 AM

### **Site Inspection Record**

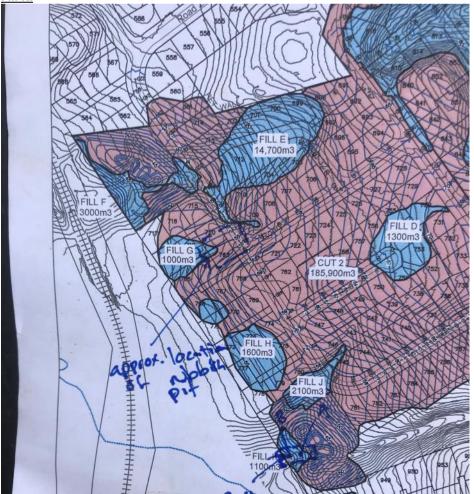
Project # & Name:	J00113 - Hitchen Block
Date & Time:	29/10/19 - 10am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

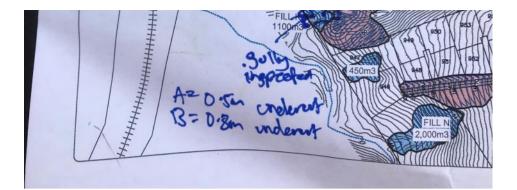
Visited site to inspect Fill K gully muckout. Inorganic, natural soils exposed throughout, however, soils were firm along a large part of the gully invert. Drilled two shallow hand augers which revealed stiff natural ground approximately 0.5m to 0.8m below current invert levels and we recommended an undercut to this depth. Refer plan attached.

Discussed the gully undercut with Dylan on site and he requested we do the undercut while we were on site to save time. See photos showing undercut of between approximately 0.5m and 1.2m.

Also found an old rubbish pit exposed which is marked out by the contractors. This will require undercutting also.



Fill K:











Rubbish pit:





Fill F walkover:





Fill K undercut/pit photos:



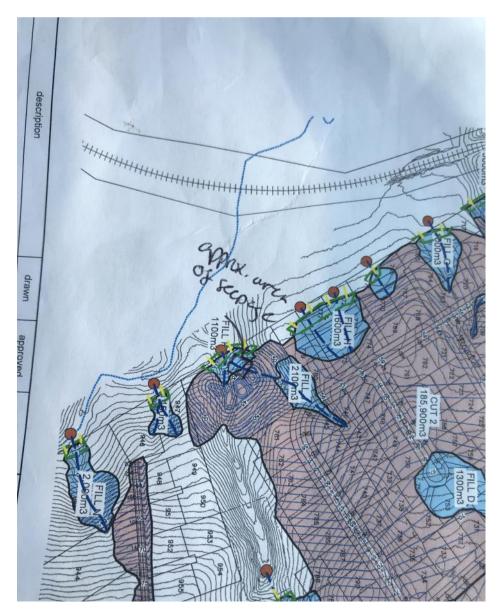


Project # & Name:	J00113 - Hitchen Block
Date & Time:	30/10/19 - 12pm
Author:	KM
Plant Operating:	
Weather:	

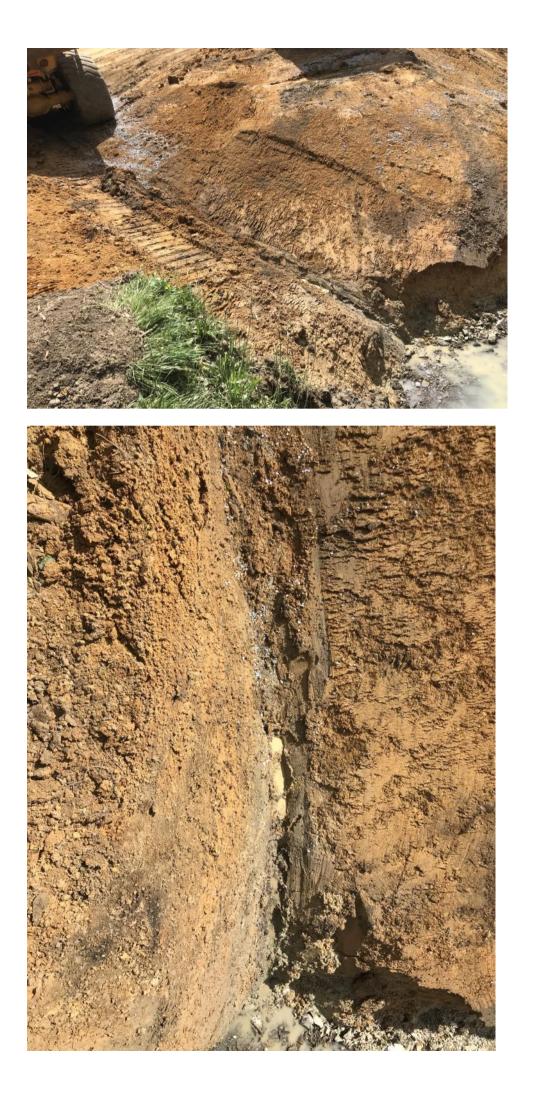
### Site Observations and Instructions:

Visited site to observe gully muckout and drain installation. Muckout was deeper than our previous recommendation due to a 1m clay cap needed below the pond.

There is a small area of seepage near the base of the gully muckout which showed some seepage. We recommended that an extra drain is installed here to connect into the main underfill, ensure a 1m clay cap beyond the pond is satisfied. We will prepare a CAN detailing this.











# 01/11/19 - Fill A gully

Friday, 1 November 2019 10:12 AM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	01/11/19 - 9:30am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at request of Dylan to provide guidance on REB shear key undercut requirements. I was not made aware that I would be looking at this before arriving on site.

Asked contractors/surveys to peg the far edge of the shear key alignment and also the silt pond so we can dig some trial pits next week. I will discuss a time to come to site with Chris next week.

Project # & Name:	J00113 - Hitchen Block
Date & Time:	05/11/19 - 10am
Author:	KM
Plant Operating:	1x 20T excavator
Weather:	Fine

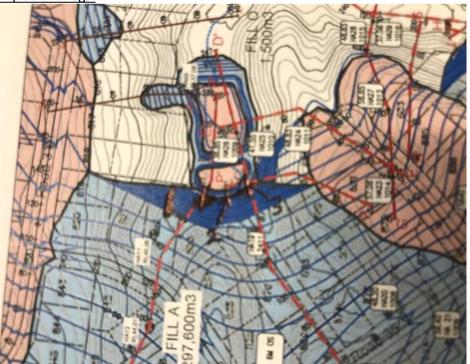
### Site Observations and Instructions:

Visited site with Chris to carry out a series of trial pits at the toe of the REB1 fill batter. Our general logs are shown below, with hard natural soils (UTP) generally being encountered at around 3m to 3.5m depth below existing ground level.

The contractors will be continuing to excavate the first section of the gully undercut / installing drainage, and we will return to site at a later stage to prove the hard ground encountered (as our pits ended at the depths above due to digger reach).

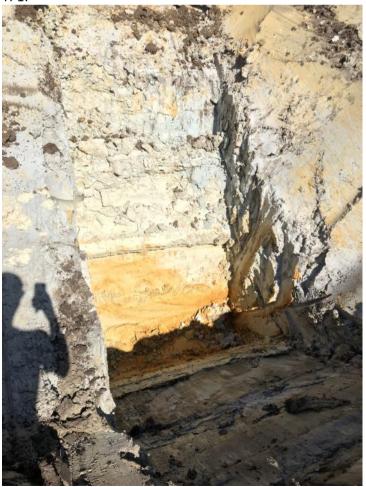
While on site we also visited the pond area in Fill K. There are groundwater seepages, however, are are at the base of the pond. Advised the contractors they should continue preparing the pond, with the likely outcome being a underfill drain connecting into the primary collector drain.

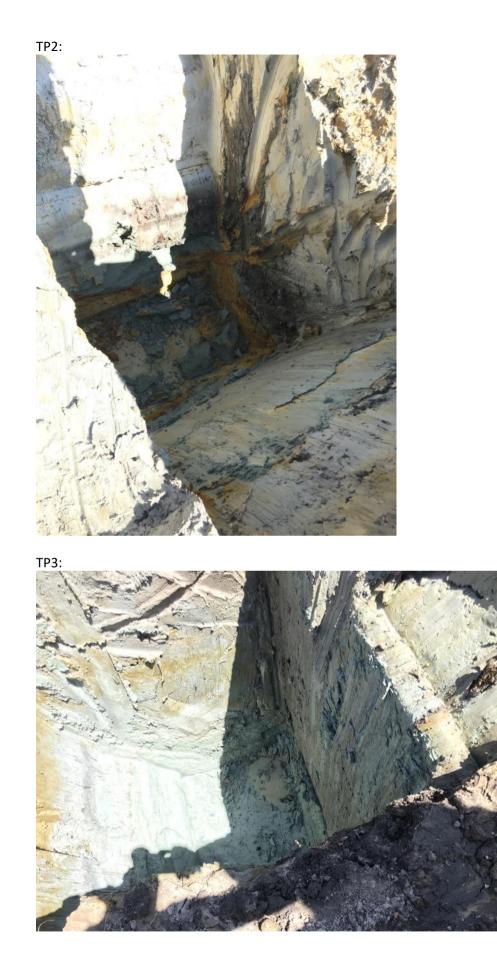
#### **REB plan and logs:**



TPI ~ GL FL = 38.85. -> Base approx - 3m. (RL 35:56?) -> avange sad manhad @ -3m approx. with - No grand water TP2 - 0 GL RL = 38.1 -D Base approx - 3.2m -> or nge sid encaled Q -2.8 m aprox UTP - Gw seapzye @ In TP3 -> GL RL = 37 - Base approx - 3m -D Grey/Blue smaly laye with in base -Duw sellinge Colon & Zun

TP1:







Fill K photos:





# 07/11/19 - Fill K and Fill A

Thursday, 7 November 2019 2:27 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	07/11/19 - 11:30am
Author:	КМ
Plant Operating:	
Weather:	

#### **Site Observations and Instructions:**

Visited site at request of Dylan to carry out an inspection of the toe key in Fill K and also to conduct further trial pits below REB1 (to provide the hard layer below the shear key).

Fill K had been excavated approx. 3m depth and shear vanes in excess of 100kPa were recorded at the base of the key. The key is still to be excavated further, however, the lower level in the photos below (which is the be the final key depth) had sufficiently removed all gully mullock.

Carried out three trial pits within the area of the shear key and one within the pond area. The ground had been brought down approximately 2m from our previous visit when the initial trial pits were carried out. The additional pits found the hard layer identified during the previous visit at approximately 1m below the top of the pit, and this was proven for approximately 3m beyond this depth (the excavator was struggling to remove the material at the base of the pit with a rock bucket).

Within the pond area the contractors had already excavated about 2m - 2.5m from original ground level, exposing stiff ground at this level. The hard layer identified in the previous trial pits was identified approximately 2m from this level.



Fill K:







REB 1 area:



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Pit 2:













Project # & Name:	J00113 - Hitchen Block
Date & Time:	12/11/19 - 2:30PM
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site with Chris and Shane to discuss construction methodology at REB1 shear key (involving subgrade depth of shear key and requirements for bench and drainage of key, especially into the ridge/flanks of the key).

There is still some work for Dines to do to facilitate access into this area. We will return to site once a section of the shear key has been opened to approve the subgrade.

# 15/11/19 - REB1 pond wall

Friday, 15 November 2019 4:29 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	15/11/19 - 4pm
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to inspect toe key embedment for pond wall below permanent pond at REB1.

Key had been excavated 1m into very stiff to hard ground (>170kPa).

Recommended hardfill be placed until above wet ground (say 0.5m to 1m) ensuring that as much clay is placed as possible to ensure a good lining around the pond. As told contractors that cleggs need to be carried out on top of the hardfill prior to compaction with clay.

Sides of batter are also sub-vertical. Recommended these be battered to a safe angle to provide a safe working area below.







Project # & Name:	J00113 - Hitchen Block
Date & Time:	20/11/19 - 2pm
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at request of Dylan to inspect pond wall key subgrade. Hard natural ground observed throughout and keyed minimum 1m into the hard ground.

Previous subgrade inspected had been backfilled with SPR.

Also discussed batter with contractors. They will be keying in with fill soon, however, they were advised to batter back / cover with plastic if it is be left open for much longer.

Ponds have been formed in Fill F / K areas. In Fill E the contractors are laying out the clay to dry.









# 22/11/19 - REB1 pond base

Friday, 22 November 2019 10:19 AM

## Site Inspection Record

Project # & Name:	J00113 - Hitchen Block
Date & Time:	22/11/19 - 10am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at request of Dylan to inspect base of pond which will be filled approx. 0.5m to 1m to get to design level.

Ground had softened at the base of the pond due to water seepages. Recommended that a nominal undercut of say 0.2m to 0.5m be carried out (based on shallow hand augers drilled), ensuring the area remains drained (i.e. leave the pumps on while filling) to ensure the subgrade stays dry. We will need to inspect the final subgrade and so recommended this should be carried out in small stages with filling operations commencing immediately after we have seen the subgrade.

Also brought up the idea of laying some trenches to catch water seepages. I will discuss this with Chris later and prepare a CAN outlining our recommendations.



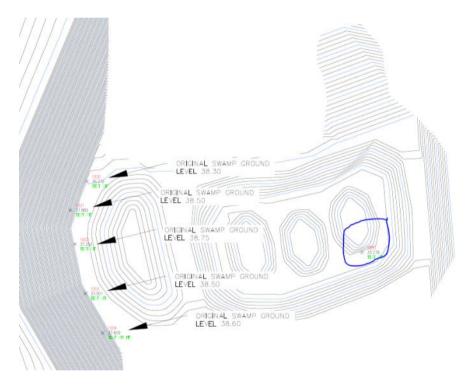




Project # & Name:	J00113 - Hitchen Block
Date & Time:	26/11/19 - 2:30pm
Author:	KM
Plant Operating:	
Weather:	

## Site Observations and Instructions:

Visited site to inspect first area of pond undercut (cut 4) as shown in blue on the plan below. Stiff natural ground in excess of 60kPa throughout. There is a small area of water ponding in the area of deepest cut and we recommended that that be pumped out / dug out prior to placing fill.



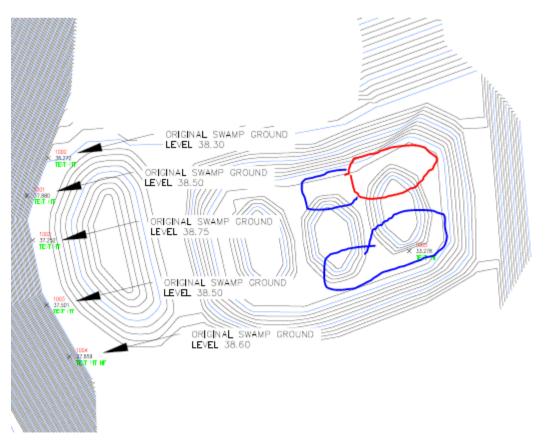


Project # & Name:	J00113 - Hitchen Block
Date & Time:	27/11/19 - 2:30pm
Author:	КМ
Plant Operating:	2x20T excavators, 4x30T moxys
Weather:	

### Site Observations and Instructions:

Visited site at request of Dylan to inspect next section of pond base subgrade to fill up to pond invert. Stiff to very stiff, inorganic natural ground exposed throughout (blue). The hardfill track still remains to be removed and we will look at this once they remove this. The area in red was not fully stripped yet, however, several areas were exposed with ground being consistent throughout. Asked contractor to send photos once complete so they commence filling to stabilise a small slip which had formed in the side of the bank.

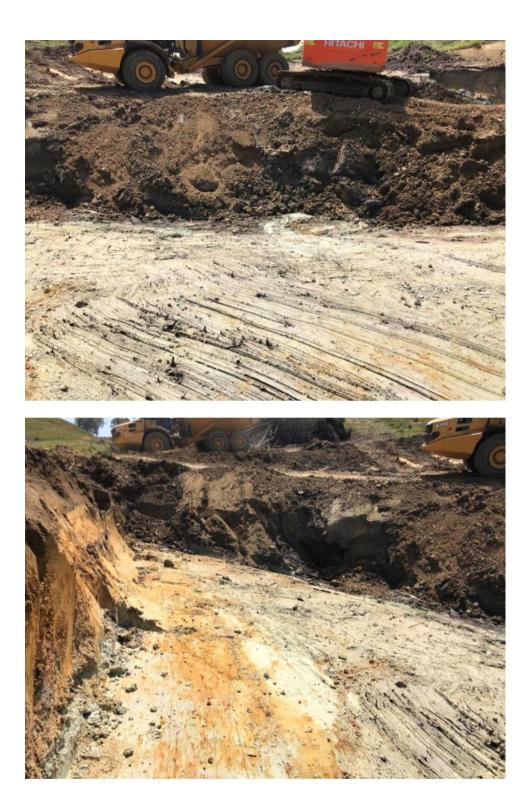
Also had a look at fill G and Fill E. Fill G and surrounding areas of Cut 2 stripped back to competent natural ground. Fill E near design level. This area currently being used to dry out the wet materials coming from cut 2 pond.















Photos from Dylan 29/11/19:





Project # & Name:	J00113 - Hitchen Block
Date & Time:	29/11/19 - 12PM
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at request of Dylan to inspect next section of pond base fill subgrade. Very stiff ground throughout. Some areas had softened where water is ponding and we recommended that these areas be undercut to remove the softer soils. We have now seen almost all of the pond base. There is only a small area and Dylan will send photos once this is down to subgrade level.

Pond wall has been filled to near final level.





Photos from Dylan 2/12/19:



03/12/19 Tuesday, 3 December 2019 12:20 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	03/12/19 - 11pm
Author:	КМ
Plant Operating:	
Weather:	

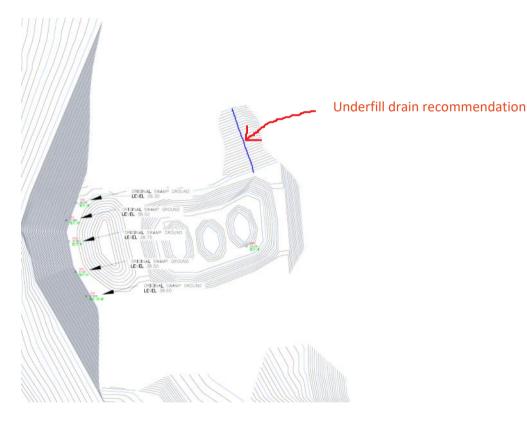
#### Site Observations and Instructions:

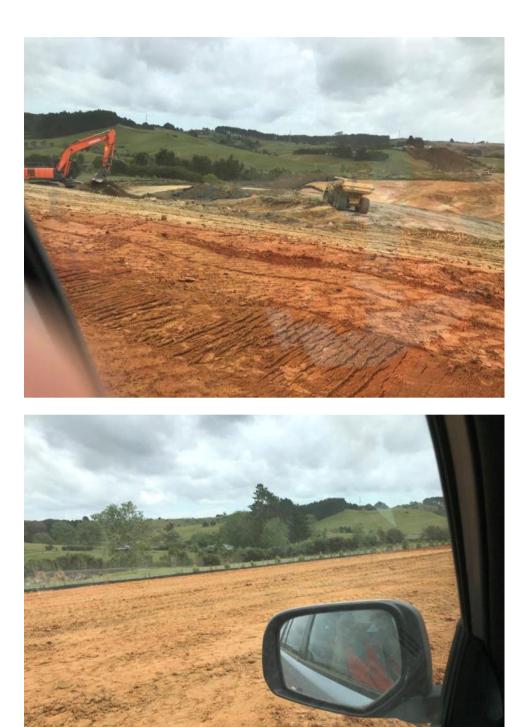
Visited site with Chris to see what is happening on site.

Observed start and outlet of underfill drain which outlets below Fill K.

Observed Cut 4 pond. This area has been filled with clear and is now near pond base invert level. While down here discussed a proposed fill batter which we recommended has an underfill drain installed to tap any groundwater pressures (area shown on the plan below).

Contractors also removing fill from an old rubbish pit found on site at the start of the season.

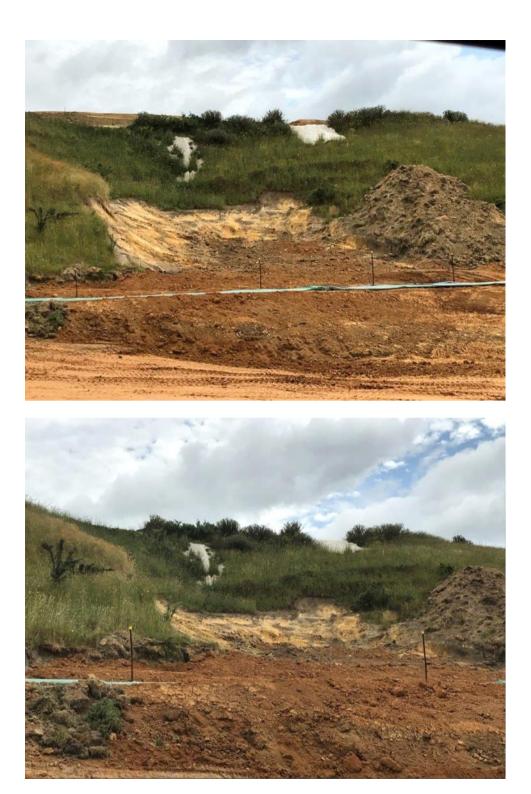














# 09/12/19 - Fill A

Monday, 9 December 2019 3:16 PM

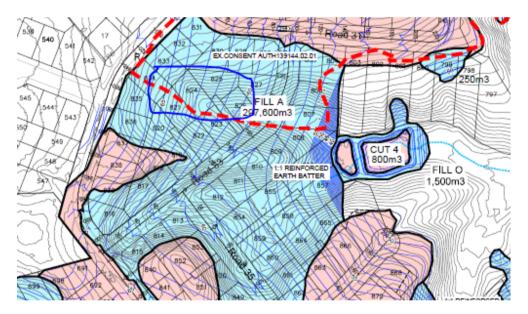
### **Site Inspection Record**

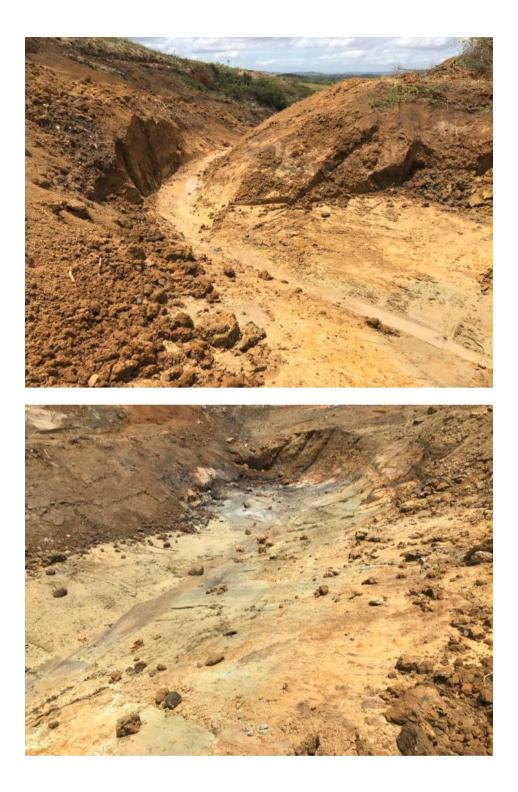
Project # & Name:	J00113 - Hitchen Block
Date & Time:	09/12/19 - 1:30pm
Author:	КМ
Plant Operating:	
Weather:	

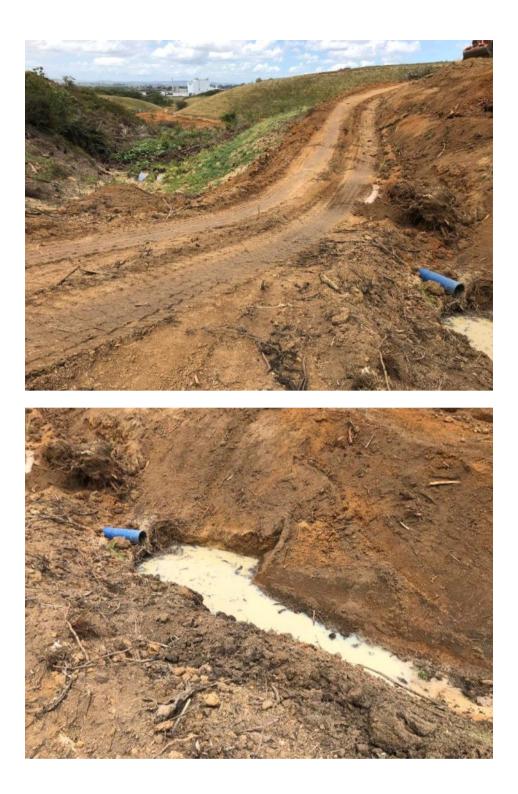
#### Site Observations and Instructions:

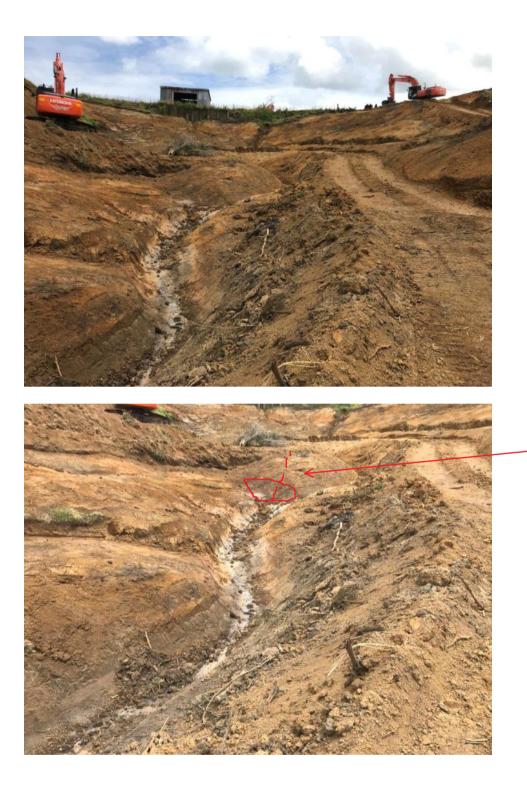
Visited site at the request of Dylan to inspect gully stripping in the area outlined on the plan below (Fill A).

Generally stiff ground was exposed throughout, however, at the base of the gully invert (lower half) water seepages were causes the ground to soften. We recommend an undercut of approx. 0.5m to 1m here prior to placement of the underfill drain. There is some material in place which is going to be undercut as there is seepage coming out below and the underfill drain will be aligned through here.

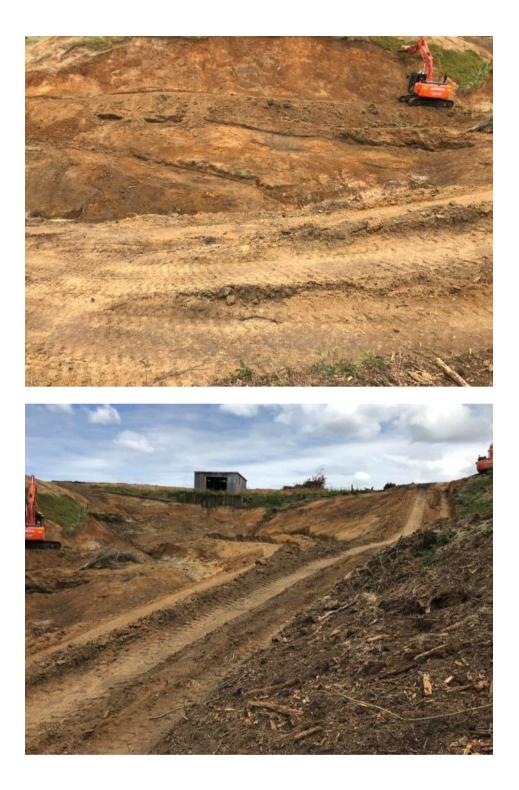








Groundwater seepage



Project # & Name:	J00113 - Hitchen Block
Date & Time:	10/12/19 - 12pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect muckout of lower portion of gully as per yesterdays recommendation. Generally stiff soils exposed at the base of the gully.







Monday, 6 January 2020 10:55 AM

06/01/20

### **Site Inspection Record**

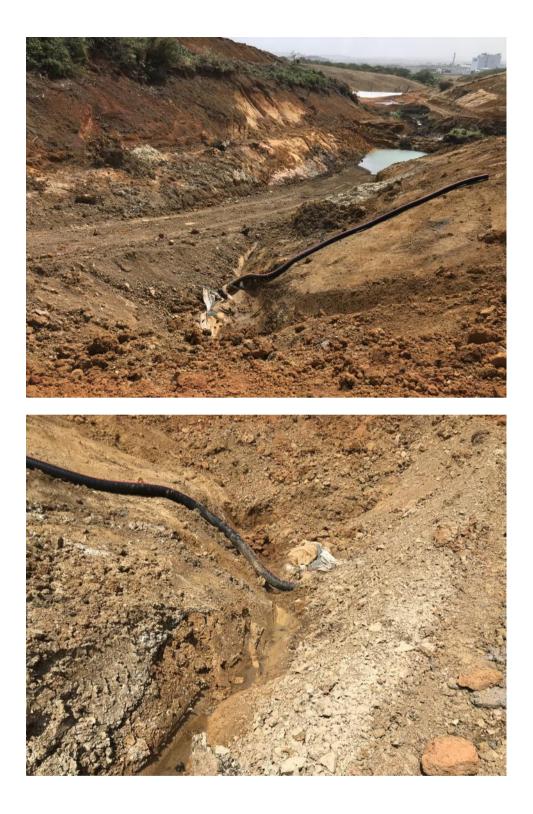
Project # & Name:	J00113 - Hitchen Block
Date & Time:	06/01/20 - 10:30am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to catch up on site works within the REB1 fill area. Contractors have filled up the areas sighted prior to the Christmas break. They will send through photos showing the removal of soft soils within the gully inverts.

Contractors plan to extend the underfill drainage so an inspection later this week is likely.







Friday, 10 January 2020 2:12 PM

10/02/20

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	10/01/20 - 2pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect gully muckout in Fill A. Stiff to hard natural ground observed throughout.





13/01/20 Monday, 13 January 2020 3:28 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	13/01/20
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site witch Chris to observe general site earthworks.

Underfill drain has been placed within Fill A gully observed during previous inspection.

Elsewhere, the site is mostly in the same condition as our previous inspections.





Project # & Name:	J00113 - Hitchen Block
Date & Time:	15/01/20 - 8:45am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to catch up around the shear key in Fill A. Dylan was asking about drainage / outlet requirements. Confirmed that that underfill drains are to run beneath the MSE fill and the shear key.

**21/01/20** Tuesday, 21 January 2020 2:14 PM

# **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	21/01/20 - 1:30pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited old slip site with Dylan and discussed benching the slip debris out.



Project # & Name:	J00113 - Hitchen Block
Date & Time:	22/01/20 - 2pm
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site with Chris to meet with Dylan to discuss remediation of the slip that we inspected yesterday.

Advised Dylan that the slip should be fully benched out to the natural ground (i.e. removing all the slip debris) at which point we can observe and provide further recommendations. This will likely be to recommend a series of benched drains running parallel along the slope with one or two collector drains running along the extend of the slip area. We also advised that the lateral drains be installed within the vicinity of the existing chimney drain as per our design (this was never done during the earlier earthworks stage).

Project # & Name:	J00113 - Hitchen Block
Date & Time:	22/01/20 - 2pm
Author:	CE + KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

CE - photos from my visit below. They appear to be progressing with REB1 pre construction work (i.e. filling in gullies upslope of it and remediating slip on site of Stage 4 which will be filled as part of REB1.



REB1 area







REB1





Monday, 3 February 2020 3:25 PM

03/02/20

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	03/02/20 - 3pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect first portion of shear key excavation for REB1 / Fill A. Shear key excavated to 5m depth (which involved using a rock ripper). Advised contractors to excavate at least 0.5m into hard or until it can't be removed with a rock bucket. Also advised a plateau ready be carried out in the SPR when fill compaction commences.

Wednesday, 5 February 2020 2:39 PM

05/02/20

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	06/02/20
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Inspected next section of shear key undercut. Hard materials exposed at base as per previous sighting.









12/02/20

Wednesday, 12 February 2020 1:51 PM

# Site Inspection Record

Project # & Name:	J00113 - Hitchen Block
Date & Time:	12/02/20 - 1:30pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Shear key excavations proving hard.





19/02/20

Wednesday, 19 February 2020 2:19 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	19/02/20 - 1pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Shear key excavations completed. Competent hard ground exposed throughout. Plateau test being undertaken while we were on site.









26/02/20

Wednesday, 26 February 2020 10:29 AM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	26/02/20 - 10am
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect top two benches into the remediated slip adjacent to REB1. Benches have been excavated into the natural ground and the contractors have further a small trench along the rear face to installed drainage. Attached below is their proposed plan.

Shear key filling still coming up. Photo below.

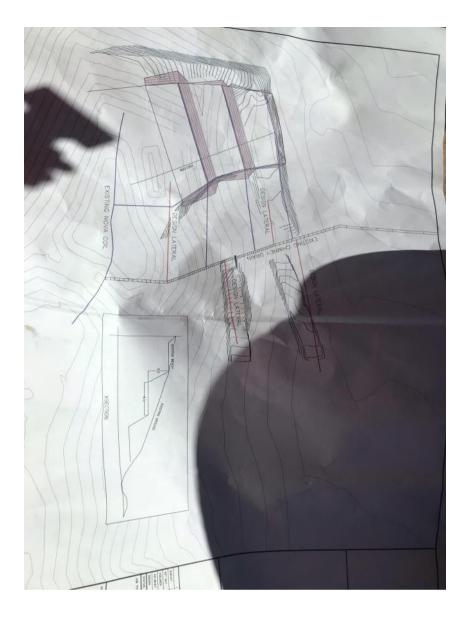












Thursday, 27 February 2020 10:29 AM

#### Site Inspection Record

Project # & Name:	J00113 - Hitchen Block
	27/02/20 - 10:30am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Slip remediation works are continuing and they are down to the final bench. Drainage has been installed on the top two benches, with a collector also placed down to the second bench. The material on the bottom bench is a stiff, wet alluvium. Recommended to Dylan that the batter faces be supported by backfilling or temporary face support and also be covered with polythene if they are to remain open.











Project # & Name:	J00113 - Hitchen Block
Date & Time:	06/03/20 - 11am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site to check in on shear key. The contractors are excavating the 2m beyond the fill batter now. Will return to site with Chris early next week to REB requirements.

Also checked outlets for Hitchen 11, 12, 14 GCR. All outlets correctly formed with concrete rip rap.

















Project # & Name:	J00113 - Hitchen Block
Date & Time:	10/03/20 - 2:30pm
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site with Chris to discuss REB1 backfill, benching and grid requirements with Dylan, Mitchell and Miles. Discussed that all benches need to be at least 2m below original ground level to key in the fill batter

Project # & Name:	J00113 - Hitchen Block
Date & Time:	16/03/20 - 10am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited sure at the request of Dylan to inspect first gully muckout for Fill B. Generally stiff ground is exposed at the base of the gully. The contractor was advised to strip wet/softer ground where encountered.

Also discussed installing another underfill drain up the gully branches as there has been some groundwater seepage / slippage observed which the contractor has cleared out prior to our visit. Advised we can look at this which stripping operations commence in this area.









	SITE INSPECTION RECOR	D
	Project no. and name: Jeol13 Hitchen Block	Date & Time: 29.4.20
	Client/contractors present: Dylan (Dines)	Lander staff: 1cm SV no: 502396
	Plant operating: 12207 TXLCUDER	
	Inspection for ( <i>tick all that apply</i> ): Shallow Foundations  Pile Foundations  Site Mee Subgrade  Other	
	Observations/Notes:	
	-Visited site at the request of Rylan to inspect gully muchaut for REB 2 undefill obtain.	ter key beching
	-Generally shift (hother) Soils exposed at bese of sully. Freesance AZ soils.	the terre
	- Contrain also eduised that seeper groundwater Seeperges were also noted in source place Coming from the sully flooks.	
	Fully wrepped underfill drea a beneath engineer	
		colled a
		1 6
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		H.
	Discussions/Recommendations given:	
-	Preasumendy some of the softer / himer sols	be shipped beels
	-Other price to please undefill drain. Also recommended that localised seerings b collecter undefill drains where observal main undefill guilty altrin.	and compacted into
	main water in group of the	
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13/03/20

Wednesday, 13 May 2020 12:25 PM

# **Site Inspection Record**

Project # & Name: J00113 - Hitchen Block

Project no. and name: Joza13 - Hitchen Black	Date & Time: 13/5/20
Client/contractors present:	Lander staff: Km
Dylan (Dires)	SV no:
Plant operating:	
Inspection for (tick all that apply): Shallow Foundations  Pile Foundation Subgrade  Retaining Wall	ns Site Meeting Other <u>FEB1</u> Shear Key
Observations/Notes:	
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	he contact has been adviced to
overlap the grid by a m	innorm of 200mm and 10kg.
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I they don't want to ve	











15/05/20 Friday, 15 May 2020 4:26 PM

## **Site Inspection Record**

Project no. and name:	Date & Time:
200113 - Hitchen Block Client/contractors present:	15/5/20 - 2:30pm Lander staff: 2m SV no:
Plant operating:	
Inspection for ( <i>tick all that apply</i> ): Shallow Foundations  Pile Foundations  Subgrade  Retaining Wall	Site Meeting Other REB 2 Sheer Key
Observations/Notes: - Virted site to observe site e Competitue of the REBI Shear lea - Lipk underfill drains fully wrepped drainage assingit sighted at ba faces de the shear leag. - Converty rollers are competing GA first Im of the VZEVB bette. - Will refurn to site with CE geogrid placement.	5. I and bockfilled with oth the fort and rea 1965, which comprises the

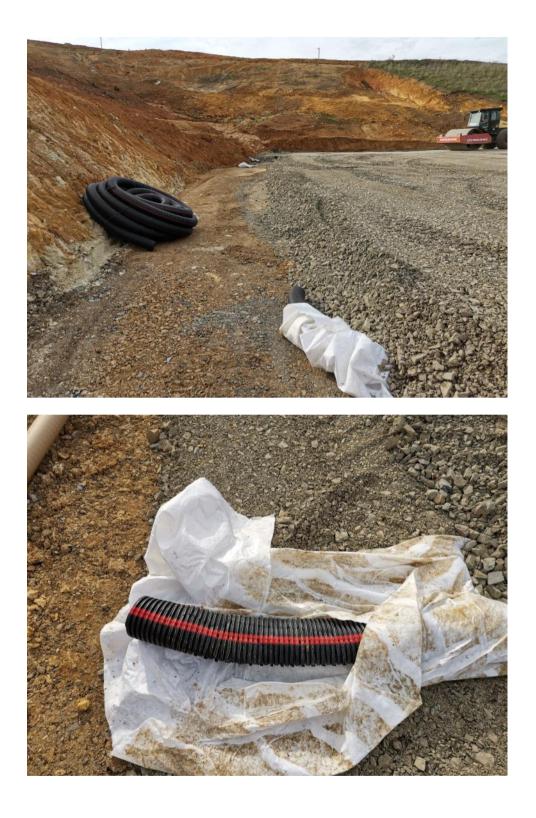














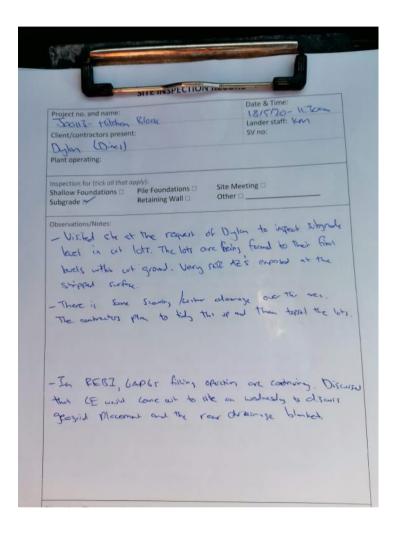




18/05/20 Monday, 18 May 2020 3:31 PM

## **Site Inspection Record**

Project # & Name: J00113 - Hitchen Block





















p. i





21/05/20

Thursday, 21 May 2020 9:59 AM

## **Site Inspection Record**

Project # & Name: J00113 - Hitchen Block

## **Site Inspection Record**

Project:	J00463 - 125B Murphys Rd
Date & Time:	21/05/20, 9am
Author:	KM / CE
Plant Operating:	
Weather:	Fine

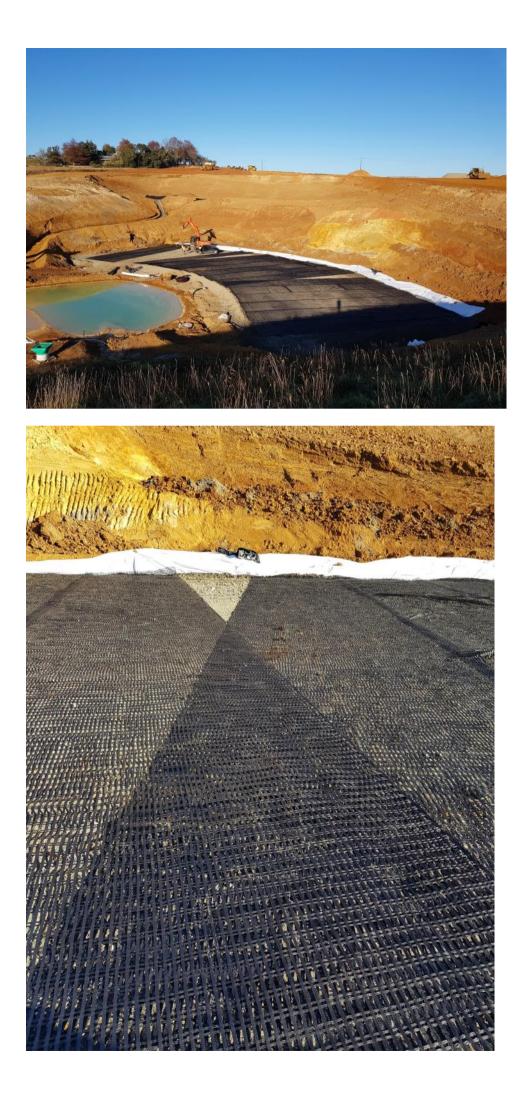
#### Site Observations and Instructions:

Meeting with Dylan to observe second layer of primary grid on REB1. They are having issues with grid coverage as the wall goes round the curve. Have queried this with Michael Sorrenson and will get him to comment on the best way to fix this issue. Meeting with Cirtex set up for tomorrow morning to discuss this, no more grid to be placed until issue resolved.









## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	22/05/20 9am
Author:	CE/ KM
Plant Operating:	
Weather:	Fine

#### Site Observations and Instructions:

Meet with Dylan and Cirtex (Paul Lombard) to discuss placement of geogrid for REB 1

The contractor had re-laid the primary grid layer.

Discussed the requirements for the grid as follows:

- 1. On straight sections of slope maintain a 200mm to 300mm overlap
- 2. On curved sections, increase the overlap at the face to up to 1m and minimise the 'wedge' of area at the rear of the reinforced section where there is no grid covering to less than 1m
- 3. On the next lift of grid (800mm) the areas where 'wedges' were are to be covered. A CAN to be sent on this matter.



4.

22/05/20 Friday, 22 May 2020 11:28 AM

Project # & Name: J00113 - Hitchen Block

### **Site Inspection Record**

Joons - Hitchen Plack 22/05/20 - 8:30cm KMICE - Visited site with CE and met with Paul of Circle, and Dylan and state from Dines to discuss geogrid placement in the REBI better - Paul drivered that the grid needed to be perpeolide to the face and that the gost between grids at the rear of the sharkey could be up to In making with my of these gass being suncycl and the caracters ennous, that these gaps are torened during the next tight by of primary goid - In coursel Dections, the contractors will be within helf lengths of grid (pureller to the thick should) to try ad reduce the gop and minimic any wastege of grid.

27/05/20 Wednesday, 27 May 2020 12:51 p.m.

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	27/05/20
Author:	CE
Plant Operating:	
Weather:	Showers

#### Site Observations and Instructions:

Stopped by to observe construction of REB1. See photos below. Grid layer appears to have been placed as per our directions last week and is now being overlaid with hardfill.



24/06/20

Wednesday, 24 June 2020 11:39 a.m.

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	24/06/20 10am
Author:	CE
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Quick stop by to see progress of REB1. Looks like they have stopped due to the weather.



## 30/07/20 - Stage 12D

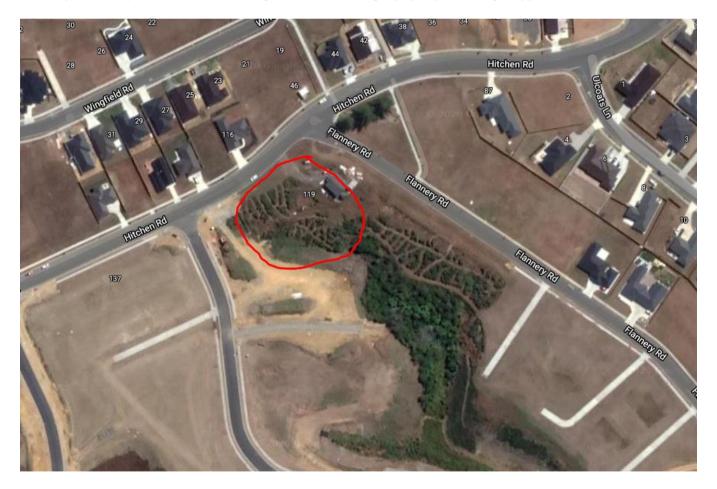
Thursday, 30 July 2020 3:07 p.m.

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	30/07/20
Author:	тт
Plant Operating:	
Weather:	Fine

#### Site Observations and Instructions:

Observed muck out of portion of gully circled below. Decant to be formed here to allow further earthworks up slope within gully. Muck out looked suitable with benches to be formed 2 compactors width in flanks to lock this fill batter in. Batter only temporary until next season when earthworks occur downslope. Underfill drain to be placed through this area (discussed specs which are consistent with other drains across site) that will lead into the earthworks upslope and eventually downslope as well. Fill materials to cut from cut area above which looked fine to use. Testing to take place every 1m lift. Contractor to get in touch when gully upslope is being stripped.













# 04/08/20 - Stage 12D

Tuesday, 4 August 2020 8:15 AM

## **Site Inspection Record**

Project # & Name: J00113 - Hitchen Block

#### Site Observations and Instructions:

JOOUNS - H.F.		4-08-20
Client/contractors pres	sent:	Lander staff: Km SV no:
Plant operating:		
12 excivity, 1x		
Inspection for (tick all th Shallow Foundations T Subgrade	nat apply): <ul> <li>Pile Foundations</li> <li>Retaining Wall</li> </ul>	Site Meeting  Other
Observations/Discussion	ns:	
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Inspection Result:	Pass Tail	



## 08/09/20 - REB1

Tuesday, 8 September 2020 11:28 AM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	08/09/20 - 11:15am
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

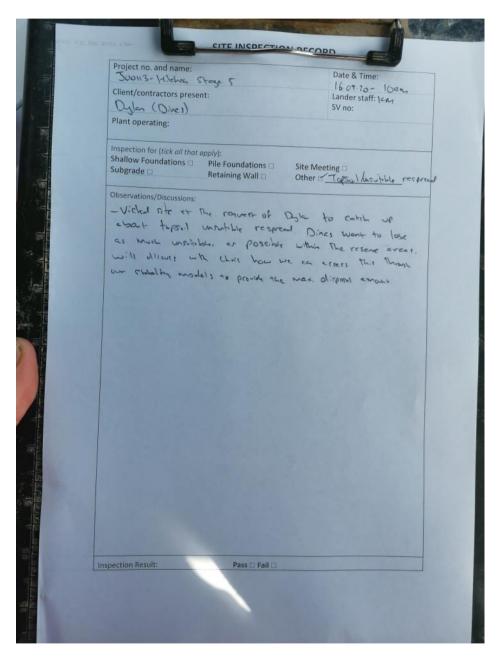




Jouris - Hitchen stoges 08.09, 20 - 11:15cm Km -Visited site to catch up on earthwardy at Hitchen Black - Causht of ut Trever and discussed earthwarter. Dires have recently started Pleaks fill in RER I, with terms ongoing with bestechnick - Converting enabling works are underway to extend your free of better to allow a compadu to fit against the real balls liese. - West loger of grid is planned for tomorrow and we cull referre to

## **Site Inspection Record**

## Project # & Name: J00113 - Hitchen Block



# **Site Inspection Record**

Project # & Name: J00113 - Hitchen Block

Droite	
Project no. and name: Doo113- Hitchan Black	Date & Time: 7.10.20
Client/contractors present:	Lander staff: Kint LE SV no:
Plant operating:	
Inspection for ( <i>tick all that apply</i> ): Shallow Foundations Dile Foundations Subgrade Retaining Wall	Site Meeting
Observations/Discussions: -Viriled Site with Chris to Alister From Dires.	mar with Dyle, withher and
- Discussed requirement for d REB better, with clay fill be the 2m toe key. No benched these will probably be installed	cing one to use for filling in
-Discussed requirement for fill Im 172 and also for ensures. to observe site worky shallong to	tering to be close every
Roads are amoney being G areas.	

















**19/10/20** Monday, 19 October 2020 9:39 AM

# **Site Inspection Record**

Project # & Name: J00113 - Hitchen Block

_		CITE HOI LC	HONTALCOI		
	Project no. and name: Dooll 3 - Hitchen	Block		Date & Time:	I GN
	Client/contractors preser	nt:		Lander staff: Y	~
	Plant operating:				
	Inspection for ( <i>tick all that</i> Shallow Foundations Subgrade	r apply): Pile Foundations Retaining Wall 🗆		ting Draining	_
	Observations/Discussions:				
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## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	23/10/20 - 9am
Author:	КМ
Plant Operating:	
Weather:	

## Site Observations and Instructions:

Visited site to observe earthworks operations in REB1. Clay fill is progressing and they had just covered up a layer of grid at the time of our inspection. Batter slope appears to be approx. 1(v) in 1(h) as per the design.









# 28/10/20 - REB1 / 12D

Wednesday, 28 October 2020 1:38 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	28/10/20 - 10am
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to observe earthworks in REB1. Contractors had finished fill placement ready for the next layer of primary geogrid placement. The fill batter is now entering zone 2 and SG500 grid is being used for these portions of the batter as per our design.

Rear drain blanket trench was open and the top of the blanket and metal backfill was present, as well as the rear of the grid in approx. 0.4m layers.

Earthworks are also commencing in the large slip area at the far end of the site. Silt controls are currently being formed for these works prior to stripping operations commencing.

In Stage 12D, earthworks filling operations are progressing, and the underfill drain is likely to be extended in the next week or two.























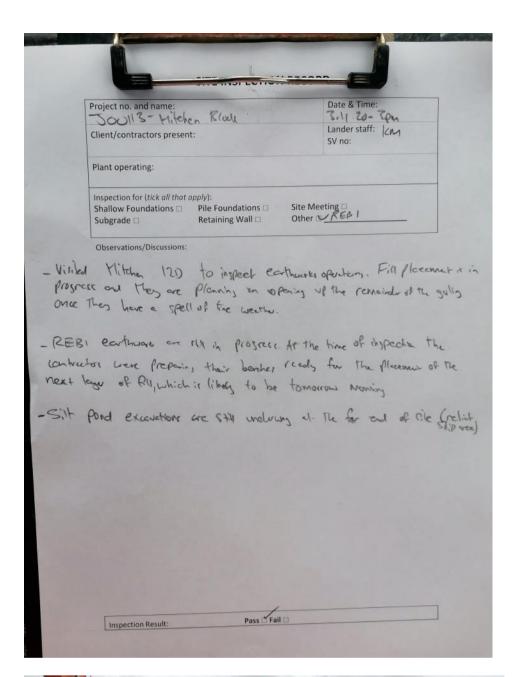
03/11/20 Tuesday, 3 November 2020 6:27 PM

# **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block









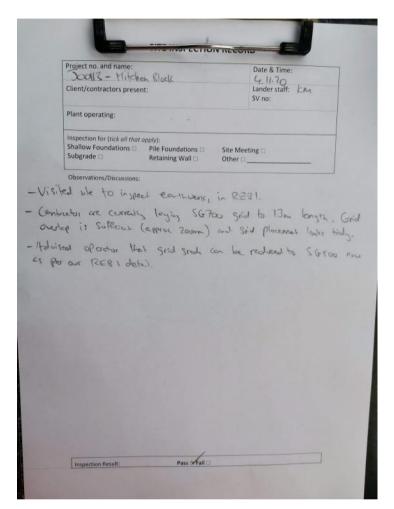


# 04/11/20 - REB1

Wednesday, 4 November 2020 2:10 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
r roject <i>n</i> & Nume.	JOOTTO THICHCHENDIOCK











13/11/20 Friday, 13 November 2020 1:04 p.m.

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	13/11/20, 1pm
Author:	CE
Plant Operating:	
Weather:	Fine

### Site Observations and Instructions:

Works have re-started on REB1 after the wet weather. Contractor was about to commence placement of another grid layer. Clay material still being dried out/ conditioned.





# 16/11/20 - AM

Monday, 16 November 2020 3:07 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	16/11/20 - 11:30am
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site to observe earthworks operations in REB1. The REB is now up to chainage 49.5 and a layer of intermediate grid had just been placed and covered by clay. The next bench was being prepared at the time of inspection. Contractor informed that the next grid would likely be placed tomorrow morning so will do another inspection of the grid then.

Was also informed that the lateral drain was trenched and installed mid-last week, for which we received the as-builts. Advised we should try inspect the next section of drainage installation if possible.







# 16/11/20 - PM - Stage 12D

Monday, 16 November 2020 3:19 PM

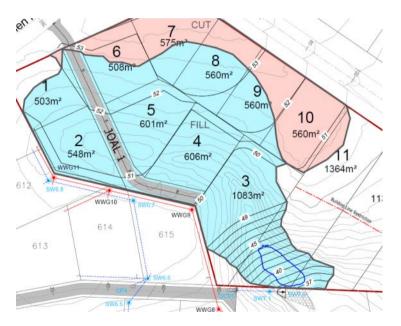
## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	16/11/20 - 1:30pm
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to observe the gully muckout within Stage 12D. The muckout is commencing from the lower portion of the gully and will tie in with the existing underfill drain once some engineered fill has been placed in the area to allow digger access. Approx. area sighted shown in blue annotation below. Will return to site tomorrow to observe progress here.

Also inspected the removal the stockpile materials within Fill D. Very stiff, orange/brown ash soils were exposed consistent with the stripped soils previously sighted within the adjacent cut areas.











### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	17/11/20 - 12:30pm
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site to observer general earthworks operations. Within Stage 12D the base of the gully had been mucked out slightly further upslope and the drain coil and bidim cloth were in place at the base of the muckout. SPR was being placed over the drain to be compacted.

Within REB1 clay fill was still being compacted. It appeared that the geogrid had already been placed.











# 18/11/20 - REB2

Wednesday, 18 November 2020 3:46 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	18/11/20 - 12:30pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect the undercut for the shear key in REB2. The contractors had cut down approx 4m from original ground level and had excavated down to hard orange silts.

Checked these works against our model which confirmed that the approximate depths and materials were consistent with our model. Informed Dylan that shear key operations could continue as planned, but 2 or 3 trial pits will be required in the base of the shear key once it has been opened up to further validate the ground conditions beneath the base of the shear key.









# 20/11/20 - REB1&2, Stage 12D

Friday, 20 November 2020 12:41 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	20/11/20 - 11am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

### Stage 10:

Visited site to inspect that underfill drain outlets have been installed. Both drains were sighted and have been formed as scruffy dome or rip-rap channel outlets.

### <u>REB2:</u>

Observed shear key operations in this gully. The shear key looked that same as our previous inpsection, and the gully is still being mucked out for the shear key. From what we can see the base appears hard, however, we waven't been able to test the base of the shear key yet due to the muckout still needing to progress.

Informed the contractors that we will inpsect once a larger section of the shear key has been opened up, and that they will need to create a level surface for the shear key and then bench up the sides of the gully when backfilling with hardfill.

### <u>REB1:</u>

Primary geogrid has been laid out to 13m length. Grid layout and overlap is sufficient. The rear fill drain was also being connected and has been sufficiently linked up.

### Stage 12D:

Not much progress has been made since our last inspection in this area. The SPR in the base of the gully is still loose and requires compaction. The contractor is planning to bring a pad-foot roller down to this area to start backfilling the gully.





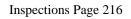














# 24/11/20 - REB2

Tuesday, 24 November 2020 3:53 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	24/11/20 - 12pm
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect earthworks operations in REB2 shear key.

The shear key has been opened up slightly more since our last inspection, however, more works are still required to enable proper access into the shear key. Once this has been done we will observe a series of trial pits along the base of the shear key to confirm the ground model.







# 30/11/20 - Stage 12D

Monday, 30 November 2020 12:58 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	30/11/20 - 9:45am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site to inspect earthworks operations in Stage 12D area.

Not much progress has been made since our previous inspection and there is spoil along the gully which is still to be cleared. Also discussed that a toe key is required at the base of the proposed fill batter, which the contractors are proposing to install prior to building the fill batter.

Also discussed the stripping in Lots 611 and 612 which Russell emailed about last week. This area is still covered by a topsoil stockpile. Advised the operator to get Dylan to get us in for an inspection prior to placing fill in this area.





### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	03/12/20 - 1:15pm
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Trevor to discuss toe key requirements for the Sage 12D fill batter. Discussed that the toe key needs to be embedded 0.5m into very stiff, 100kPa soils, and needs to extend along the face of the batter and be benched up the sides of the fill. Dines are going to use SPR in the base of the toe key and then use clay fill once above the water / underfill drain line. The toe key will likely be ready for us to inspect early next week.

The remained of the gully is still being stripped, and excess spoil is still placed along the gully waiting to be loaded out. There is a vertical cut along the gully flank which is located just below an existing dwelling. Advised this should be backfilled and supported in the short term to provide stability to the dwelling.

Filling has also occurred over lots 612 and 613 from the adjacent GCR stage. We will drill several 2m boreholes here to confirm adequate stripping. Asked Trevor to also organise a fill test in this area for inclusion in our addendum GCR letter.







### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	07/12/20 - 7:30am
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Trevor to determine extent of toe key in Stage 12D. Decided that the key should run along the toe of the batter, be 3m wide and at least 0.5m deep into very stiff natural soils. The key will be backfilled with SPR and the underfill drain will then run on top of the key (i.e. along the gully floor) and outlet into the gully below.

The gully has also been stripped and the underfill drain is to be placed next.

Dines has also placed some temporary fill in the cut that was open below the existing house.





## 08/12/20 - REB2, Stage 12D

Tuesday, 8 December 2020 2:05 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	08/12/20 - 8:45am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect the following:

### <u>REB2:</u>

The front/downslope part of the shear key has been excavated to depth, with hard, orange/brown silty soils exposed at the base. A trial pit was excavated with the rock bucket and went to a depth of approximately 2m before it became too hard to excavate further.

A small section of softened soils remains to be removed in part of the base.

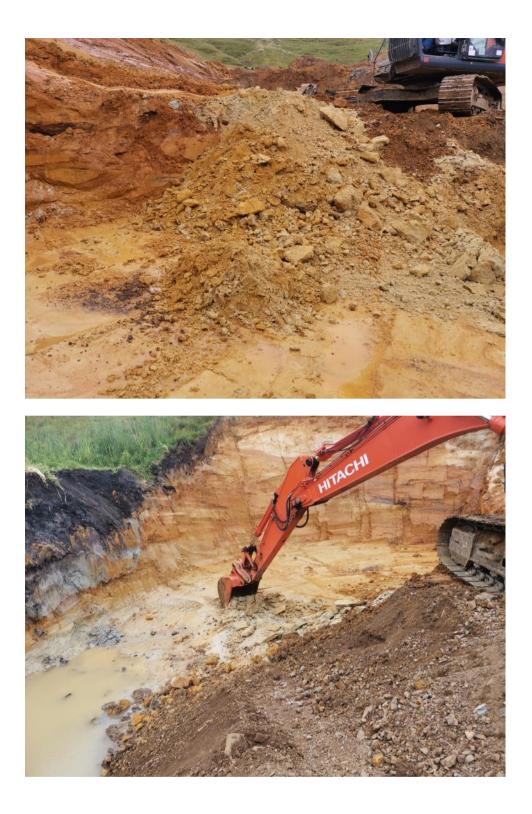
A section of the existing underfill drain behind the shear key has been flattened due to plant tracking over this area. Advised that this section of the underfill drain would need to be chased out and a new drain relayed here. The contractor also had concerns about the soils under the drain being soft, so we will look at this when the gully has been opened up.

### <u>12D:</u>

Toe key construction is still in progress. The contractors have excavated down approximately 3m and have encountered slop. Advised them the clean this out and continue to muck out the area to very stiff soils as per our detail.

REB2:







REB1 progress:





12D toe key:





Damaged underfill drain in REB2:





# 09/12/20 - REB1&2, Stage 12D

Wednesday, 9 December 2020 3:08 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	09/12/20 - 11am
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect the following:

- Inspected toe key in Stage 12D. Contractors have excavated to approx. 4m with no improvement in ground conditions. Advised that they could place a 500mm raft of compacted SPR to allow them to start backfilling.
- Inspected shear key excavations in REB2. Hard ground exposed throughout along the base of the key which had been further excavated since yesterday. Advised that backfilling can commence with SPR100 once they have pumped the water out of the excavation. Plateau test from REB1 will still remain relevant.

#### REB 2 shear key:









Stage 12D:



# 10/12/20 - Stage 12D

Thursday, 10 December 2020 12:45 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	10/12/20 - 10:30am
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site to check on toe key works in Stage 12D. The first section of the toe key has been backfilled with hardfill.

Also, drilled at hand auger in Lot 612. Encountered approx. 0.6m of fill overlying very stiff, natural ash soils.







## 14/12/20 - Welded tuff area

Monday, 14 December 2020 1:39 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	14/12/20
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at the request of Dylan to inspect an area of cut which the contractors are struggling to excavate. Dylan was concerned that the material may cause limitation for future end-users / builders. Dines were querying whether an undercut backfilled with certified clay would be a good solution.

On site, the material comprised of a welded tuff. Advised that I would talk to Chris or Shane before providing any advice.



Client : DFH JOINT VENTURE LIMITED Project Location : HITCHEN ROAD STAGE 5, POKENO				Machine Borehole No. MH01								
	Project Location	on: HITCHEN	RUAD STAGE	D, PUK	ENU	Vane Head:	ogged	nu D	00051		eet 2 of	
	Job Number:	J00741				2007	KM		RG	ior .	Start Date Finish Dat	: 19.03.1 e: 19.03.1
h	Borehole mN	mE			Ground R.L.		ter/	pour	1		2 2 5	1.0000
Stratigraph	Location: Descriptio	n: Refer to site plan			Orientation:	vertical	Sroundwater	asin Mel	very	100	Sample and Laboratory Test Details	Vane Dial / Sensitivity & SPT
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	encountered medium gr inclusions	avel sized silt carbonace	routs	Piezonel		uuuuu		SPT		9.0-9.1m	SPT at 9.0-9.46m 7/9/12 N=21	
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	becoming dark grey and		rown/grey	10.5			1111111111		SPT			UTP SPT at 10.5-10.95 10/10/7 N=17
In	with very thin bed of stiff	sity CLAY		L 					76			
				12.0			anan anan		SPT			UTP SPT at 12.0-12.45 9,13,15 N=28
				113.0					100			
	becoming dense			- 13.6 - 14.0					SPT			UTP SPT at 13.5-13.95 9/13/18 N=31
	-			14.5					94			
	at 15.0m, becoming med EOB at 15.0m. Target De Piezometer		leasured in	15.0 - - - - - - - - - - - - - - - - - - -					SPT			UTP SPT at 15.0-15.45 10/12/14 N=26
1	LANDER	Comments: Core loss from 7.	95m-8.65m	Drilling wa	tor Fal	Sand	- 666665	s	ndstor	1111	Pluto	



## 15/12/20 - Stage 12D

Tuesday, 15 December 2020 2:24 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	15/12/20 - 12pm
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to inspect 12D toe key. Contractor had already filled with SPR to 1.5m without getting us to inspect or getting fill testing so asked them to dig back to subgrade in the centre. Firm soils exposed at the base, however, the toe key will be about 2-3m thick so no undercut was recommended. Contractors said they needed to backfill the area immediately. We advised that they need to give adequate notice for geotech and fill test inspection in this case.

Also, inspected stripping over Lots 612 & 612. Generally very stiff natural ash soils were exposed, however, some topsoil/rubbish was observed which we recommended be undercut to natural ground. Will look at this again tomorrow afternoon with Chris.

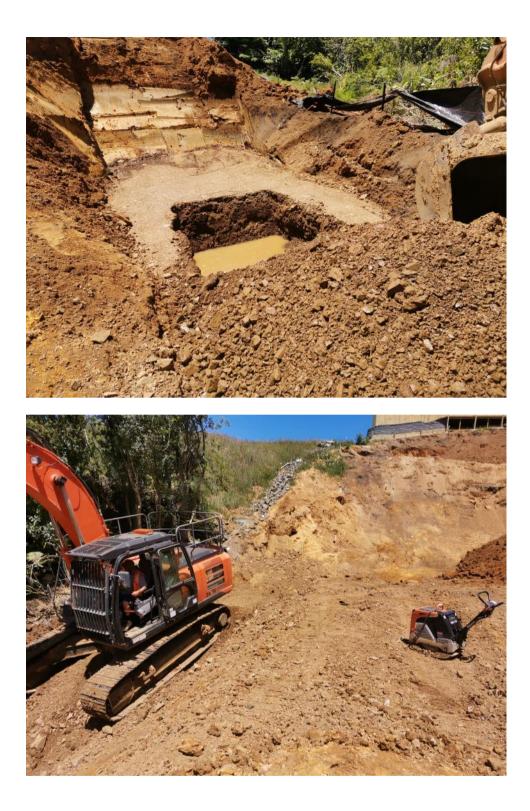


12D topsoil stripping:



12D fill/toe key area:







## 16/12/20 - REB1&2, welded tuff area, Stage 12D

Wednesday, 16 December 2020 5:56 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	16/12/20 - 1pm
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site with Chris and Shane for a senior/principals site visit. Observed the following:

- Welded tuff materials are still being cut behind the site office. We need to discuss with Russell options here. Ether the materials can be left in place and the end users will need undercut the materials with bigger diggers etc when doing footings and drainage lines, or the material can be undercut by approx. 0.6-1m and backfilled with certified clay, however, this will raise the expansive class in these lots (which would currently be good ground).
- REB2 shear key excavations. They have filled about 2m and have missed testing again. Advised Dylan to excavate some areas down to 1m for a test prior to placing further fills. Will inform Josh.
- REB1 is near level and is set to be completed in the new year. Will catch up with Trevor on the first week back to organise inspections for the final geogrid layers here to ensure they are placed properly around drainage lines.
- Stage 12D toe key. The contractors failed their previous test here, achieved CIV readings of around 10. They removed the material, discarded the wetter SPR and refilled with a heavier roller. Josh to retest. The model also needs to be rechecked to confirm minimum toe key requirements are adequate.
- Also observed the stripping area in lots 611 and 612. Rubbish/pre-existing fills are still present. Reiterated that these materials be fully undercut. Will return to site tomorrow to observe general earthworks operations.





# 17/12/20 - Stage 12D

Thursday, 17 December 2020 1:23 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	17/12/20 - 10am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Inspected backfill in 12D toe key. Compaction appears to be going well with no obvious deflections noted under a 12T smooth drum roller. The latest fill test passed on both clegg hammer and backscatter testing.

Also observed undercutting of topsoil/pre-existing fill in the top section of Stage 12D. The base of the undercut is generally down to natural ground, although there is some minor topsoil mottling to chase out. The existing fill cut shows a layer of topsoil within the fill placed by Dines which is within 2m of surface level. Advised that this should be undercut and removed from the fill. Will return to site again tomorrow morning to inspect.







## 18/12/20 - REB1&2, Stage 12D

Friday, 18 December 2020 12:21 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	18/12/20 - 10:45am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Inspected the following at the request of Dylan:

- At top of Stage 12D, topsoil horizon has been chased out and is ready to be filled again.
- At Stage 12D toe key, they have filled up another metre and are waiting for the next test. Hardfill surface appears well compacted visually. Asked Dylan to get the extent of the key surveyed to ensure that the key matches the full extent of the fill batter.
- At REB1, geogrid is being laid out, with grid lengths of around 5m and 14m being recorded as per our design for the top layers.
- At REB2 shear key, contractor has informed that additional testing was undertaken at 1m and 2m fill heights. Also observed stripping to the base of the key in the rear portions. Hard surface encountered here. A trial pit at the base was only able to be excavated approximately 1m with a rock bucket, consistent with the previous test.



Stage 12D toe key:

REB1:



REB 2 shear key:









12D topsoil horizon stripping:





## 21/12/20 - Stage 12D

Monday, 21 December 2020 2:09 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	21/12/20 - 12:15pm
Author:	
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at request of Dylan to observe site works prior to the Christmas break. Not much has progressed since our previous visit. Toe key has been completed and clay fill for the 1(v) in 3(h) batter is now commencing.

Also requested that some more fill be placed to buttress the site cut at the top of the gully flank to ensure stability is maintained to the house.



# 06/01/21 - REB2, Fill K, welded tuff area

Wednesday, 6 January 2021 4:22 PM

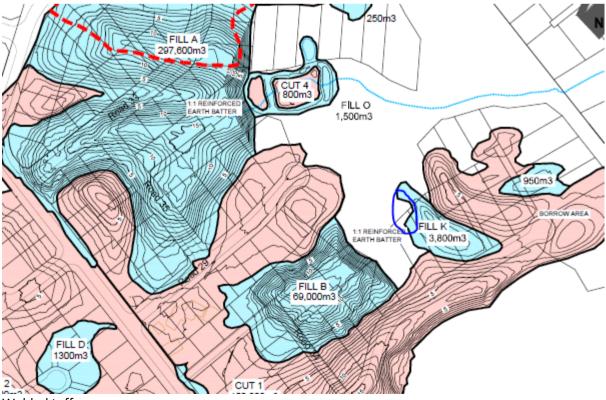
### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	06/01/20 - 2pm
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at the request of Trevor to inspect the following areas:

- REB 2 shear key: Inspected the excavation to the base of the shear key in the rear/southern corner of the key. Hard, orange silts exposed consistent with the previous observations. There is still an area in front of the key which needs to be fully pumped out an backfilled also.
- Extent of works adjacent to Fill K: Approximate area shown in blue on the annotated drawing below. Trevor discussed that the area is to be extended down to the main gully now. Advised that any stripping needs to be down to the stiff natural ground and any mullock or colluvium removed. Further advice to be given once the area is opened up and we can observe ground conditions. Trevor will send some plans through to us for our initial thoughts on what engineering solution may be required.
- Welded tuff area: The contractors have about 1m of further cut required to achieve design subgrade level. Trevor advised that they will survey the welded tuff once they reach this depth so we can advise Dines.



Welded tuff area



Addition to extent of works



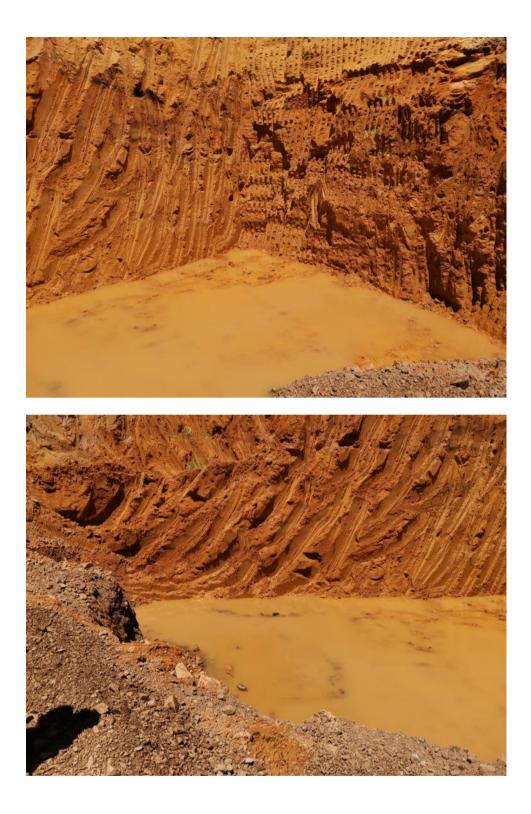




REB2 shear key







# 12/01/21 - REB2, old slip area

Wednesday, 13 January 2021 12:45 PM

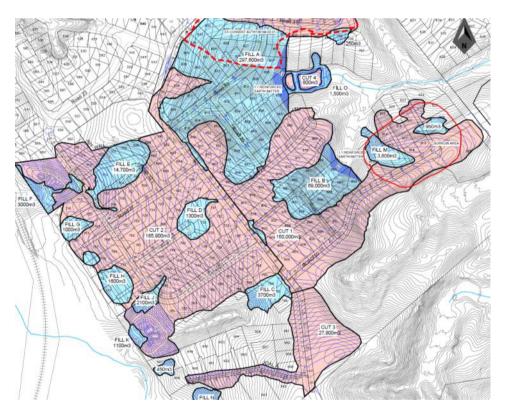
## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	12/01/21 - 2:45pm
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at the request of Dylan to inspect stripping for cut 1 in the approximate area outlined in red on the image below. Cut exposed very stiff natural ground. No seepages or soft ground observed though this area. Minor fills to be placed in this area next.

Also inspected REB 2 shear key progress. Backfill of the shear key in progress. Fill testing was being carried out at the time of our inspection of the area beyond the key where the groundwater was being pumped from.















## 13/01/21 - REB1&2

Wednesday, 13 January 2021 4:31 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	13/01/21 - 2:30pm
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site with Chris to carry out general inspection of the site.

In REB1, drainage is being installed and the contractors have followed our advice by only cutting the grid perpendicular to the slope face and installation additional perforated drainage in the service trench as an additional measure.

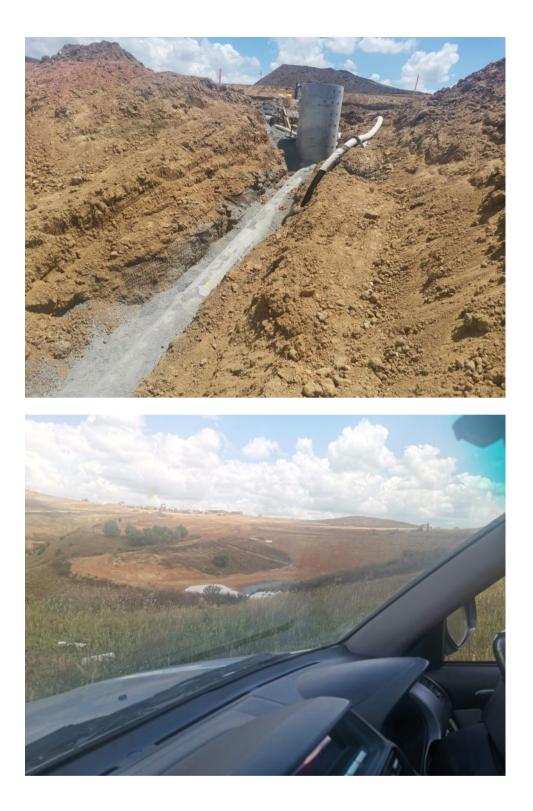
REB 2 shear is continuing with backfill operations.

Observed cut 1 stripping (which was observed with Dylan yesterday). Will have a look at recent reporting and assessments that Matt has done recently with Chris tomorrow to determine if there are any additional requirements here.

















# 15/01/21 - REB2

Friday, 15 January 2021 2:06 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	15/01/21 - 12:15pm
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at the request of Dylan to inspect the final section for the base of the REB2 shear key. The shear key was extended down to the hard, orange silty materials previously identified at the base of the shear key. A scratch with diggers rock bucket confirmed hard materials.

On site Mitchell confirmed that 1.4m of SPR hardfill is required until the base of the GAP65 layer at the base of the REB batter.

Mitchell is also planning on the setout for the counterfort drains next week. Advised we will inspect the setout and modify this if required.





# 18/01/21 - REB1

Monday, 18 January 2021 4:52 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	18/01/21 - 11am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at the request of Trevor to catch up on site wide earthworks that occurred while he was on leave last week.

Visited the old slip adjacent to the silt pond below REB1. Dines plan to start remediating this area and placing a minor fill batter. Advised that an underfill drain is required here below the fill. The plan is the bench up the existing feature, ensuring any old slip debris or otherwise unsuitable soils are removed prior to fills being placed. Advised that we should be on hand regularly for inspections while these works are being carried out.

Elsewhere, not much has progressed since last weeks inspections. Some clay is being laid out to dry in the large cut area adjacent to the REB2 shear key.

# 25/01/21 - REB2, old slip area

Monday, 25 January 2021 12:45 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	25/01/21 - 10:30am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to inspect the following:

- Observed placement of underfill drain within Fill M. Advised contractors that they could remove the top half of the drain as this is above the natural gully feature.
- Contractors wanted to know whether they should put an underfill drain in the eastern part of Fill M. 1.5m of clay capping is proposed. Advised the contractors to trench this in 0.5m to enusre a 2m cover is maintained to avoid any tags on the lots (920, 926, 927).
- Observed stripping operations below proposed Fill C. The topsoil strip revealed hard ash soils and tuff materials consistent with the materials observed in our trial pits from the GIR. Advised that an underfill drain should be trenched in here similar to below Fill M.
- Observed muckout of organic and soft alluvium materials below the underfill drainage line upslope of the REB2 shear key. The contractor had excavated down to the hard materials exposed at the base of the shear key. Advised him that this could be reduced to the base of any major organic layers, provided that the inorganic soils are competent enough the place and compact engineered certified fills upon. Will return in a day or two to observe progress.









# 26/01/21 - REB2

Tuesday, 26 January 2021 1:44 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	26/01/21 - 11am
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site at the request of Dylan to inspect REB2 earthworks operations.

The undercut of organic alluvium has now been backfilled with SPR and the undercut had extended back into the gully to intercept the undisturbed/intact underfill drain. A drainage line has been trenched through the centre of the shear key to allow this drain to outlet into the main gully. The contractors plan to place two coils in this trench for redundancy.





# 27/01/21 - REB2, Stage 12D

Wednesday, 27 January 2021 2:03 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	27/01/21 - 10:15am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to inspect general earthworks operations. Observations and recommendations as follows:

- Within Stage 12D the 1(v) in 3(h) batter has been formed to final height and has been trimmed and topsoiled.
- Stripping has commenced up the flanks of the gully in REB2 for preparation of the fill batter placement. Recommended the installation of 3 additional underfill drains in areas of seepage, two up the eastern flank and one up the southern (rear) which will connect into the existing underfill drain.
- Discussed that benching into the flanks will require an average bench depth of 2m which Dines have already programmed up.
- Will meet on Friday with Cirtex and Chris to discuss topsoiling the face of REB1 as the contractors are having difficulty getting the topsoil to stay in place.





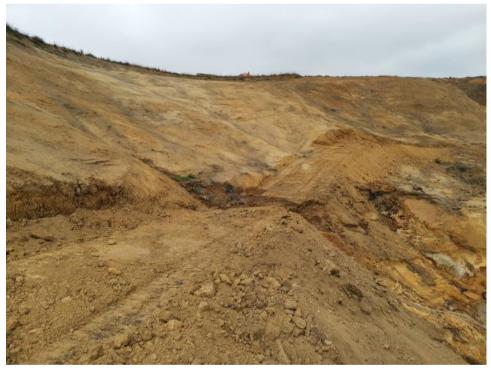


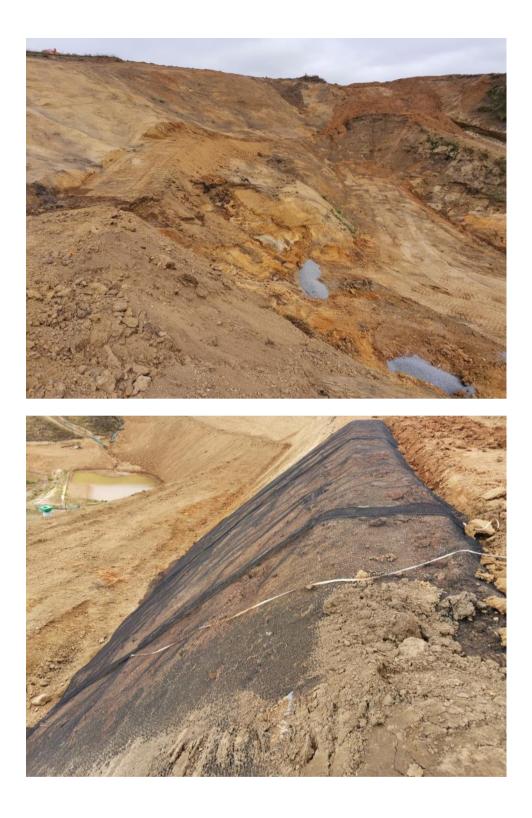












# 29/01/21 - REB1&2

Friday, January 29, 2021 5:27 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	29/01/21 - 10am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site with Chris to inspect the construction of the REB1 batter. Discussed that it would be difficult to get a 150mm layer of topsoil to bind to the face / the fortrak grid system. Will organise for Cirtex to inspect and provide advice.

At REB 2, construction of the shear key is almost complete and several underfill drains have been installed as per our recommendations.

Also inspected final works for lots 611 and 612, adjacent to stage 12D. Both lots have been topsoiled, with 100mm of topsoil measured in both lots.







## 05/02/21 - REB 1

Friday, 5 February 2021 2:52 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	05/02/21 - 10am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site to meet with Grant (Cirtex) and Dylan to discuss the facing for REB1.

Grant advised the product Dines are using is not the best for such a steep slope, however, he is going to discuss options with others at Cirtex and provide further advice as to how the facing mat should be installed and how they are joined down the slope.

Also inspected REB2 - the 1m GAP65 layer has been placed and tested via clegg hammer, and the 19m long layer of grid is laid over top as per our design recommendations.







Advice email from Cirtex:



#### **Kyle Meffan**

From: Sent:	Grant Suckling   Cirtex Industries Ltd <grant@cirtex.co.nz> Tuesday, 9 February 2021 5:55 AM</grant@cirtex.co.nz>
То:	Kyle Meffan
Cc:	DylanH@dinesgroup.co.nz; trevorm@dinesgroup.co.nz
Subject:	CIRTEX - Hitchen Road Topsoil Batter

#### Hello Kyle,

Good to meet with you onsite last week. The Fortrac 3D on the first cut looks good, some additional small pins on overlaps may help hold he Fortrac in place while topsoil is placed. The purpose of the Frotrac is to initially help retain top soil then long term is to reinforce the root zone of the vegetation to stop vegetation sliding off the face. The top soil being currently used is very dry and powdery so very little soil retention is being achieved. As discussed there is an area at one end where wetter soil had been packed on with excavator which may help achieve a better outcome with the vegetation long term.

As to placing he next runs of Fortrac 3D we would suggest the Fortrac is pinned in the 300mm x 300mm V Trench already cut with Platipus Anchors or 400mm DuraMat Pins. The advantage of the Platipus Anchors being higher pull out resistance and also the anchor plate that would hold the Fortrac better.

https://cirtexcivil.co.nz/product/platipus-args-anchors/ https://cirtexcivil.co.nz/product/duramat-pins/

Cirtex does not get involved with hydro-seeding however Jon Tomsett from Cirtex has had a lot of experience in the past and has some suggestions:

#### Grant,

Assuming the goal on these slopes is long term grass establishment I would recommend the following: Engage a reputable Hydroseeding contractor to tailor an application with a broad spectrum seed mix inclusive of Kikuyu for long term establishment and stability. The immediate application ideally will include a heavy application and tackifier that will assist with bonding the lose topsoil providing some protection against a significant rainfall event.

I would then suggest applying a tackified hay mulch over the top to retain moisture and encourage germination and establishment in these drier months. A Hessian overlay could be considered but a Hay mulch would be my preference if a contractor is comfortable they can achieve coverage.

Happy to come to site or discuss further if required.

Do not hesitate to contact me if you have any questions.

Kind Regards,

Grant Suckling Technical Sales

T 0800 247 839 W www.cirtexcivil.co.nz





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# 09/02/21 - REB2

Tuesday, 9 February 2021 4:46 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	09/02/21 - 9:30am
Author:	КМ
Plant Operating:	
Weather:	

### Site Observations and Instructions:

Visited site to observe earthworks progress over the weekend, however, not much had been done since last week.

There is still one more layer of GAP65 to place in REB2 prior to clay filling starting.





# 11/02/21 - REB1&2, welded tuff area

Thursday, 11 February 2021 1:02 PM

### Site Inspection Record

Project # & Name:	J00113 - Hitchen Block
Date & Time:	11/02/21 - 10am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Visited site at the request of Dylan to inspect site works. The following observations were made:

- Within the area of welded tuff, Dines have decided to change the subgrade levels and build up the area with terraced platforms utilising clay fills. Competent, hard subgrade exposed at base suitable to place fill upon.
- Within REB2, the 1m GAP65 layer is now complete and clay filling is about to commence. The rear blanked drain is being installed, however, the main underfill drain has been severed and the contractors were advised to repair this prior to backfilling.
- At the REB1 wall face, the contractors are still having difficulty placing topsoil along the facing. Advised the contractor to keep talking to Cirtex and their hydroseeders for advice as necessary.
- The old slip above the REB1 silt pond is about to be backfilled. Advised contractors to trench in an underfill drain prior to benching into the slope and filling. The underfill drain will outlet at the same point at the REB underfill drains, but with a different outlet point.

























# 19/02/21 - REB2

Friday, 19 February 2021 8:42 AM

### **Site Inspection Record**

Project # & Name: J00113 - Hitchen Block

#### Site Observations and Instructions:

Joolis - Hitchen Block 18/2/21 Km 1:30pm - observed contraction of fills (class fille) within REB2 - Adequite benching into sully flacks observed.



# 24/02/21 - REB1&2

Wednesday, 24 February 2021 12:11 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	24/02/21 - 10am
Author:	KM
Plant Operating:	
Weather:	

### Site Observations and Instructions:

- Fills are progressing in REB2. All 4x underfill drains can still be observed at the toe of the batter.
- Slip remediation works progressing below DEB10 (adjacent to REB1 silt pond). Underfill drain observed below current fill batter as per our recommendations.



















# 26/02/21 - REB2

Friday, 26 February 2021 4:42 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	26/02/21 - 2pm
Author:	KM
Plant Operating:	
Weather:	

- Observed progress of REB2 batter fill operations.
- Geogrid laid out to approx. 19m length as per our design. A gap remains in the centre section which is to be picked up and covered at the next primary grid lift.
- Fill compaction results have generally been passing, after a series of failed fill tests a few weeks ago (failed on air voids).
- Adequate benching exposed along the gully flanks.







# 08/03/21 - REB2, Stage 12D

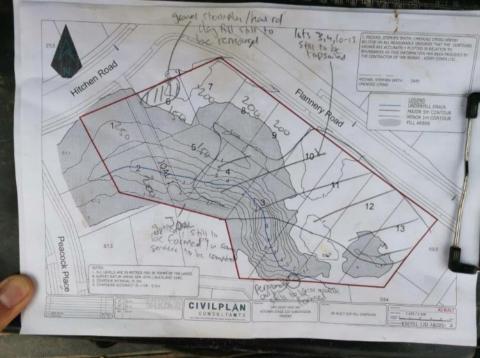
Monday, 8 March 2021 1:08 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	08/03/21 - 10:15am
Author:	KM
Plant Operating:	
Weather:	

- REB2 fill placement is progressing. 2m face tightening grid currently exposed and waiting for the next layer of clay fill.
- At stage 12D carried out topsoil measurements. The following still needs to be addressed:
  - No permanent concrete outlet formed for underfill drain at toe of batter.
  - Lots 3,4, 10-13 are still to be topsoiled
  - $\circ~$  Stockpile of gravel / loose clay present adjacent to JOAL within lot 6, to be cleared and topsoiled









Inspections Page 318





REB2:





## 12/03/21 - REB1, REB2

Friday, 12 March 2021 4:29 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	12/03/21 - 1pm
Author:	KM
Plant Operating:	
Weather:	

- Filling is now complete in the old slip adjacent to the stormwater pond below REB1. The area is now being trimmed and topsoiling is planned soon.
- Inspected filling operations in REB2. The REB is now in the middle grid area and 10m lengths of SG500 grid were recorded throughout.
- Cut progressing above REB2.

















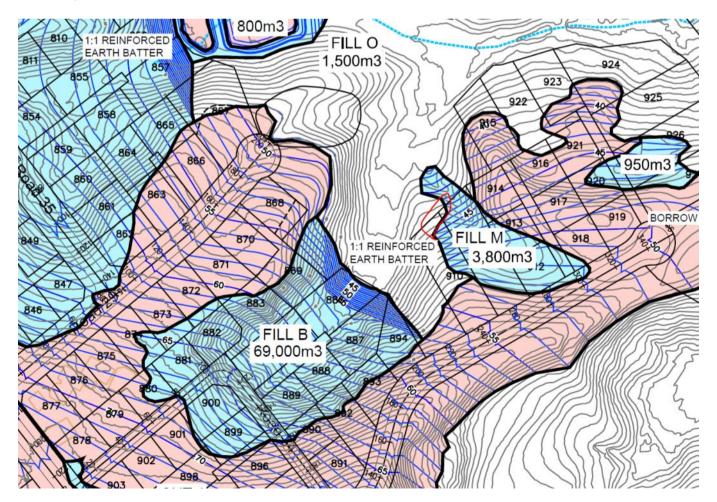
# 24/03/21 - Fill M&O, REB2,

Wednesday, 24 March 2021 3:25 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	24/03/21 - 9am
Author:	KM
Plant Operating:	
Weather:	

- Inspected topsoil stripping in Fill O. Approximate location shown by black annotation. Very stiff ash soils exposed throughout. Recommended that an underfill drain to be placed along the existing overland flowpath here.
- Observed progress of REB 2 fill works. Approx. 3.5m of fill left to place before trimming of the 1(v) in 1(h) batter begins. Rear face drain is now complete, with the final section of this drain being installed today.
- Observed permanent stormwater pond below Fill M, approximate position shown in red below. Contractors had mucked out the pond and backfilled without notifying us. Several trial pits dug in this area which identified SPR to beyond 3m and at least 1.5-2m below the adjacent fully invert.































# 25/03/21 - Trevors photos of REB2 / Fill M pond

Thursday, 25 March 2021 8:48 AM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	
Author:	
Plant Operating:	
Weather:	









# 26/03/21 - Fill M&O, REB2

Friday, 26 March 2021 12:42 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	26/03/21 -
Author:	
Plant Operating:	
Weather:	

- Filling operations continuing in REB 2. Another layer of primary grid was in the process of being laid out. Grid measured at 10m as per the design.
- Trial pits in the Fill M stormwater pond have been partially backfilled with SPR and fill testing has come back indicating adequate compaction and density. The contractors intend to backfill the remainder of the pond with at least 1m of clay as per our advice. Pond construction will be finalised next season.
- Fill O underfill drain has not been installed yet due to bad weather forecast next week.











# 07/04/21 - REB1, REB2

Wednesday, 7 April 2021 2:25 PM

## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	07/04/21 - 12pm
Author:	KM + CE
Plant Operating:	
Weather:	

- REB 1 face is currently being trimmed. The contractors are working along a bench at approx. mid slope height and trimming back to design level. No grid was observed on the face of the excavation. Requested that up to date as-builts be supplied to us to ensure that design requirements are being met
- REB 2 has been filled to near full height and a layer of secondary grid was exposed at the time of inspection.

















# 16/04/21 - Stage 12D

Friday, 16 April 2021 4:50 PM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	16/04/21 - 10am
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Refer photos below.

Telecom with Trevor, discussed that underfill drain was not visible. He is going to install a solid pipe to avoid blockage of the drain through the topsoil at the toe of the 1(v) in 3(h) batter, and then form the permanent outlet structure at the end of the solid pipe. Advised to return on Monday to sight this.

underthill Sight 200 ain worked 250 Som 200 lot 150 100 200











# 20/04/21 - Stage 12D, REB2, Fill M

Tuesday, 20 April 2021 11:59 AM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	20/04/21 - 8am
Author:	KM
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

- Sighted underfill and permanent outlet structure in Stage 12D.
- Filling progressing in REB2, with fill placement near final subgrade level. Several more layers of fill to be placed near the crest to meet design heights.
- Underfill drain in Fill M is to outlet into the permanent stormwater pond. Advised that this should be protected with a rock channel or similar to present scour along the side of the pond.













## **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	30/04/21 - 10:15am
Author:	КМ
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

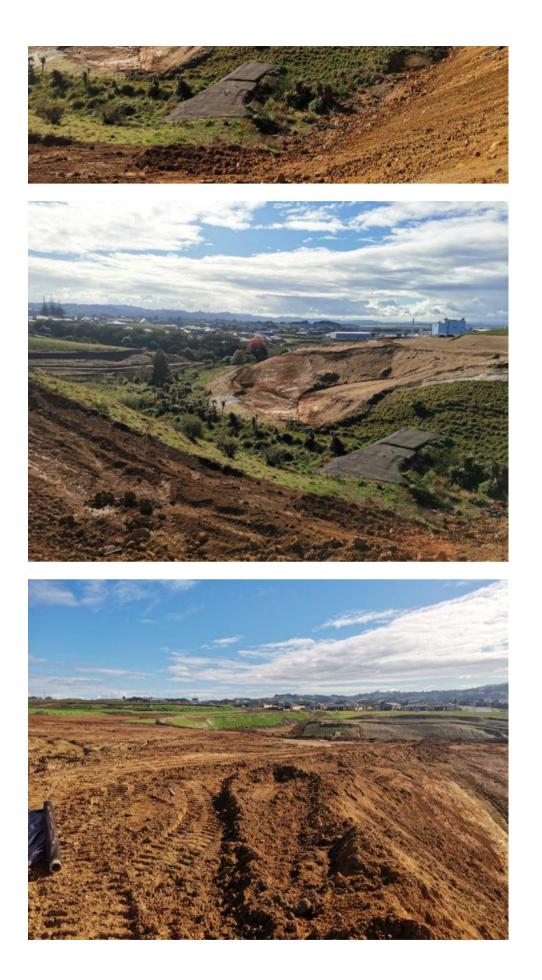
- Received a phone call from Mitchell Rutter (surveyor) this morning advising that the second to last layer of primary grid has been placed using SS20 secondary grid instead. Advised that this should be removed and the appropriate primary grid be placed. He said they would undercut the clay fill over this layer and install the correct grid.
- Primary grid had already been placed and covered at the time of arrival on site. Secondary grid was being trimmed and excavation was at least 2m wide as per the design.











## 28/05/21 - REB1 & 2, Stage 13

Friday, 28 May 2021 11:03 AM

### **Site Inspection Record**

Project # & Name:	J00113 - Hitchen Block
Date & Time:	28/05/21 - 10am
Author:	
Plant Operating:	
Weather:	

#### Site Observations and Instructions:

Observed progress at Hitchen Block. Observations as follows:

- REB 1 has been formed to a 1(v) in 1(h) face and has been hydroseeded. The initial section of hydroseeding is showing good vegetation cover.

Below REB 1, connection of the underfill drains into the manholes is about to commence. Dines will place 2x solid pipes between the manholes and a perforated novacoil within the same trench wrapping around the outside of the manhole to ensure any additional water seepages are captured.

- REB 2 has also been formed at a 1(v) in 1(h) face. Hydroseeding still planned.

Both REB's required check dam installation.

- Stage 13 nearly complete. Observed permanent underfill drain outlet beneath the permanent stormwater pond.



































